

Improving Transportation Infrastructure Resilience Against Hurricanes, Other Natural Disasters, and Weathering: Part II - Analysis of Pedestrian Bridges Failures Due to Hurricane Maria

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16. Abstract After the passage of Hurricane Maria as a strong category 4 storm through Puerto Rico, many different types of structures were affected. The objective of this investigation was to determine whether Hurricane Maria caused damages to pedestrian bridges in Puerto Rico, taking the San Juan Metropolitan Area (SJMA) as a case study. To achieve the project objective, the following tasks were performed: first, a literature review on pedestrian bridges in general was performed; second, several visits were made to the Bridges Office of the Puerto Rico Highway Transportation Authority to search for available information on pedestrian bridges location and condition; third, all the bridges on the SJMA were geolocated; fourth, visits were made to visually inspect the pedestrian bridges; fifth, by comparing the visual inspection findings to the previous inspection findings and old photographs, an assessment of possible damages induced by wind action was performed; and finally, all the documentation process was carried out. It was concluded that, out of the 21 bridges that were inspected, only four could have been affected by Hurricane Maria. It is understood that the main cause of the damages was the presence of traffic signs that were mounted on the bridges. Two of the damaged pedestrian bridges experienced inelastic lateral deflections due to the wind pressure these signs exerted on the bridge structure. On the other two, the transit signs were detached from their bottom supporting bases, probably due to wind induced vibrations; the signs were swinging due to the lack of lower end supports, situation that constituted a safety hazard to the vehicles that passed below the bridge. It is recommended that special considerations are taken when attaching signs to pedestrian bridges and that, after an extreme wind event, preliminary inspections should not only contemplate the main bridge structure, but any flexible elements attached to it. Volume 2 of this project investigates a steel truss pedestrian bridge that experienced permanent lateral deflections due to the hurricane. The main purpose of the research was to determine if the presence of traffic signs mounted on the bridge were the main cause for the inelastic lateral response. A detailed inspection of the bridge was performed, a condition assessment was developed, lab tests were carried out to characterize the material, and FE simulations were executed to capture the response with and without the signs. It was concluded that the signs played a predominant role in the inelastic response, but that there were also other conditions that may have contributed to the plastic deflections.			
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1 Introduction

Pedestrian bridges are structures that are required to provide pedestrians with a safe passage in areas that are not accessible by walking (i.e., crossing water bodies) or that impose special risks to pedestrian flow (i.e., crossing high volume roadways). As they are obviously used in different situations with different characteristics, pedestrian bridges can be built with different materials (such as reinforced concrete, prestressed concrete, steel, wood, stone, aluminum, among others) and use different structural systems (such as arches, beams and slabs, trusses, suspension cables, or cable-stayed, among others). As these structures are very important for the safety of the public, it is imperative to continuously evaluate their integrity and serviceability, especially after an extreme event like a hurricane.

On September 20, 2017, Hurricane Maria crossed Puerto Rico from its southeast end to its northwest end (see Figure 1.1). The island suffered major damages in diverse structures, such as houses, multistory buildings, vehicular bridges, roads, sport venues, and storage facilities, among many others. The damages caused by Hurricane Maria in Puerto Rico and the Virgin Islands has been estimated to be around \$90 billion, making it the third most expensive hurricane in the history of the United States (Pasch, Penny, & Berg, 2019). Due to the intensive and extensive damages produced by Hurricane Maria, it was decided to explore the effects of this extreme event on pedestrian bridges in order to evaluate to what extent they were affected.

The objective of this project was assessing pedestrian bridge damages that may be attributed to Hurricane Maria and determining their causes and possible improvements that may have avoided such damages. The San Juan Metropolitan Area (SJMA) was selected as the case study area. The project had five principal stages: conduct a literature review on pedestrian bridges (the specifications that apply to the design and inspection of pedestrian bridges in Puerto Rico), identify

and geolocate the pedestrian bridges in the SJMA, consult and document available drawings and inspections performed in previous years for each pedestrian bridge identified in the SJMA, perform field visits and inspections to each pedestrian bridge, and finally evaluate the findings and assess if the encountered damages were related to the hurricane. This document covers all the stages of the project and presents conclusions and recommendations.

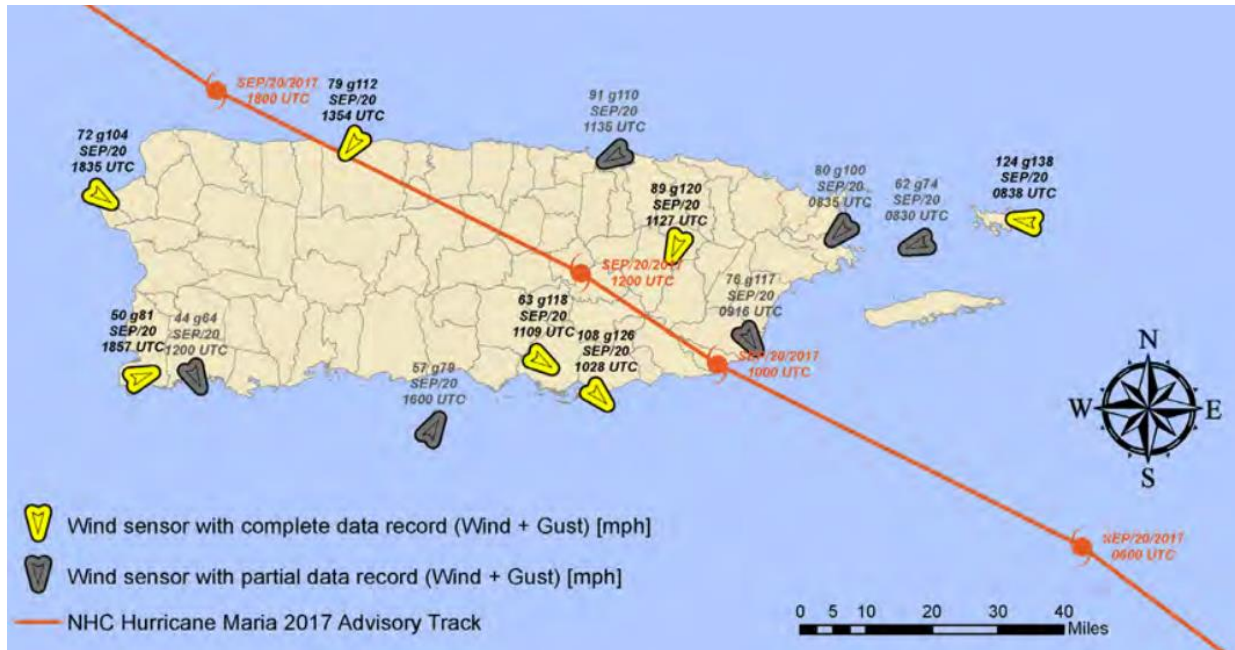


Figure 1.1: Trajectory of Hurricane Maria over Puerto Rico (FEMA, 2018)

2 Literature Review

Two important aspects for a resilient pedestrian bridge construction are the design process and the inspection and condition assessments that allows a proper maintenance. In this chapter, a summary is presented of the specifications that apply to the design and inspection of pedestrian bridges in Puerto Rico.

2.1 Design Specification for Pedestrian Bridges

The design and construction of pedestrian bridges should be done in accordance with the *LRFD Guide Specifications for the Design of Pedestrian Bridges* (AASHTO, 2009). These specifications are meant to be used as a supplement to the *LRFD Bridge Design Specifications* (AASHTO, 2020), which are widely used for the design of vehicular bridges.

Pedestrian bridges are designed to support different loads, such as dead load, pedestrian loading, maintenance vehicle load (unless vehicular access is prevented by permanent physical methods), equestrian load, wind load, and fatigue load, among others. Again, both sets of specifications mentioned in the previous paragraph must be used together in order to determine the applicable loads and their combinations.

Given the objective of this project, the determination of wind loads is of particular interest. The *LRFD Guide Specifications for the Design of Pedestrian Bridges* (AASHTO, 2009) indicates wind loads shall be determined in accordance with the *Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals* (AASHTO, 2015), unless otherwise required by the bridge owner. This is done because pedestrian bridges are potentially more flexible than vehicular bridges and because of the possibility of traffic signs being mounted on pedestrian bridges (AASHTO, 2009). The *LRFD Guide Specifications for the Design of Pedestrian Bridges*

(AASHTO, 2009) clearly indicate that signs mounted on pedestrian bridges must be considered when calculating wind loads.

2.2 Inspection Procedures for Pedestrian Bridges

The Puerto Rico Highway and Transportation Authority (PRHTA) used to be in charge of the inspection of pedestrian bridges in Puerto Rico. For these inspections, PRHTA followed the standards of the National Bridge Inventory (NBI). When visual inspections of pedestrian bridges were performed, they focused their assessments mostly to NBI Items 58, 59, and 60, which corresponds to the deck, the superstructure, and the substructure, respectively. A condition assessment was given according to the findings using a rating scale that ranges from nine (9) to zero (0). Table 2.1 presents the Condition Rating Scale used for the bridge Items 58, 59, and 60 and a description of the condition.

Table 2.1: NBI general condition rating guidelines (Federal Highway Administration, 1995)

Code	Description
N	NOT APPLICABLE
9	EXCELLENT CONDITION
8	VERY GOOD CONDITION (No problems noted.)
7	GOOD CONDITION (Some minor problems.)
6	SATISFACTORY CONDITION (Structural elements show some minor deterioration.)
5	FAIR CONDITION (All primary structural elements are sound but may have minor section loss, cracking, spalling or scour.)
4	POOR CONDITION (Advanced section loss, deterioration, spalling or scour.)
3	SERIOUS CONDITION (Loss of section, deterioration, spalling or scour have seriously affected primary structural components. Local failures are possible. Fatigue cracks in steel or shear cracks in concrete may be present.)
2	CRITICAL CONDITION (Advanced deterioration of primary structural elements. Fatigue cracks in steel or shear cracks in concrete may be present or scour may have removed substructure support. Unless closely monitored it may be necessary to close the bridge until correction action is taken.)
1	“IMMINENT” FAILURE CONDITION (Major deterioration or section loss present in critical structural components or obvious vertical or horizontal movement affecting structure stability. Bridge is closed to traffic but corrective action may put back in light service.)
0	FAILED CONDITION (Out of service; beyond corrective action.)

Appendix A presents the forms and procedures used by PRHTA for the documentation of inspections of pedestrian bridges. These documents are:

- Bridge Load Capacity Summary Form
- Critical Finding Memorandum
- Safety Inspection Report
 - Initial Inspection Report
 - Routine Inspection Report
- Standards for Photographs of Existing Bridges
- Standard Element Numbering Procedure
- Quality Assurance / Quality Control Forms
- Scope of Work of Underwater Inspection
- Procedure for Conducting Underwater Inspection
- Failure Critical Inspection Procedures

All inspections were carried out with the most up-to-date forms by PRHTA. These above-mentioned forms were filled out based on findings, giving ratings (as per Table 2.1) and comments. Any rating of 7 or less was accompanied with an explanatory comment. When the rating was 4 or less, in addition to the comment, the inspector had to attach photographs that exemplify the condition and justify the rating.

At some point before the passing of Hurricane Maria through Puerto Rico, the ownership of the pedestrian bridges passed to the municipalities, together with the responsibility of inspecting them. Still, old inspection reports by the PRHTA were collected for this project.

3 Methodology

In order to achieve the objectives of this research, the following list of tasks summarizes the procedure followed to assess the impact of Hurricane Maria on pedestrian bridges in the San Juan Metropolitan Area (SJMA) of Puerto Rico:

- Perform a literature review and collect background data.
 - Visit the offices of the Puerto Rico Highway and Transportation Authority to obtain relevant background data (i.e., inspection reports, photos, drawings, manuals of inspections).
 - Perform a literature review on pedestrian bridges.
 - Study the available inspections reports on pedestrian bridges.
 - Review news reports on bridge damages.
- Identify pedestrian bridges in the SJMA.
 - Obtain a list of bridges in the area.
 - Geo-locate the pedestrian bridges with the platform Google Earth Pro.
 - Assess ownership and accessibility.
- Perform field visit and visual inspection of pedestrian bridges in the area of study.
 - Try to identify possible damages that may have been produced by the hurricane.
 - Complement the visual inspection with interviews of local residents, when possible, to receive their input on the condition of the bridge previous to the hurricane.
 - Document with several high-quality photographs each bridge.

- Fill in forms with comments on the preliminary findings during the visit. The forms used for field inspections are presented in Appendix B.
- Evaluate the findings and compare them to previous inspections finding by the PRHTA to identify damages that may be attributed to Hurricane Maria.
- Document all information collected and the findings.

It should be pointed out that, for this project, the inspected pedestrian bridges were not rated, since the objective was only to identify the damages that were understood to have been caused by the hurricane. Those damages are described in detail.

4 Pedestrian Bridges in the San Juan Metropolitan Area

As previously mentioned, in the past the Puerto Rico Highway and Transportation Authority (PRHTA) was responsible for the inspections and evaluation of pedestrian bridges, but at some point before Hurricane Maria made landfall in Puerto Rico, that changed. Currently, the responsibility of inspecting and evaluating pedestrian bridges is of the municipality in which the bridge is located. Nonetheless, PRHTA performed visual inspections of many pedestrian bridges and still maintains information about them.

For this project, PRHTA provided inspection reports (which include plans and photos) of 32 pedestrian bridges located in the San Juan Metropolitan Area (SJMA). Out of the 32 pedestrian bridges, four had been removed prior to Hurricane Maria due to deteriorating conditions; one was removed between 2014 and 2017, while three had already been completely removed by April 2014. Interestingly, the four removed bridges were made of steel and were constructed between 1971 and 1978.

The information of the remaining 28 bridges was used to determine their location, as listed in Table 4.1. Also indicated in Table 4.1 is whether the owner of the bridge is a municipality or if it is owned by another type of entity. These bridges were then geolocated using Google Earth Pro, as shown in Figure 4.1.

Out of the 28 bridges, it was decided to inspect only the 21 bridges owned by the municipality. The seven bridges privately owned were not inspected because they would require a permit from the owner. The following two chapters present the findings of the visual inspections of the 21 bridges.

Out of the 21 pedestrian bridges inspected, it was found that four presented damages that may be attributed to Hurricane Maria. These bridges are discussed in Chapter 5, including background information and descriptions of the damages attributed to the hurricane.

Meanwhile, the bridges that did not present damages due to Hurricane Maria are discussed in Chapter 6. Although these bridges did not present damages due to the hurricane, the inspection team documented any deterioration they detected.

Table 4.1: List of Pedestrian Bridges in the SJMA

Bridge No.	Latitude	Longitude	Ownership
0626	18° 23' 46" N	66° 03' 09" W	Municipality
0745	18° 27' 08" N	66° 05' 12" W	Municipality
0748	18° 23' 42" N	66° 02' 50" W	Municipality
0960	18° 23' 35" N	66° 04' 14" W	Municipality
1095	18° 25' 03" N	66° 02' 36" W	Municipality
1137	18° 26' 50" N	66° 02' 59" W	Municipality
1307	18° 26' 47" N	66° 02' 41" W	Municipality
1418	18° 24' 36" N	66° 02' 37" W	Municipality
1478	18° 23' 57" N	66° 06' 17" W	Municipality
1513	18° 21' 04" N	66° 05' 26" W	Municipality
1616	18° 27' 38" N	66° 04' 52" W	Hotel
1736	18° 23' 42" N	66° 02' 41" W	Municipality
1743	18° 23' 48" N	66° 02' 35" W	Municipality
1762	18° 24' 03" N	66° 03' 02" W	University
1774	18° 24' 26" N	66° 04' 21" W	Municipality
1888	18° 26' 41" N	66° 04' 09" W	Hospital
1897	18° 23' 12" N	66° 03' 41" W	University
1926	18° 25' 27" N	66° 04' 14" W	Municipality
1927	18° 25' 29" N	66° 03' 33" W	Airline
2087	18° 26' 37" N	66° 04' 14" W	Municipality
2336	18° 24' 43" N	66° 01' 44" W	Municipality
2351	18° 24' 40" N	66° 06' 08" W	Telecom Co.
2355	18° 24' 30" N	66° 02' 10" W	Municipality
2549	18° 23' 22" N	66° 04' 51" W	Hospital
2566	18° 24' 28" N	66° 03' 32" W	Municipality
2665	18° 27' 55" N	66° 05' 27" W	Municipality
2682	18° 26' 53" N	66° 03' 35" W	Municipality
2683	18° 26' 51" N	66° 03' 21" W	Municipality



Figure 4.1: Aerial photo of pedestrian bridges in the San Juan Metropolitan Area (Source: Google Earth Pro)

5 Pedestrian Bridges with Damages Possibly Caused by Hurricane Maria

Out of the 21 pedestrian bridges in the San Juan Metropolitan Area (SJMA) inspected for this project, four exhibited damages that may have been caused by Hurricane Maria. These are:

- PB 0960 – Traffic sign detached from mounting assembly and damaged handrail
- PB 1137 – Lateral deflections in the plastic range
- PB 1307 – Lateral deflections in the plastic range
- PB 2336 - Traffic sign detached from mounting assembly

In this chapter, background information is presented on these four pedestrian bridges, as well as the findings of the visual inspections performed for this project. The background information was obtained from information provided by the Puerto Rico Highway and Transportation Authority (PRHTA).

5.1 Pedestrian Bridge 0960

Pedestrian Bridge 0960 (shown in Figure 5.1 and Figure 5.2) was constructed in 1967. It is a slab type bridge made of reinforced concrete. The bridge is located at coordinates 18° 23' 35" N and 66° 04' 14" W, connecting Villa Nevarez Urbanization with Quintas de San Ramon Urbanization, which is next to the Puerto Rico Medical Center (see Figure 5.3). The bridge crosses above Las Americas Expressway (PR-18), where by 2005 it had an ADT of 178,900 and a Truck ADT of 4%. Neighbors from the area estimate that the bridge is mostly used by medical students living in Villa Nevarez and working at the Medical Center.



Figure 5.1: South view of PB 0960



Figure 5.2: North view of PB 0960

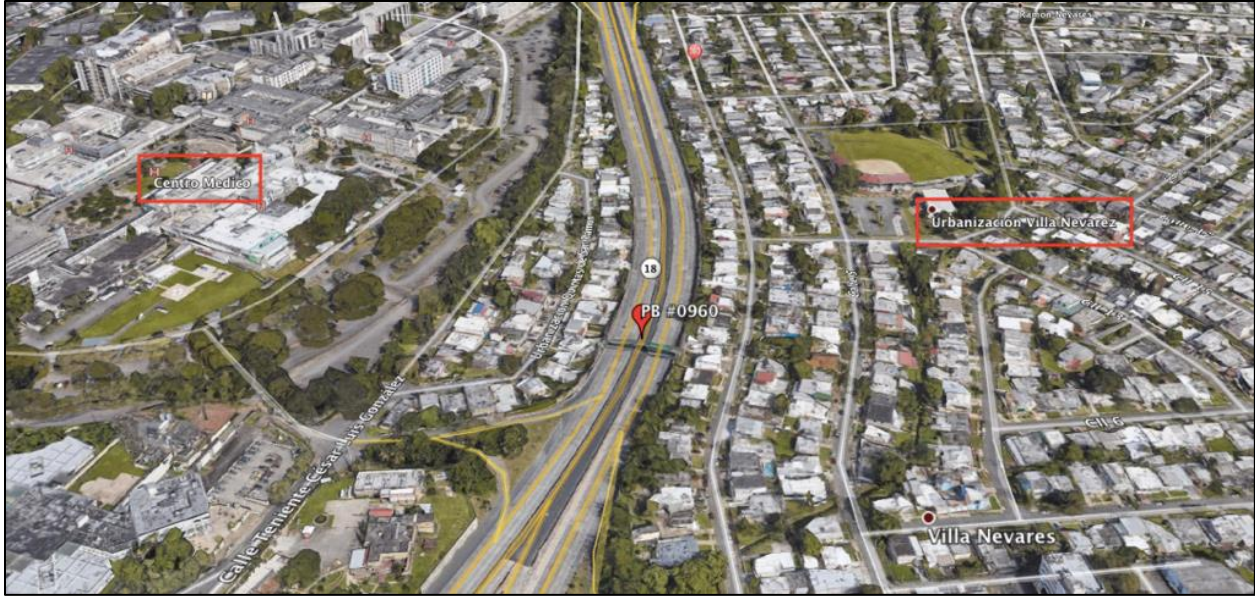


Figure 5.3: Location of PB 0960 (Source: Google Earth Pro)

5.1.1 PRHTA Inspection Reports

PRHTA provided inspection reports with the following dates, (full reports are presented in Appendix C):

- April 15, 2014
- August 12, 2002
- February 9, 2000
- March 20, 1998
- March 5, 1996
- September 30, 1992

The latest inspection report, dated April 15, 2014, refers to an inspection carried out on December 9, 2008. The report for the 2008 inspection was not available. Table 5.1 presents the ratings given to the bridge's deck, superstructure, and substructure on two most recent inspections. It can be seen that the deck showed a slight improvement, but the superstructure and substructure kept the same rating.

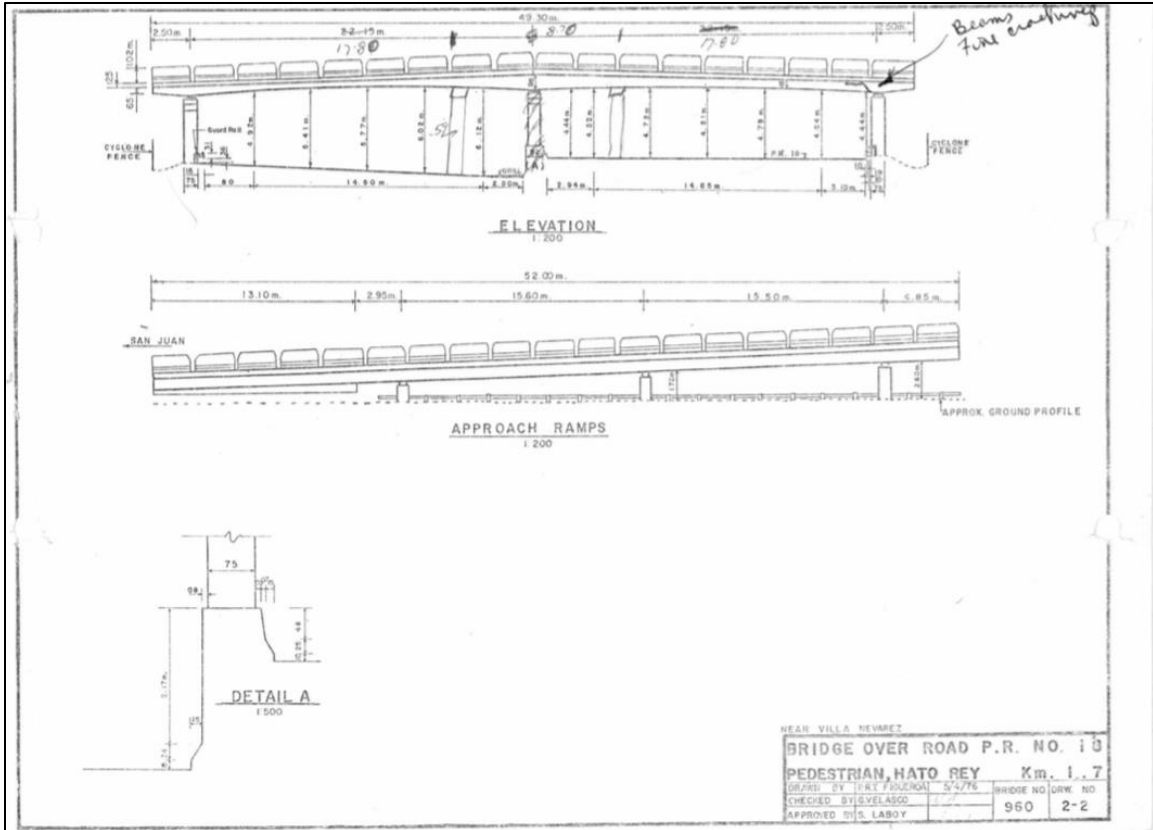


Figure 5.5: PB 0960 plans Sheet 2 of 2 (Source: PRHTA)

5.1.3 Photos of Inspections by PRHTA

Figure 5.6, Figure 5.7 and Figure 5.8 present photos of the bridge from 1972. It can be seen in this pictures that, back then, the bridge did not have chain-link fencing, nor did it have traffic signs attached to it.



Figure 5.6: North side of PB 0960 on July 21, 1972 (Source: PRHTA)



Figure 5.7: Underside of PB 0960 on July 21, 1972 (Source: PRHTA)



Figure 5.8: Walkway of PB 0960 on July 21, 1972 (Source: PRHTA)

5.1.4 Photos Before and After Hurricane Maria

To detect if the bridge suffered any major deformations due to Hurricane Maria, old photographs from PRHTA were compared to pictures taken in 2019. One such comparison can be made between a photo of the walkway from 2000 (Figure 5.9) and a another from 2019 (Figure 5.10). No deformation in the structural elements was detected from the comparison of photographs.



Figure 5.9: Walkway of PB 0960 on September 6, 2000 (Source: PRHTA)

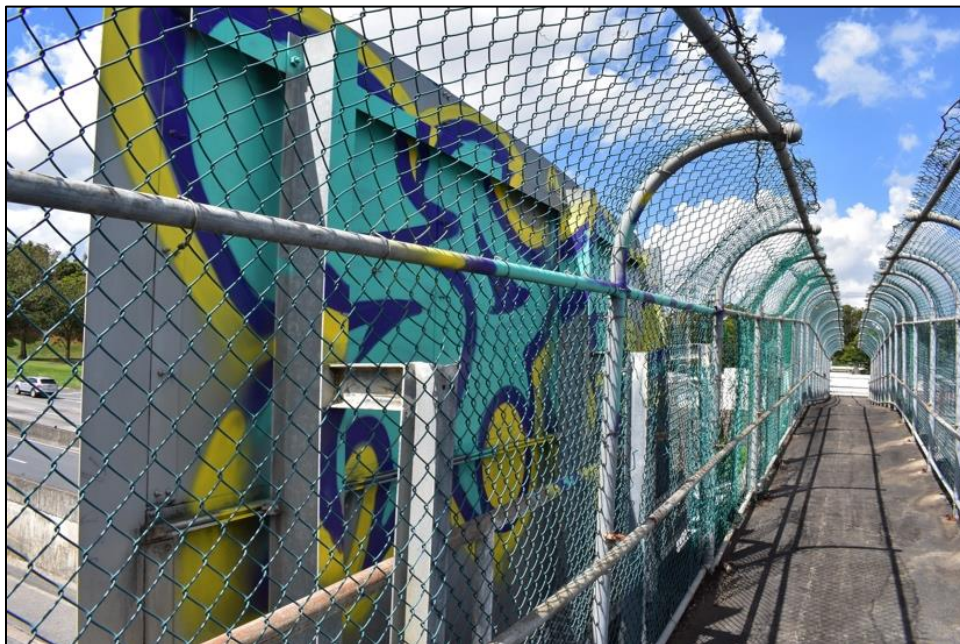


Figure 5.10: Walkway of PB 0960 on March 13, 2019

5.1.5 Inspection by PUPR

The inspection by PUPR was conducted by Civil Engineering undergraduate students Adriana Murati-Núñez and Jonathan Hernández on March 13, 2019. During the inspection, the weather was sunny. The bridge was open to pedestrians.

During the visit, a neighbor approached the inspection team and expressed concern for the safety of the people that use the bridge. One of the concerns was that the sidewalk (at the location indicated in Figure 5.11) used to approach the bridge ramp on the west side has a hole with a depth of more than 1 ft. This hole was covered with a pallet, as shown in Figure 5.12. In the same area, a section of the sidewalk has sunk, as shown in Figure 5.13. Other observations made by the inspection team were the growing vegetation on the chain-link fence of the ramp (see Figure 5.14) and presence of corrosion on the support bases of the chain-link fence (see Figure 5.15).



Figure 5.11: Pedestrian path with the red circle indicating location of tripping hazards (Source: Google Earth Pro)



Figure 5.12: Holes in sidewalk covered with a pallet



Figure 5.13: Sunk section of sidewalk



Figure 5.14: Vegetation in chain-link fencing



Figure 5.15: Corrosion in base support of chain-link fencing

At the time of the inspection, the bridge had attached five signs of different sizes, as it can be appreciated in Figure 5.1 and Figure 5.2. In terms of possible damages caused by Hurricane Maria, it was observed that one of the traffic signs mounted on the bridge was detached at the bottom end from the mounting assembly, as indicated in Figure 5.16. It is theorized that the detachment was due to wind induced vibrations that occurred during the hurricane.



Figure 5.16: Traffic Signs partially detached from support

It was observed that all the traffic signs mounted on the bridge were attached to the handrails, as shown in Figure 5.17. It was noticed that there were cracks at the base of handrails posts (see Figure 5.18), and one section was detached (see Figure 5.19) and bent, affecting the sign (see Figure 5.20). Wind vibrations may have induced the cracks and triggered the bent.



Figure 5.17: Sign mount attached to handrail



Figure 5.18: Cracks at the base of the handrail posts



Figure 5.19: Detached handrail post



Figure 5.20: Deformed sign due to handrail bent

5.2 Pedestrian Bridge 1137

Pedestrian Bridge 1137 (shown in Figure 5.21) was a steel structure constructed in 1968. The bridge was located at coordinates $18^{\circ} 26' 49.75''$ N and $66^{\circ} 02' 59.80''$ W. This Bailey bridge connected Norte Shopping Center and Luis Lloréns Torres public housing complex at the north with the Villa Palmeras sector at the south, as shown in Figure 5.22. The structure is very important for the security of pedestrians, as it crosses over the Román Baldorioty de Castro Expressway, which has a very high vehicular flow (67,300 ADT with the Truck 4% ADT in 2017). As it will be explained later, PB 1137 was removed by PRDOT because it was highly affected by Hurricane Maria. Still, the PUPR team was able to inspect the structure before its removal.



Figure 5.21: Pedestrian Bridge 1137



Figure 5.22: Location of PB 1137 (Source: Google Earth Pro)

5.2.1 PRHTA Inspection Reports

PRHTA provided inspection reports with the following dates, (full reports are presented in Appendix D):

- May 15, 2017
- January 23, 2014
- June 29, 2001
- March 11, 1999
- August 19, 1996
- December 22, 1994
- February 24, 1993
- June 8, 1991
- June 23, 1989
- July 28, 1987

- April 21, 1986
- July 16, 1975
- November 13, 1972

Table 5.2 presents the ratings given to the bridge’s deck, superstructure, and substructure on two most recent inspections. It can be seen that the deck had reached a rating of Poor (4), while the substructure kept deteriorating.

Table 5.2: PB 1137 ratings from most recent inspections (Source: PRHTA)

Item	Rating	
	January 23, 2014	May 15, 2017
Deck (Item 58)	5	4
Superstructure (Item 59)	5	5
Substructure (Item 60)	6	5

The latest inspection report made the following observations about the deck: “There is moderate to severe corrosion on the surface, there are several big holes due to corrosion on the decking plates, there is a big section with a detached and skewed corner creating a hole, big patches to repair corroded sections, there are several sections of the deck that have disconnected from its weld points, ...” Referring to the superstructure, the report indicates that “several of the lateral bracing components have light, moderate and sever corrosion and loss of section, some of the upper chord sections have moderate loss of section due to corrosion.” Finally, the report indicates that the “steel columns are in good condition, light corrosion.”

5.2.2 Plans of the Bridge

The oldest set of plans obtained for this bridge were undated and drawn by hand, as shown in Figure 5.23 and Figure 5.24. Also obtained were a plan from 1976 with a 1982 revision (Figure 5.25) and 1987 revision (Figure 5.26), and a set of plans from 2001 (Figure 5.27 to Figure 5.32).

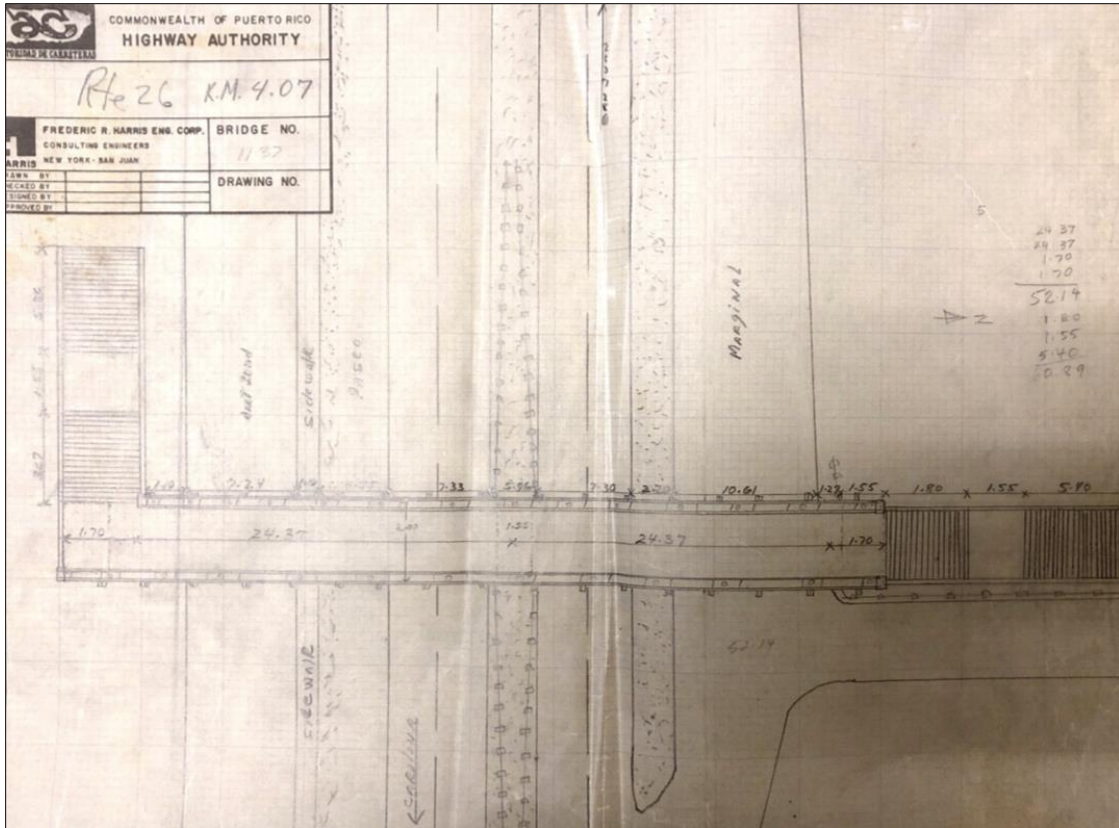


Figure 5.23: PB 1137 undated plan Sheet 1 of 2 (Source: PRHTA)

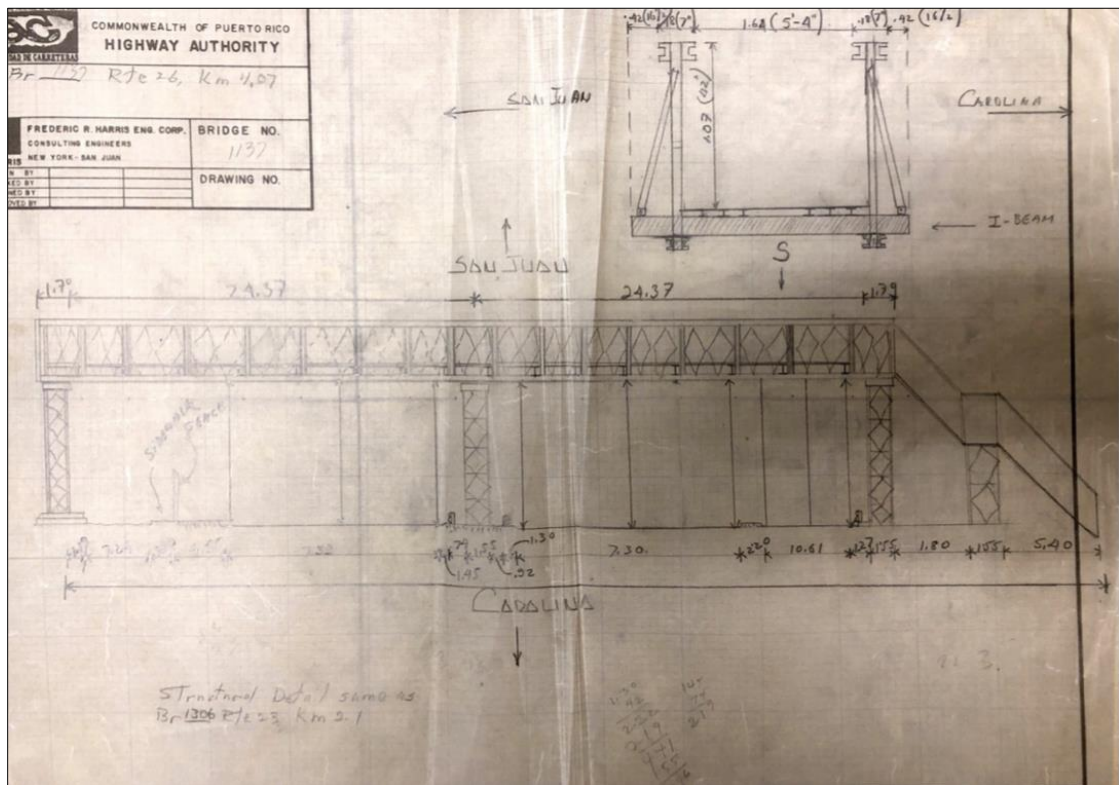


Figure 5.24: PB 1137 undated plan Sheet 1 of 2 (Source: PRHTA)

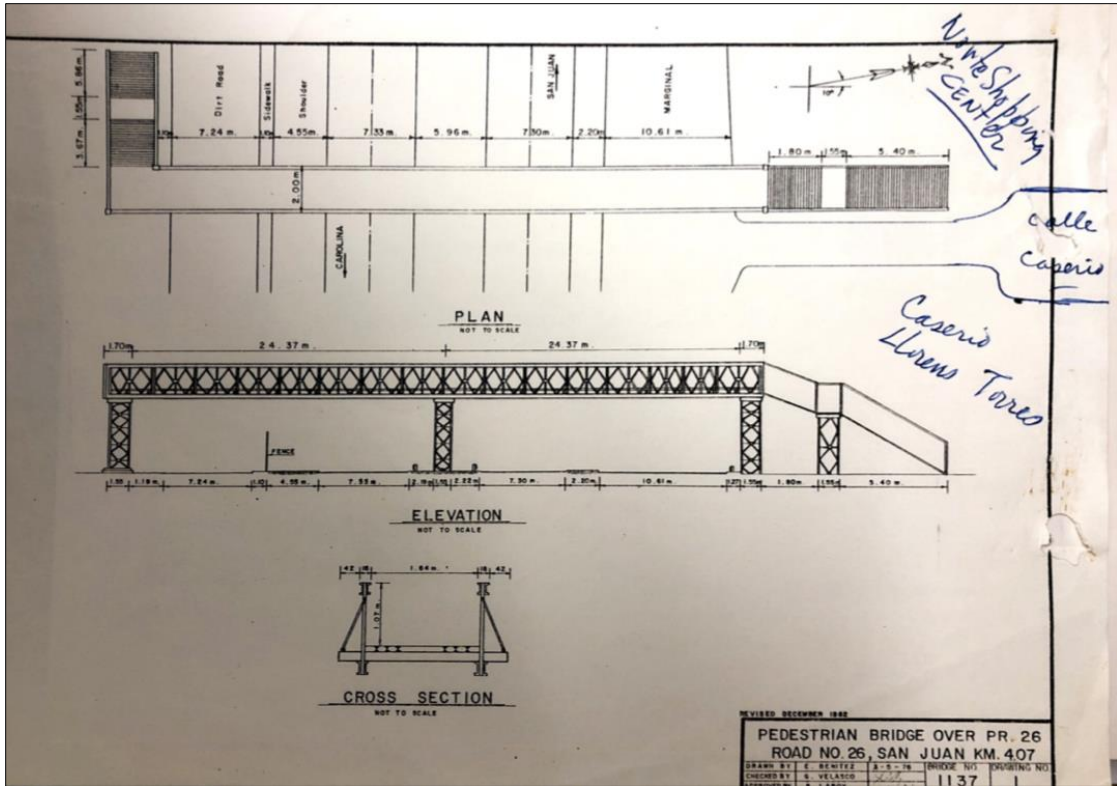


Figure 5.25: PB 1137 plan from 1976 with 1982 revision (Source: PRHTA)

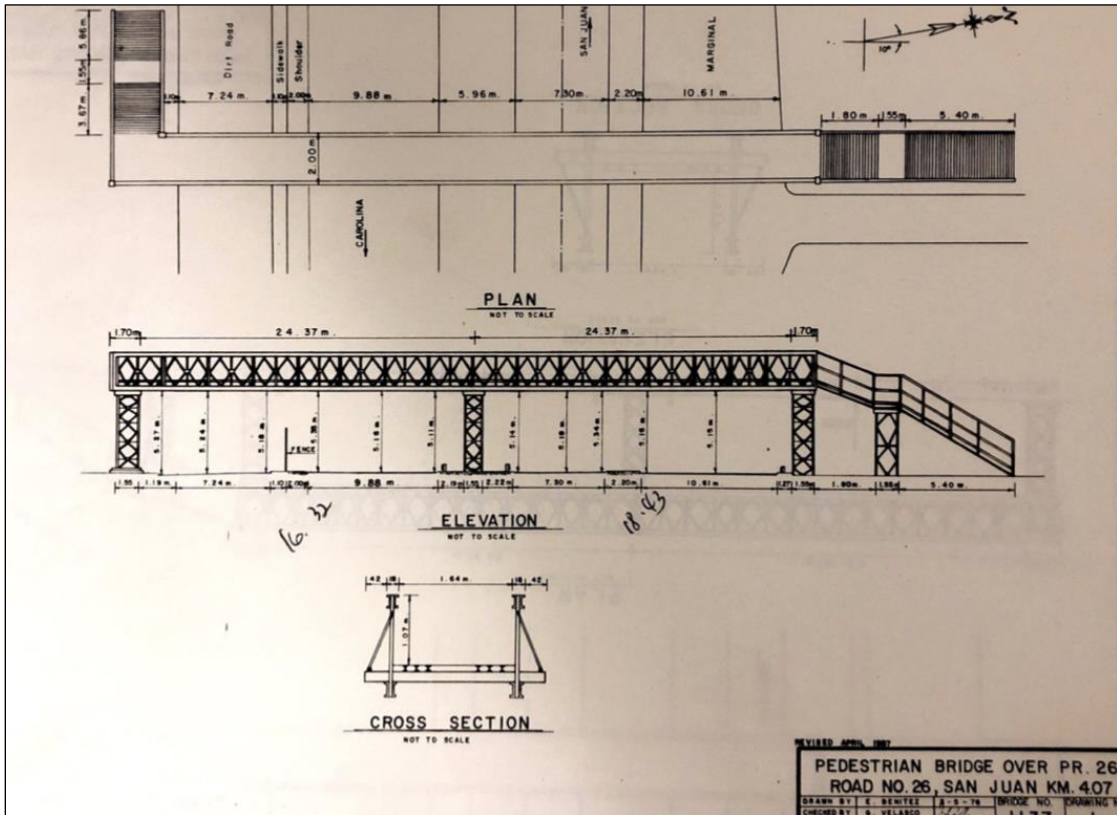


Figure 5.26: PB 1137 plan from 1976 with 1987 revision (Source: PRHTA)

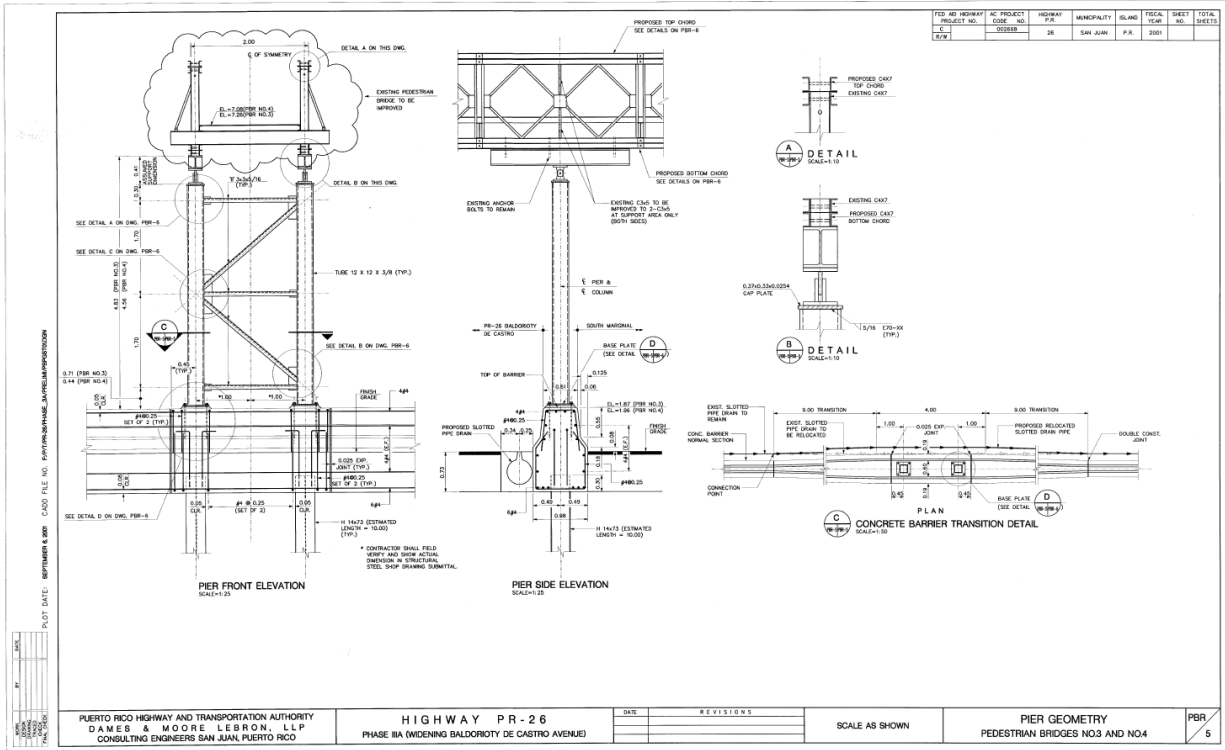


Figure 5.31: PB 1137 plans from 2001 sheet 5 of 6 (Source: PRHTA)

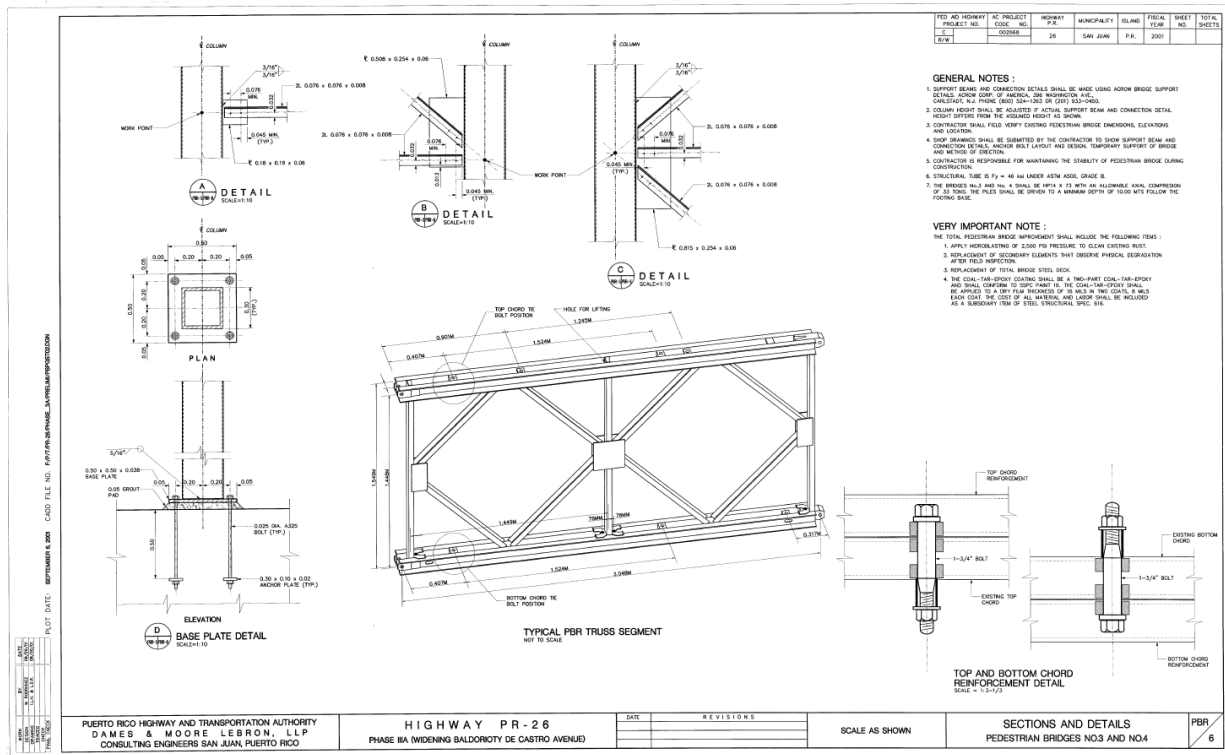


Figure 5.32: PB 1137 plans from 2001 sheet 6 of 6 (Source: PRHTA)

5.2.3 Photos of Inspections by PRHTA

Figure 5.33 presents a set of photos of the bridge taken in 1999. It can be seen in this pictures that, back then, the bridge did not have chain-link fencing, but that it already has at least two traffic signs attached to it.

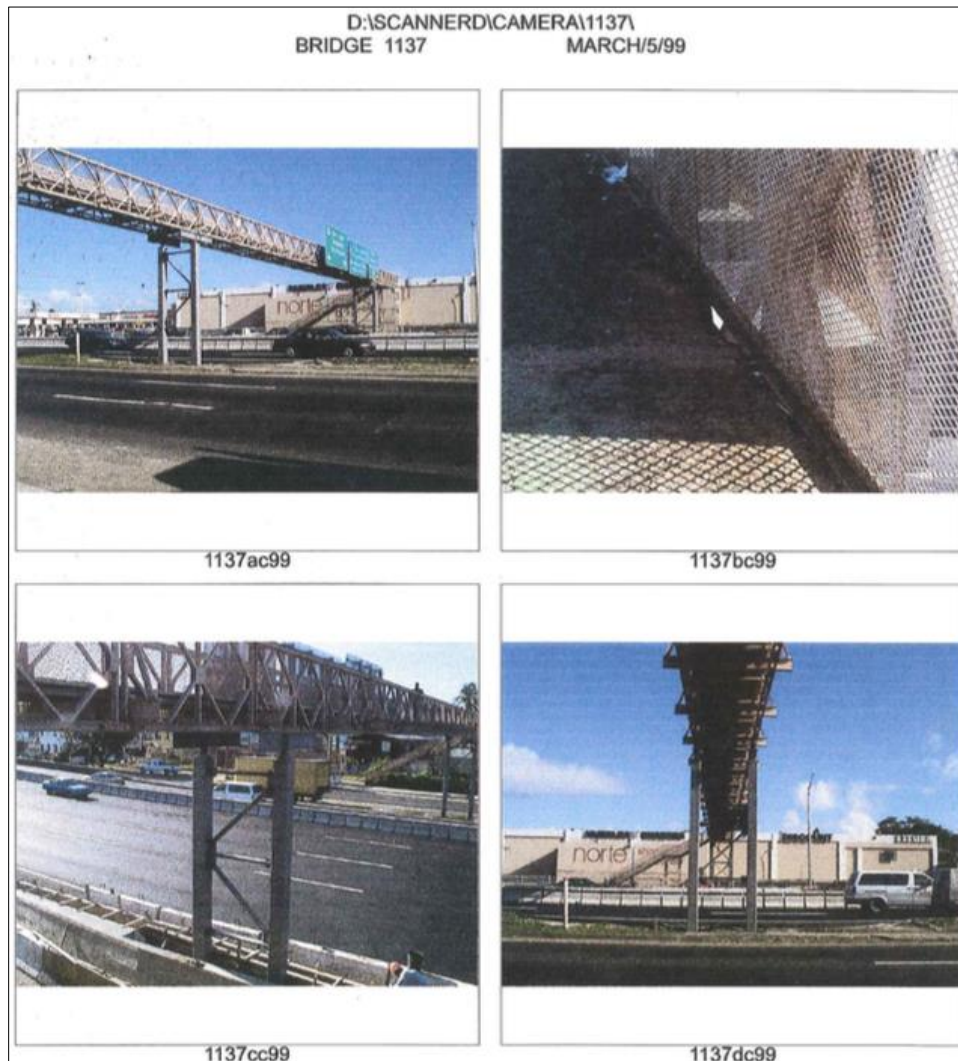


Figure 5.33: Photos taken by PRHTA in 1999 (Source: PRHTA)

5.2.4 Photos Before and After Hurricane Maria

To detect if the bridge suffered any major deformations due to Hurricane Maria, photographs from before and after the hurricane were compared. Examples of these are the comparison that can be made between Figure 5.34 and Figure 5.35, and between Figure 5.36 and Figure 5.37. By the

comparing the photographs, it is appreciated that the bridge experienced lateral deflections in the plastic range.



Figure 5.34: Aerial photo of PB 1137 before Hurricane Maria (April 8, 2016) (Source: Google Earth Pro)

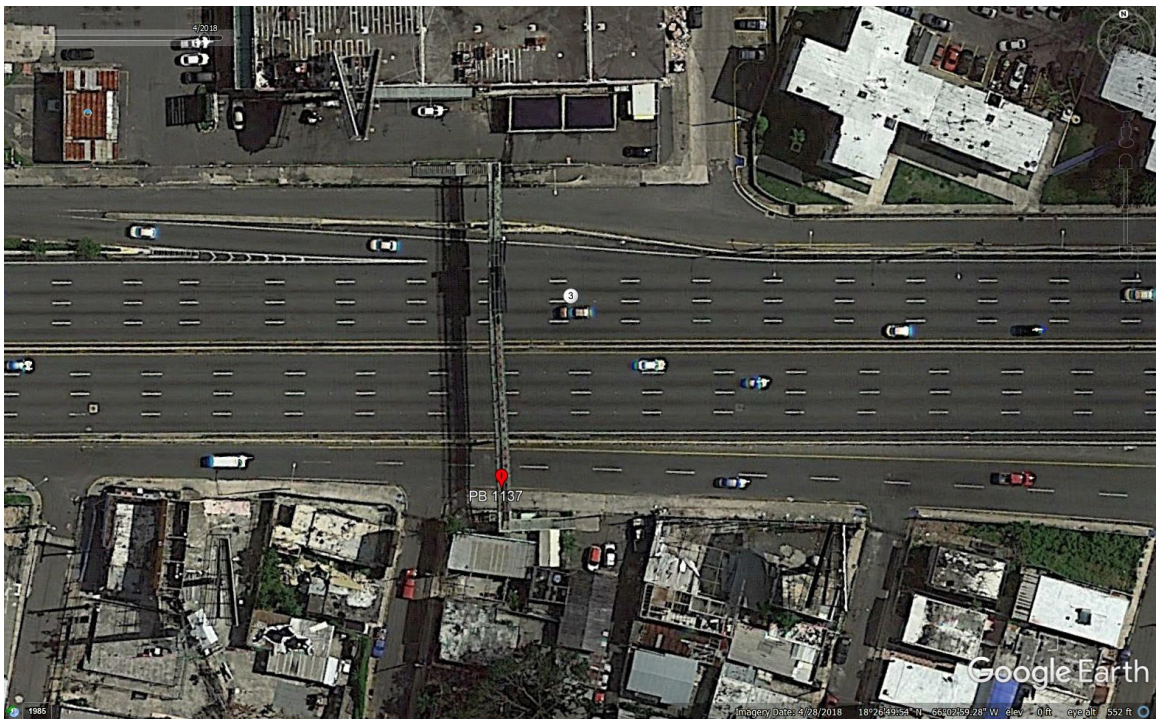


Figure 5.35: Aerial photo of PB 1137 after Hurricane Maria (April 28, 2018) (Source: Google Earth Pro)



Figure 5.36: Underside of PB 1137 before Hurricane Maria (March 5, 1999) (Source: PRHTA)



Figure 5.37: Underside of PB 1137 after Hurricane Maria (November 3, 2018)

5.2.5 Inspection by PUPR

The inspection by PUPR was conducted by Civil Engineering graduate students Nelson Sotelo and Geoffrey Vega, and undergraduate students Adriana Murati-Núñez, Jonathan Hernández and Gustavo Cruz on November 13, 2018. During the inspection, the weather was sunny. The bridge was closed to pedestrians and there were no traffic signs attached to it.

During the visit, it was evident that the bridge had corrosion problems in many components. Corrosion had caused holes in the walkway, as shown in Figure 5.38 and Figure 5.39. Some transversal elements were missing, as shown in Figure 5.40. (Further details about the inspection of this bridge can be found in Volume 2 of this document).

In terms of possible damages caused by Hurricane Maria, it was evident that the bridge had experienced lateral deflections in the plastic range as evidenced by the curvature in the structure that can be appreciated in Figure 5.41 and Figure 5.42. Although the traffic signs that were attached to the bridge had already been removed, the mounting assembly was still present, as shown in Figure 5.43. It was observed that the location of the mounting assembly coincides with the section of the bridge with larger curvature. Therefore, it is theorized that the lateral deflection on the bridge was partly due to the extreme wind loads on the signs during Hurricane Maria. This theory is furthered studied in Volume 2 of this document.

PRDOT deemed Pedestrian Bridge 1137 as unreliable and in danger of collapsing (ElNuevoDia.com, 2019). Therefore, in March 2019, PB 1137 was removed in sections to be later demolished at another site.



Figure 5.38: Some holes in the steel



Figure 5.39: Original piece with all elements



Figure 5.40: Piece without one element



Figure 5.41: Curvature in deck



Figure 5.42: Underside view of lateral deflection



Figure 5.43: Base of the Traffic Sings

5.3 Pedestrian Bridge 1307

Pedestrian Bridge 1307 (shown in Figure 5.44) was a steel structure constructed in 1968. The bridge was located at coordinates 18° 26' 48.40" N and 66° 02' 41.89" W. This Bailey bridge connected the Luis Lloréns Torres public housing complex at the north with the Villa Palmeras sector at the south, as shown in Figure 5.45. The pedestrian bridge crosses over the Román Baldorioty de Castro Expressway, which has a very high vehicular flow (67,300 ADT with the Truck 4% ADT in 2017). The reader may notice that PB 1307 is a structure very similar to PB 1137, almost identical. They both are Bailey bridges constructed the same year, with each structure located down the road from the other. PB 1307 was also removed by PRDOT because it was highly affected by Hurricane Maria. Still, the PUPR team was able to inspect the structure before its removal.



Figure 5.44: Pedestrian Bridge 1307



Figure 5.45: Location of PB 1307 (Source: Google Earth Pro)

5.3.1 PRHTA Inspection Reports

PRHTA provided inspection reports with the following dates, (full reports are presented in Appendix E):

- May 15, 2017
- January 24, 2014
- June 30, 2008
- March 14, 2003
- March 12, 2003
- November 17, 2000

- May 8, 1998
- September 24, 1993
- February 28, 1991
- December 3, 1991
- November 17, 1988
- October 23, 1986
- February 2, 1984
- June 18, 1975
- November 13, 1972

Table 5.3 presents the ratings given to the bridge’s deck, superstructure, and substructure on two most recent inspections. It can be seen that the bridge maintained a rating of Fair (5) for its major items.

Table 5.3: PB 1307 ratings from most recent inspections (Source: PRHTA)

Item	Rating	
	January 24, 2014	May 15, 2017
Deck (Item 58)	5	5
Superstructure (Item 59)	5	5
Substructure (Item 60)	5	5

The latest inspection report made the following observations about the deck: “Steel surface with severe corrosion, slight loss of sections and slight perforations on the sides of the surface of the decking plates. Several decking plates have detached from their weld points, large sections have been repaired, although there are sections in poor conditions, several small holes due to corrosion and sections of the welded decking plates that leave small gaps between them.” Referring to the chain-link fencing, the report indicated: “Severe corrosion of the security mesh, several sections are detached from their points of support. In the stairways there are vertical fencing posts

detached from their welding supports.” superstructure, the report indicates that “” Finally, the report indicates that there is “moderate and severe corrosion on all lower members of the structure. There are perforations with loss of sections. Stairs have severe corrosion and perforations with loss of sections. There are railings that have severe corrosion in the anchors or bases, they have lateral movements.”

5.3.2 Plans of the Bridge

PRHTA provided a plan drafted originally in 1976 and revised in 1993, as shown in Figure 5.46. Another set of drawings from 1991 were obtained and shown on Figure 5.47 and Figure 5.48. The orientation of the stairways differs between the 1976 and 1991 drawings. It appears that, at some point, the stairways were modified. This is confirmed with photographic evidence.

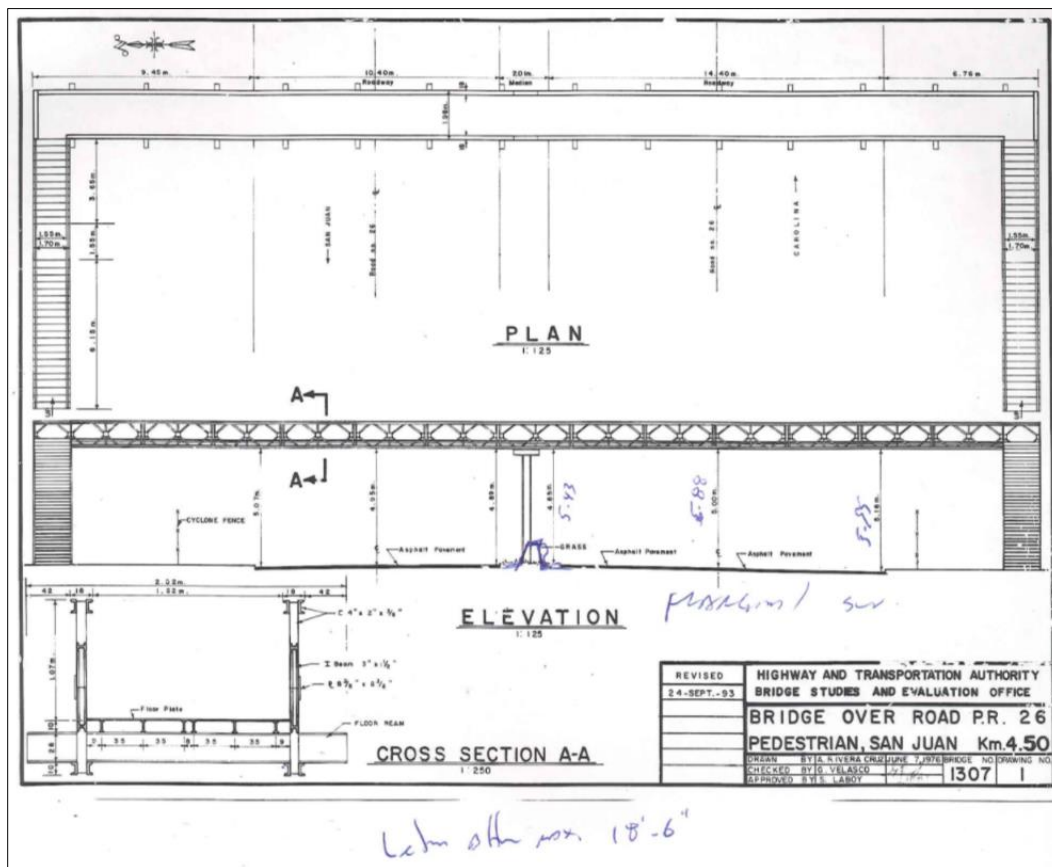


Figure 5.46: PB 1307 plan from 1976 with 1993 revision (Source: PRHTA)

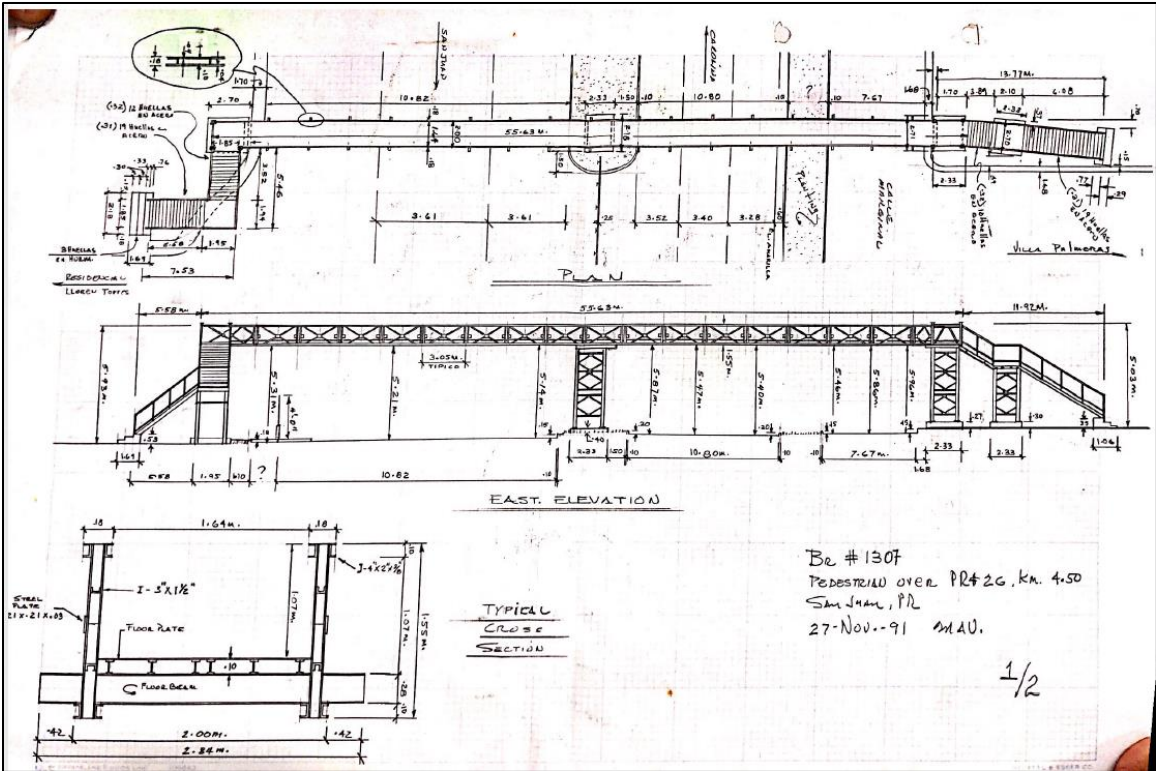


Figure 5.47: PB 1307 plans from 1991 sheet 1 of 2 (Source: PRHTA)

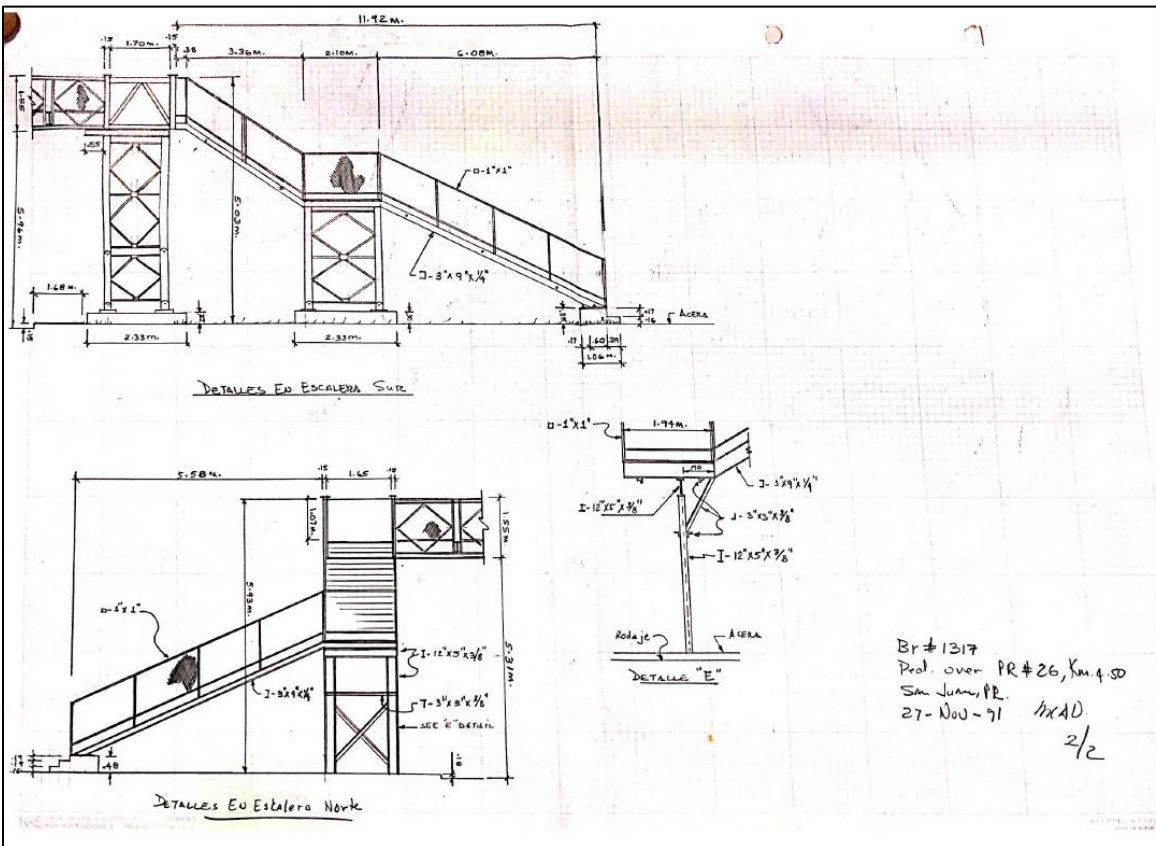


Figure 5.48: PB 1307 plans from 1991 sheet 1 of 2 (Source: PRHTA)

5.3.3 Photos of Inspections by PRHTA

Several photographs were provided by PRHTA. Some undated photographs are shown in Figure 5.49 to Figure 5.52. As explained in the previous section, these photographs show the original orientation of the stairways. It is assumed that these undated photographs are from some point between 1968 and 1990. Figure 5.53 presents a photograph from 1991 which shows the stairways with the modified orientation. Other photographs from 1993 are shown in Figure 5.54 and Figure 5.55. It can be seen in these photos that in 1993, the bridge still did not have chain-link fencing, nor any sign attached to it. A photograph from 2000 (see in Figure 5.56) shows the bridge with traffic signs attached to it, but still with no chain-link fencing.



Figure 5.49: East side view of PB 1307 taken between 1968 and 1990 (Source: PRHTA)



Figure 5.50: Phot of west side view of PB 1307 taken between 1968 and 1990 (Source: PRHTA)



Figure 5.51: Photo of south end of PB 1307 taken between 1968 and 1990 (Source: PRHTA)



Figure 5.52: Underside and north end of PB 1307 taken between 1968 and 1990 (Source: PRHTA)



Figure 5.53: Photo of PB 1307 taken on November 27, 1991 (Source: PRHTA)



Figure 5.54: Photo of deck of PB 1307 taken on September 24, 1993 (Source: PRHTA)



Figure 5.55: Photo of underside of PB 1307 taken on September 24, 1993 (Source: PRHTA)



Figure 5.56: Photo of PB 1307 taken on November 17, 2000 (Source: PRHTA)

5.3.4 Photos Before and After Hurricane Maria

To detect if the bridge suffered any major deformations due to Hurricane Maria, photographs from before and after the hurricane were compared. Examples of these are the comparisons that can be made between Figure 5.57 (taken in 1996) and Figure 5.58 (taken in 2019), and between Figure 5.59 (taken in 1993) and Figure 5.60 (taken in 2019). By the comparing the photographs, it is appreciated that the bridge experienced lateral deflections in the plastic range as evidenced by the curvature exhibited by the deck and the superstructure in the most recent photographs.



Figure 5.57: Deck of PB 1307 before Hurricane Maria (March 18, 1996) (Source: PRHTA)

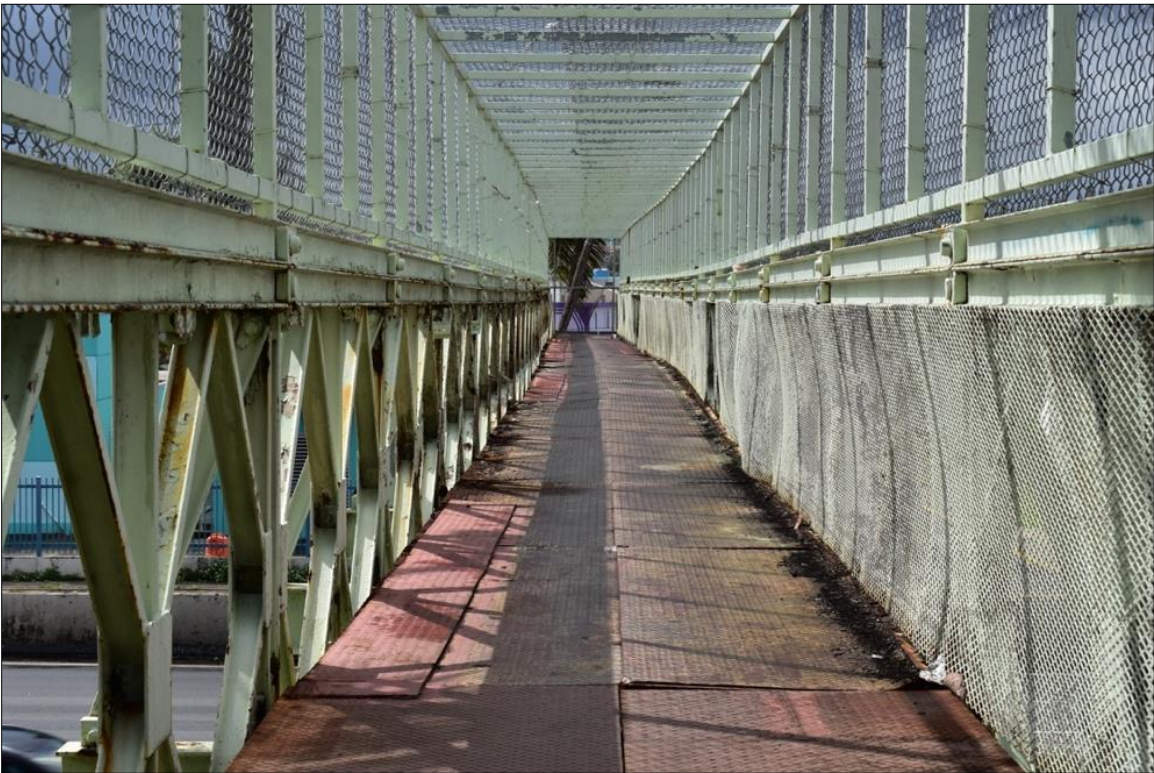


Figure 5.58: Deck of PB 1307 after Hurricane Maria (February 28, 2019)



Figure 5.59: Underside of PB 1307 before Hurricane Maria (September 24, 1993) (Source: PRHTA)



Figure 5.60: Underside of PB 1307 after Hurricane Maria (February 28, 2019)

5.3.5 Inspection by PUPR

The inspection by PUPR was conducted by Civil Engineering undergraduate students Adriana Murati-Núñez and Jonathan Hernández on February 28, 2019. When the inspection started, the weather was sunny, but it later started raining. The bridge was officially closed to pedestrians, but a hole had been cut through the meshing that was supposed to keep the public out. The traffic signs once attached to the bridge were not present, but the mounting assembly still remained. A neighbor from the area assured that he saw as the bridge “moved from side to side” and as the traffic signs were blown away during Hurricane Maria.

During the inspection, the stairs and deck exhibited corrosion, as shown in Figure 5.61 and Figure 5.62, respectively. It can also be observed in Figure 5.62 that sections of the security meshing attached to the guardrail had fallen off. Gaps and holes in the deck were observed as shown in Figure 5.63 and Figure 5.64.

In terms of possible damages caused by Hurricane Maria, PB 1307 exhibited the same damage found on PB 1137, which was a lateral deflection in the plastic range. Although the traffic signs that were attached to the bridge had already been removed or had been blown away, the mounting assembly was still present, as shown in Figure 5.65. It was observed that the location of the mounting assembly coincides with the section of the bridge with larger curvature. Therefore, as with PB 1137, it is theorized that the lateral deflection in PB 1307 was partly due to the extreme wind loads on the signs during Hurricane Maria.

Pedestrian Bridge 1307 was also deemed as unreliable and in danger of collapsing by PRDOT (ElNuevoDia.com, 2019). Therefore, in March 2019, PB 1307 was removed in sections to be later demolished at another site.



Figure 5.61: Corrosion on stairway

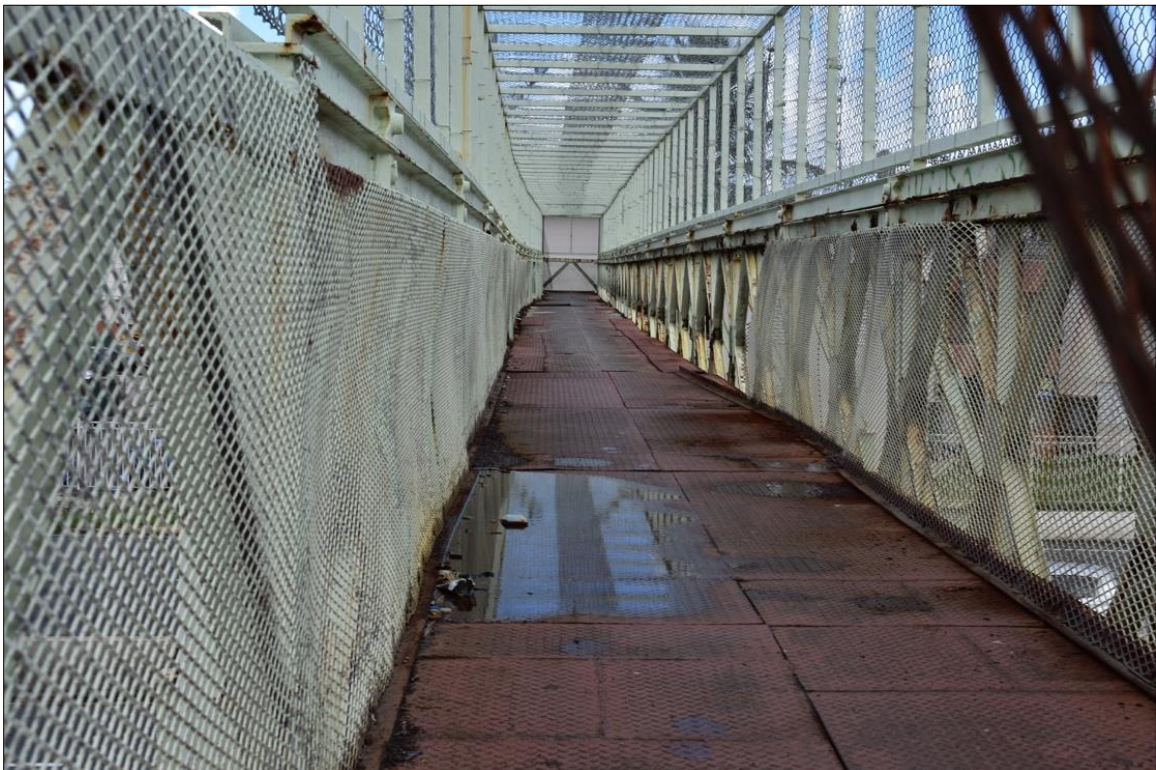


Figure 5.62: Corrosion on deck and section of guardrail without security meshing



Figure 5.63: Detached plates



Figure 5.64: Hole in plates



Figure 5.65: Lateral deflection at mounting assembly location

5.4 Pedestrian Bridge 2336

Pedestrian Bridge 2336 (shown in Figure 5.66) was constructed in 1994. It is a slab type bridge made of reinforced concrete. Access to the bridge is given by a circular ramp at each end. The bridge located at coordinates $18^{\circ} 24' 39''$ N and $66^{\circ} 01' 43''$ W. It connects Matienzo Cintron Urbanization and the San Juan School of Sports in the West with the Ramos Antonini Public Housing complex and the Mall of San Juan in the East (see Figure 5.67). The bridge crosses above Jesus T. Piñero Expressway (PR-17), which by 2005 had an ADT of 89,200 and a Truck ADT 5%.



Figure 5.66: Pedestrian Bridge 2336 (Source: Google Earth Pro)



Figure 5.67: Location of PB 2336 (Source: Google Earth Pro)

5.4.1 PRHTA Inspection Reports

PRHTA only had available on inspection report for PB 2336. This report is date April 16, 2014. The full report is included in Appendix F. This report refers to a previous inspection carried out on December 10, 2008. Table 5.4 presents the ratings given to the bridge’s deck, superstructure,

and substructure on two most recent inspections. It can be seen that the ratings of the main items remain unchanged.

Table 5.4: PB 2236 ratings from most recent inspections (Source: PRHTA)

Item	Rating	
	December 10, 2008	April 16, 2014
Deck (Item 58)	6	6
Superstructure (Item 59)	7	7
Substructure (Item 60)	7	7

The latest inspection report made the following observations: “At the present time (04-16-2014) this structure is being reconstructed, construction of additional span at south side by construction of ramp with access to PR-17.” It has been determined that the bridge was not being reconstructed, as stated in the inspection report, by examining the photos from the same report and historical aerial photography. It is theorized that merely there was a construction of a new ground level ramp to give access to the circular ramp on the east side. It appears that the bridge structure itself was not modified.

The 2014 inspection report also states: “Fine transverse and longitudinal cracks, exposed aggregates and small spallings at wearing surface some holes at cyclone fence of railings.”

5.4.2 Plans of the Bridge

Figure 5.68 and Figure 5.69 reproduce plans of the pedestrian bridge. The plans are dated 1994, but they were copied from the 2014 inspection report.

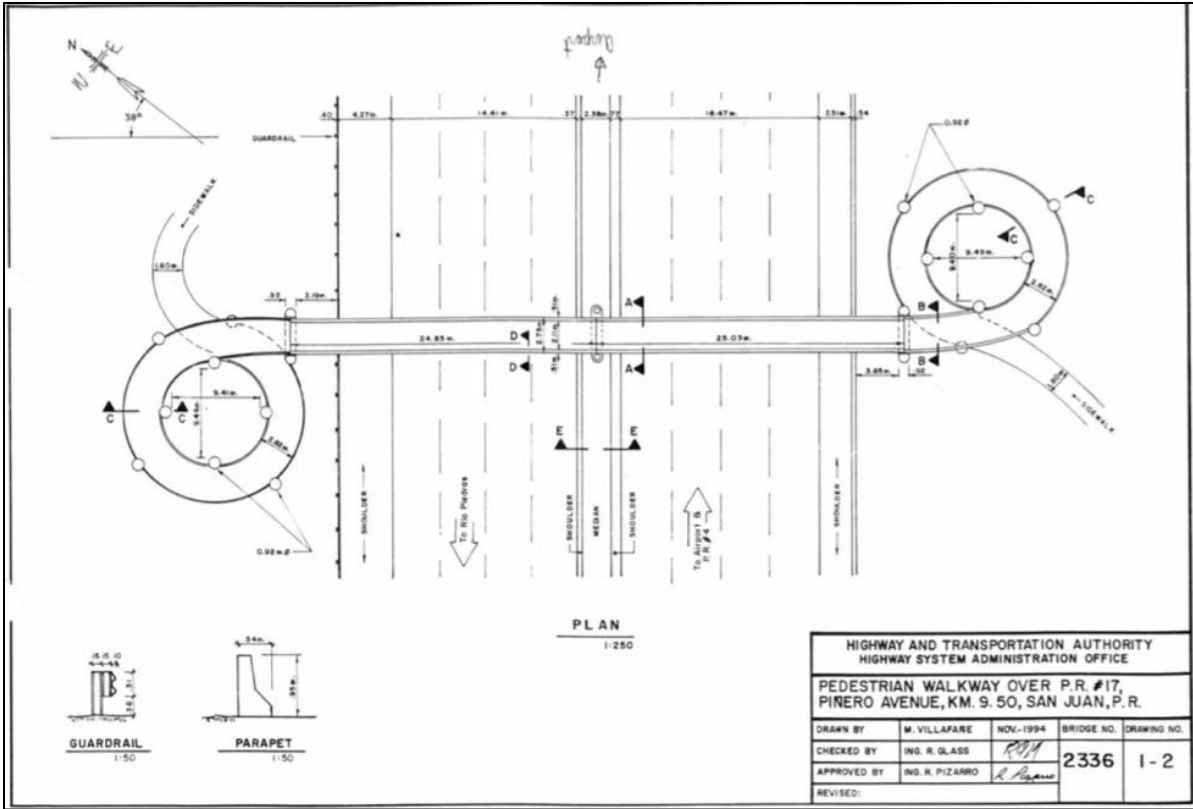


Figure 5.68: PB 2336 plans from 1994 sheet 1 of 2 (Source: PRHTA)

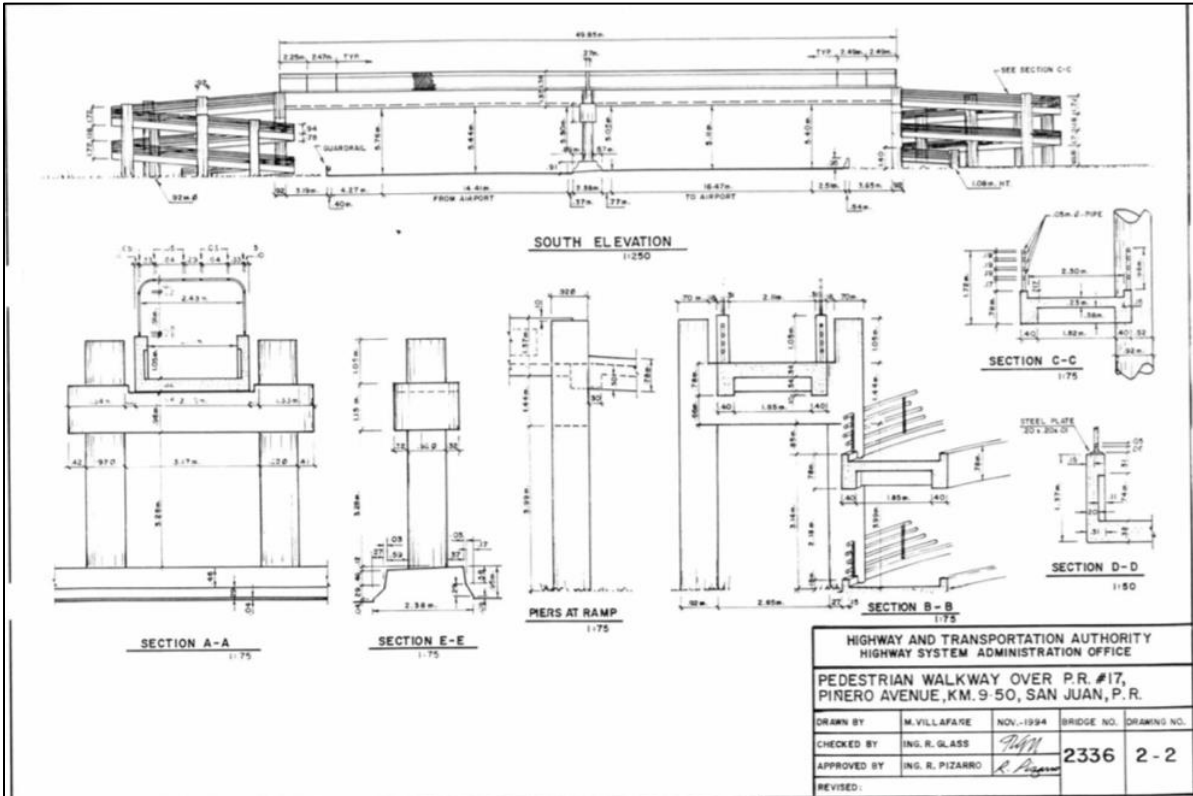


Figure 5.69: PB 2336 plans from 1994 sheet 2 of 2 (Source: PRHTA)

5.4.3 Photos of Inspections by PRHTA

Figure 5.70 presents a set of photographs taken for the inspection report dated April 16, 2014. It can be seen that in 2014, the bridge had traffic signs mounted on it.



Figure 5.70: Photos of the inspection report dated April 14, 2016 (Source: PRHTA)

5.4.4 Photos Before and After Hurricane Maria

To detect if the bridge suffered any major deformations due to Hurricane Maria, photographs from 2014 by PRHTA were compared to pictures taken in 2019. An example of this is the comparison between Figure 5.71 and Figure 5.72. No deformation in the structural elements was detected from the comparison of photographs.



Figure 5.71 Walkway of PB 2336 on April 16, 2014 (Source: PRHTA)



Figure 5.72: Walkway of PB 2336 on April 12, 2019

5.4.5 Inspection by PUPR

The inspection by PUPR was conducted by Civil Engineering undergraduate students Adriana Murati-Núñez and Jonathan Hernández on April 12, 2019. During the inspection, the weather was sunny. The bridge was open to pedestrians.

The bridge itself did not appear to have any problems. In terms of possible damages caused by Hurricane Maria, it was observed that one of the traffic signs mounted on the bridge (the larger sign inside the red rectangle indicated in Figure 5.73) detached at the bottom end from the mounting assembly (see Figure 5.74) It is theorized that the detachment was due to wind induced vibrations that occurred during the hurricane. It was also observed that the smaller airport sign has only one support (see Figure 5.75), and it was oscillating and twisting.



Figure 5.73: Transit Signs in south side of PB 2336 (Source: Google Earth Pro)



Figure 5.74: Transit sign detached from mounting assembly



Figure 5.75: Small airport sign on top of larger sign

6 Undamaged Pedestrian Bridges

As explained in Chapter 4, a total of 21 pedestrian bridges were inspected for this project, out of which four were determined to have suffered some kind of damage due to Hurricane Maria. These four bridges were covered in Chapter 5.

This chapter presents the remaining 17 pedestrian bridges that were inspected but that did not exhibit damages due to the hurricane. Although these bridges did not present damages due to wind loads, they showed problems commonly associated to lack of maintenance.

6.1 Pedestrian Bridge 0626

Pedestrian Bridge 0626 was built in 1959 and is made of reinforced concrete. It has one span and is approximately 91 feet long. The following findings were made during the visual inspection:

- Presence of mold and silt on access ramps (see Figure 6.1)
- Blocked pluvial drains (see Figure 6.2)
- Corrosion on the railings (see Figure 6.3)
- Missing sections of safety meshing (see Figure 6.4)
- Exposed rebars (Figure 6.5)



Figure 6.1: Mold and silt in the access ramps of PB 0626



Figure 6.2: Blocked pluvial drain in PB 0626

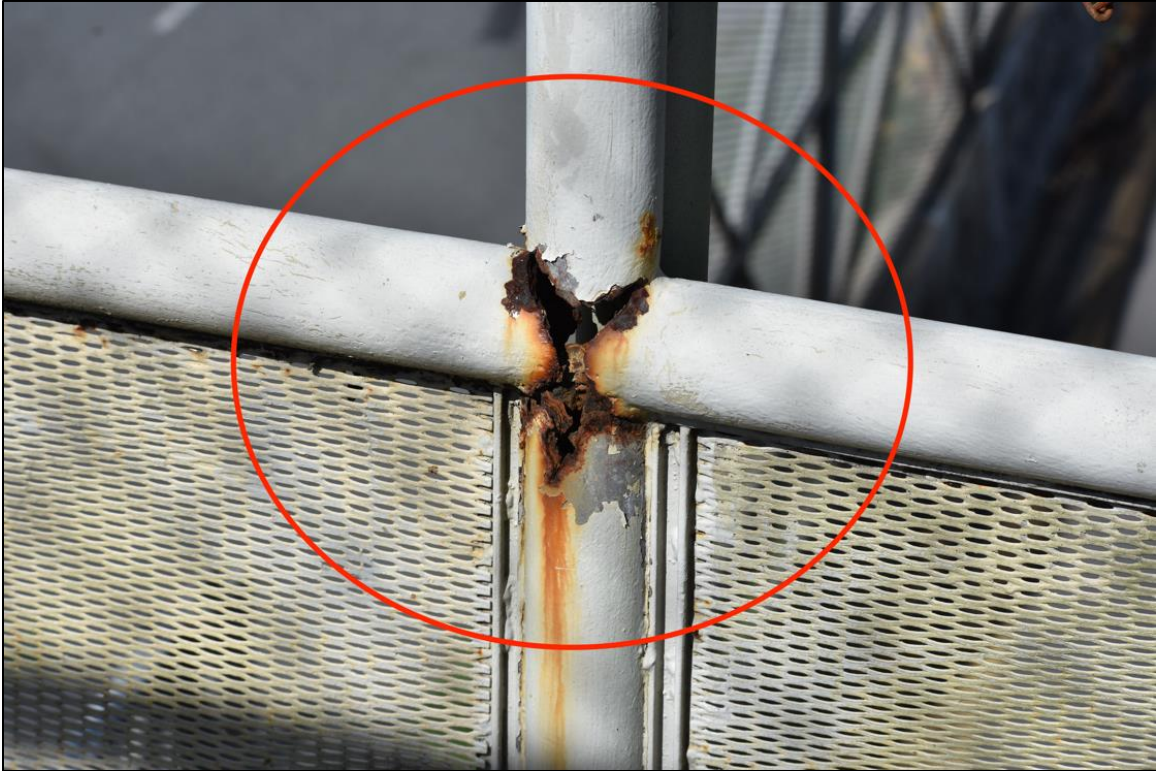


Figure 6.3: Corrosion in the railings of PB 0626



Figure 6.4: Missing section of safety meshing on PB 0626



Figure 6.5: Exposed rebar on PB 0626

6.2 Pedestrian Bridge 0745

Pedestrian Bridge 0745 was built in 1957 and is made of reinforced concrete. It has three spans and is approximately 176 feet long. The following findings were made during the visual inspection:

- Vegetation (see Figure 6.6)
- Blocked pluvial drains (see Figure 6.7)



Figure 6.6: Vegetation in PB 0745



Figure 6.7: Blocked pluvial drains in PB 0745

6.3 Pedestrian Bridge 0748

Pedestrian Bridge 0748 was built in 1958 and is made of reinforced concrete. It has two spans and is approximately 147 feet long. The following findings were made during the visual inspection:

- Broken pluvial drainpipe blocked with vegetation (see Figure 6.8)
- Exposed rebar (see Figure 6.9)



Figure 6.8: Broken drainpipe blocked with vegetation on PB 0748



Figure 6.9: Exposed rebar on PB 0748

6.4 Pedestrian Bridge 1095

Pedestrian Bridge 1095 was built in 1974 and is made of a prestressed concrete beam on top of reinforced concrete columns. It has one span and is approximately 112 feet long. The following findings were made during the visual inspection:

- Presence of mold and silt on the deck (see Figure 6.10)
- Holes in safety fence (see Figure 6.11)



Figure 6.10: Mold and silt on PB 1095



Figure 6.11: Hole in safety fence on PB 1095

6.5 Pedestrian Bridge 1418

Pedestrian Bridge 1418 was built in 1975 and is made of a prestressed concrete beam on top of reinforced concrete columns. It has two spans and is approximately 125 feet long. The following findings were made during the visual inspection

- Mold on the stairways (see Figure 6.12)
- Cracks on concrete (see Figure 6.13)
- Holes in security fence (see Figure 6.14)
- Broken railings (see Figure 6.15)
- Corrosion on railings (see Figure 6.16)
- Exposed steel tendons (see Figure 6.17)



Figure 6.12: Mold on stairways of PB 1418



Figure 6.13: Cracks on PB 1418

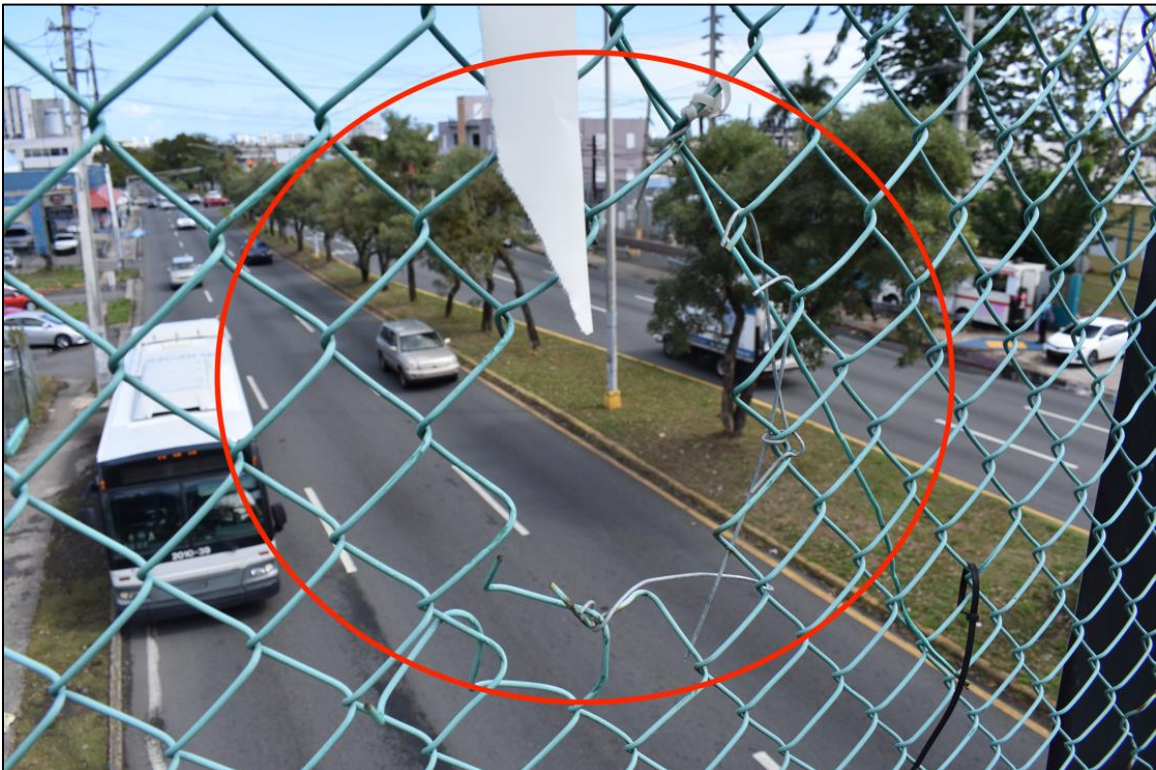


Figure 6.14: Hole in security fence of PB 1418



Figure 6.15: Broken railings on PB 1418



Figure 6.16: Corrosion on railing on PB 1418



Figure 6.17: Exposed steel tendon on PB 1418

6.6 Pedestrian Bridge 1478

Pedestrian Bridge 1478 was built in 1973 and is made of a prestressed concrete beam on top of reinforced concrete columns. It has one span and is approximately 100 feet long. The following findings were made during the visual inspection:

- Exposed rebars (see Figure 6.18)
- Transverse cracks on the deck (see Figure 6.19)



Figure 6.18: Exposed rebar on PB 1478

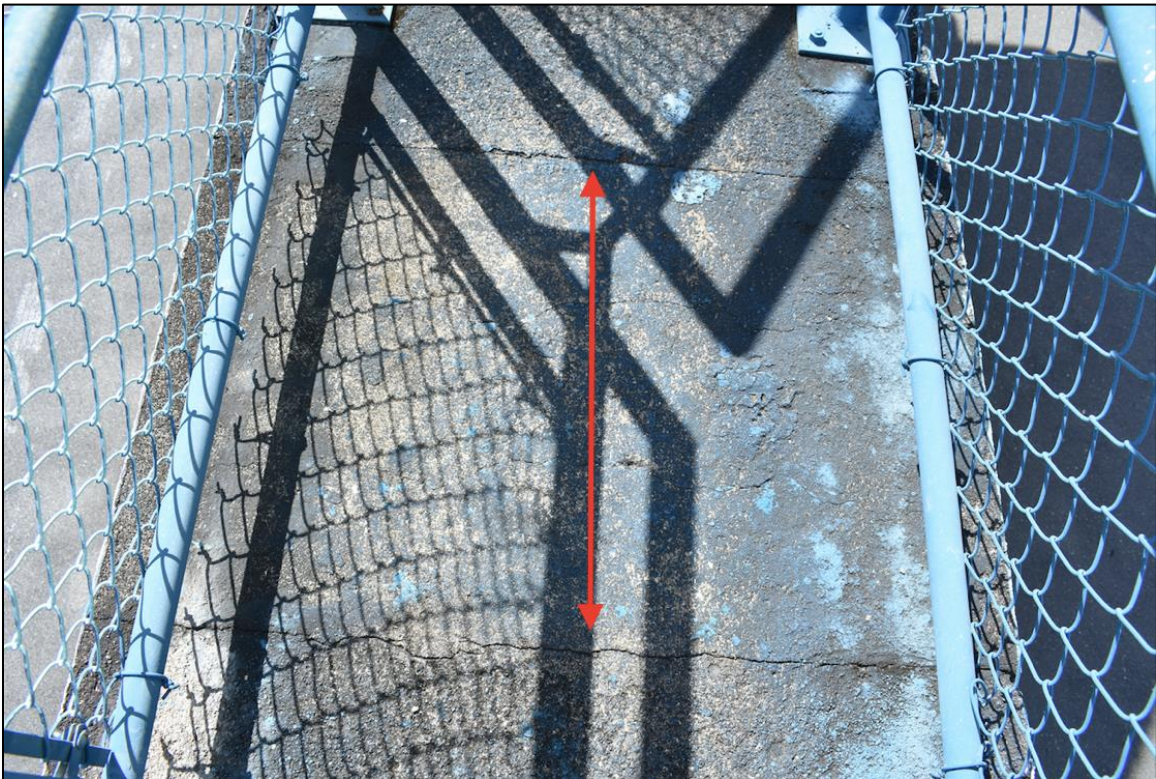


Figure 6.19: Transversal cracks on deck of PB 1478

6.7 Pedestrian Bridge 1513

Pedestrian Bridge 1513 was built in 1975 and is made of prestressed concrete beam on top of reinforced concrete columns. It one span and is approximately 111 feet long. The following finding was made during the visual inspection:

- Mold and silt on one of the stairways (see Figure 6.20)



Figure 6.20: Mold and silt on PB 1513

6.8 Pedestrian Bridge 1736

Pedestrian Bridge 1736 was built in 1981 and is made of a prestressed concrete beam on top of reinforced concrete columns. It has one span and is approximately 113 feet long. The following findings were made during the visual inspection:

- Exposed rebar (see Figure 6.21)
- Mold and silt on stairways (see Figure 6.22)
- Corrosion of security fencing (see Figure 6.23)
- Cracks on stairways (see Figure 6.24)



Figure 6.21: Exposed rebar on PB 1736



Figure 6.22: Mold and silt on stairway of PB 1736



Figure 6.23: Corrosion of security fencing on PB 1736



Figure 6.24: Crack on stairway of PB 1736

6.9 Pedestrian Bridge 1743

Pedestrian Bridge 1743 was built in 1981 and is made of a prestressed concrete beam resting on top of reinforced concrete columns. It has one span and is approximately 111 feet long. The following findings were made during the visual inspection:

- Mold and silt on the stairways (see Figure 6.25)
- Corrosion of security fencing (see Figure 6.26)
- Vegetation growing on one of the piers (see Figure 6.27)



Figure 6.25: Mold and silt on stairway of PB 1743



Figure 6.26: Corrosion of security fencing of PB 1743



Figure 6.27: Vegetation on pier of PB 1743

6.10 Pedestrian Bridge 1774

Pedestrian Bridge 1774 was built in 1980 and is made of steel superstructure and a reinforced concrete substructure. It has three spans and is approximately 186 feet long. No findings were made during the visual inspection.

6.11 Pedestrian Bridge 1926

Pedestrian Bridge 1926 was built in 1989 and is made of reinforced concrete substructure. It has one span and is approximately 65 feet long. The following findings were made during the visual inspection:

- Electrical cables block the pathway of pedestrians (see Figure 6.28 and Figure 6.29)
- Mold and silt on deck and stairways (see Figure 6.29)



Figure 6.28: Electrical cables passing closely over PB 1926



Figure 6.29: Electrical cables blacking pathway and mold and silt on PB 1926

6.12 Pedestrian Bridge 2087

Pedestrian Bridge 2087 was built in 1979 and is made of a prestressed concrete beam resting on reinforced concrete columns. It has one span and is approximately 131 feet long. The following findings were made during the visual inspection:

- Exposed rebar (see Figure 6.30 and Figure 6.31)
- Corrosion of guardrail and security fencing (see Figure 6.32 and Figure 6.33)



Figure 6.30: Exposed rebar on PB 2087



Figure 6.31: Exposed rebars on PB 2087



Figure 6.32: Corrosion of guardrail on PB 2087



Figure 6.33: Corrosion of security fencing on PB 2087

6.13 Pedestrian Bridge 2355

Pedestrian Bridge 2355 was built in 1995 and is made of reinforced concrete. It has two spans and is approximately 129 feet long. The following finding was made during the visual inspection:

- Holes on security fence (see Figure 6.34)



Figure 6.34: Security Fence with holes, Pedestrian Bridge 2355

6.14 Pedestrian Bridge 2566

Pedestrian Bridge 2566 was built in 1998 and is made of prestressed concrete superstructure with a reinforced concrete substructure. It has one spans and is approximately 142 feet long. The following finding was made during the visual inspection:

- Detachments of security mesh from guardrail (see Figure 6.35)
- Vegetation-covered drains (see Figure 6.36)



Figure 6.35: Detachment of security mesh from guardrail on PB 2566



Figure 6.36: Vegetation covered drains on PB 2566

6.15 Pedestrian Bridge 2665

Pedestrian Bridge 2665 was built in 2000 and is made of a prestressed concrete superstructure and a reinforced concrete substructure. It has a main span and is approximately 84 feet long. No findings were made during the visual inspection.

6.16 Pedestrian Bridge 2682

Pedestrian Bridge 2682 was built in 2000 and is made of steel. It has two spans and is approximately 174 feet long. The following findings were made during the visual inspection:

- Damaged to the façade of and signs of corrosion on the central pier (see Figure 6.37)
- Corrosion on the superstructure (see Figure 6.38)
- Detachment of security mesh (see Figure 6.39)



Figure 6.37: Damage to the façade and signs of corrosion on the center pier of PB 2682



Figure 6.38: Corrosion on the superstructure of PB 2682

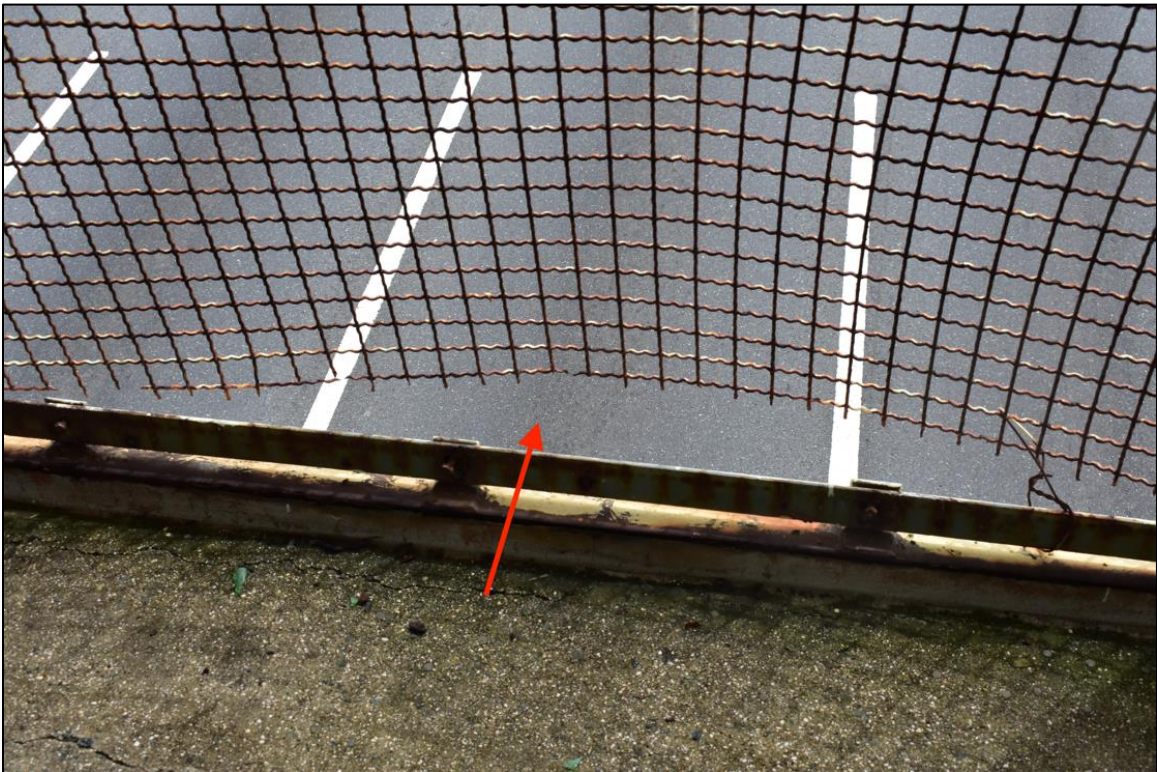


Figure 6.39: Detachment of security mesh on PB 2682

6.17 Pedestrian Bridge 2683

Pedestrian Bridge 2683 was built in 2000 and is made of steel. It has three spans and is approximately 174 feet long. The following findings were made during the visual inspection:

- Corrosion on the superstructure (see Figure 6.40)
- Missing roof panel (see Figure 6.41)
- Detachment of security mesh (see Figure 6.42)



Figure 6.40: Corrosion on the superstructure of PB 2683



Figure 6.41: Missing roof panel on PB 2683



Figure 6.42: Detachment of security mesh on PB 2683

7 Conclusions and Recommendations

Out of the 21 pedestrian bridges inspected for this project, only two experience structural damages. It is apparent that these two bridges were not designed considering that traffic signs were going to be attached to them. Another two bridges were structurally undamaged, but signs attached to them, and their mounting assemblies, exhibited damages that appear to have been caused by the hurricane. The remaining 17 bridges showed no evidence of damages due to the hurricane. Therefore, it is concluded that, in general, pedestrian bridges in the San Juan Metropolitan Area behaved adequately during Hurricane Maria.

The two pedestrian bridges that experienced structural damages and were ultimately removed by PRDOT (PB 1137 and PB 1307) were constructed in 1968, were made of steel trusses, and showed severe deterioration due to corrosion. Prior to Hurricane Maria passing through Puerto Rico, another four bridges made of steel trusses and constructed between 1971 and 1978 had been removed due to deteriorating conditions. The other two pedestrian bridges made of steel trusses and inspected for this project (PB 2682 and PB 2683) were constructed in 2000 and are already showing signs of corrosion, although they did not show any structural damages. When designing a pedestrian bridge, many aspects have to be taken into consideration. It is not suggested here that steel trusses should not be used in Puerto Rico, but that their past performance should be further investigated when this structural system is being considered for new bridges.

Given the findings of this project, the following recommendations are given:

- Attaching signs to pedestrian bridges require the following considerations:
 - If signs are to be attached to existing pedestrian bridges, the structure needs to be adequately reinforced to support additional loads, especially if the

original design of the bridge did not consider signs attached to the structure. This was the case of PB 1137 and PB 1307.

- Systems used to attach signs to pedestrian bridges have to be carefully design, considering support layout and maintenance, in order to avoid damages and disconnection during high wind events, like it happened to PB 0960 and PB 2336.
- After an extreme wind event, preliminary inspections should not only contemplate the main bridge structure, but any flexible elements attached to it, including signs, guardrails, safety meshing, luminaires, and any other element that, if detached, may imply a safety risk to pedestrian or vehicles
- Teams of routine and emergency inspectors should be established by the municipalities in Puerto Rico. Currently, PRHTA Bridge Department does not inspect pedestrian bridges as they have experienced large reduction in personnel due to budget cuts.

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Appendix A Inspections Forms

A.1 Bridge Load Capacity Summary Form Example

BRIDGE LOAD CAPACITY SUMMARY

BRIDGE DATA

Bridge Number 2364

STR Type Main [Item 43] 119

STR Type App [item 44] 119

POSTING DATA

Current Restrictions _____

Item 41 A

Is Posting Needed _____

Proposed Restrictions NONE

Item 70 5

Item 31 5

BASIS FOR ANALYSIS

Design Drawings X

As Built Drawings X

Shop Drawings _____

Field Measurement _____

Coupon Testing _____

OTHER: _____

LIVE LOAD DISTRIBUTION

As Indicated on Plans _____

AASHTO LFD AS PER ART. 6.40 SECTION 6

AASHTO LRFD _____

OTHER: _____

LONGITUDINAL GOVERNING COMPONENT

Main / Approach Span Main

Description BOX CULVERT TOP SLAB (span 1)

Material REINFORCE CONCRETE

Simple / Continuous Span CONTINUOUS

Span Length 12.10 ft

Flexure, Shear or Principal Tension FLEXURE

TRANSVERSE GOVERNING COMPONENT

Main / Approach Span _____

Description _____

Material _____

Deck, Box or Substructure _____

Flexure, Shear or Principal Tension _____

Longitudinal Capacity – Operating Rating (Tons)							
Design Loading							
Inventory Rating Factor <u>17.45</u> Operating Rating Factor <u>29.09</u>							
Vehicle Type	Vehicle GVW	Inventory Rating	Operating Rating	Span No.	Span Length	M/S or PT	LLDF
HS20	36 Ton	75.35	125.61	1/2	12.1 ft.	M	0.10
HS30	54 Ton	74.99	125.01	1/2	12.1 ft.	M	0.10

Responsible Engineer

Signature: _____

Name: Anderson Vasquez

Date: Jun 15, 2011

Lic. No. 15541

Peer Review by

Signature: Rubén González Colón

Name: Rubén González Colón

Date: Jun 15, 2011

Lic. No. 15758


Concrete Box Girder Transverse Capacity – Operating (Tons)	
Design Loading	
Inventory Rating Factor _____ Operating Rating Factor _____	
Single Axle	_____
Tandem Axle (total 2 Axles)	_____

Rating Method	
LFR	<u>X</u>
LRFR	

Engineer comments (Next Page)

A.2 Critical Finding Memorandum Example

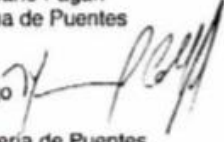
ESTADO LIBRE ASOCIADO DE PUERTO RICO

 **AUTORIDAD DE CARRETERAS Y TRANSPORTACION**

1 de febrero de 2010

Ing. Héctor Laureano Pagán
Gerente Programa de Puentes

Manuel Coll Borgo
Jefe Interino
Oficina de Ingeniería de Puentes




Área de Servicios de Ingeniería
FEB - 3 2010

RECOMENDACIÓN DE ATENCIÓN A PUENTE: 1200
NIVEL DE PRIORIDAD¹: 1 (180 días)

SOCAVACIÓN EN PILASTRA #5 DE PUENTE ESTATAL 1200 EN LA CARRETERA PR 3 KILÓMETRO 10.8 SOBRE EL RÍO GRANDE DE LOIZA EN CAROLINA

En la inspección de rutina del puente número 1200, realizada el 8 de diciembre de 2009, por el personal de inspección de la Oficina, se observó lo siguiente:

- La socavación en la pilastra #5 sigue en aumento, 4.40 metros máximo al tope de la zapata y 3.4 metros máximo de pilotes expuesto.
- Esta condición ha aumentado 10 centímetros desde la última medida en noviembre de 2007.
- Algunos de los pilotes expuestos tienen hasta 50% de pérdida de sección debido a corrosión.

Se recomienda lo siguiente:

- Reparar y proteger la pilastra #5.
- De no repararse la condición existente es necesario realizar cálculos estructurales para determinar si la condición actual es estable, y a que profundidad de socavación la estructura del puente sería inestable.

Para información adicional se puede comunicar al que suscribe a la extensión 2900 ó por correo electrónico a mcoll@act.dtop.gov.pr.

6602/mcb

¹NIVEL DE PRIORIDAD:

- 1- Requiere atención dentro de seis meses (en este caso especificar tiempo)
- 2- Requiere atención dentro un año.
- 3- Requiere atención dentro de tres años.
- 4- Requiere atención dentro de tres años, pero se considera menos serio, en términos de falla estructural, que la prioridad tres.
- 5- Requiere atención dentro de cinco años.

c. José Orozco, Director Interino, Área Servicios Ingeniería

Autoridad de Carreteras y Transportación • Departamento de Transportación y Obras Públicas
PO Box 42007 • San Juan, Puerto Rico 00940-2007 Tel. (787) 721-8787 Fax (787) 727-5456

A.3 Safety Inspection Report Forms (Initial Inspection) Example

Team Leader: Heriberto González
Bridge Inspector: Micky Santiago
Bridge Evaluator: Heriberto González
Assistants: Juan C. Otero Luis. D. Collazo _ _ _
Driver: Alfredo Erazo
Inspection date: Oct.-22-2008
Weather Conditions: Sunny
Amount of Time on Inspection: 2.5 Hours
Equipment: Bus or Van Underwater Ladders Snooper
 Camera Boat Other:

Bridge Number: **2921**
Road on Structure: State Highway Number or Name: PR-751 Km. 0.01
Road Under Structure: N/A Number or Name: _ Km. _
Ident. Plaque: No Num. _

IDENTIFICATION
2. HIGHWAY DISTRICT: 6. Guayama
3. Municipality: Arroyo
4. Place Code: 62192 Bo. Pitahaya
5. Route on Structure: PR-751 Km. 0.01 **Route Under Structure:** _
 Km. _
B: 3 State Highway **C:** 1: Mainline **E:** 0: N.A. **B:** N.A. **C:** 0: None of the Below
E: 0: N.A.

6. Features Intersected: Waterway Pitahaya Creek Km.
 _ **Description:** _

7. Facility Carried: Highway: PR-751 Km. 0.01 **Ramp From:**
 _

8. Structure Number: 2921
9. Location: 2 Kms. @ North of Arroyo Town (17°-59'-45", 66°-03'-37")
10. Minimum Vertical Clearance: **Over:** _ **Under:** _
19. Bypass Length: Structure over river
20. Toll: No N.A.

COMMENTS AND/OR RECOMMENDATIONS:

Inspection by: **Micky Santiago** Revised and Approved by:
Heriberto González
Bridge Inspector **Team Leader**

STRUCTURE DATA

27 (a). Year Built: 1992 (b) Year Reconstructed: _

28 (a). Lanes On Structure (total): 2 (b) Lanes Under Structure (total): _

32. Total Approach Width: [1] 9.40 [2] 8.35

<i>Position</i>	<i>L. Shldr.</i>	<i>L. Rdwy.</i>	<i>Median Shldr.</i>	<i>R. Rdwy.</i>	<i>R. Shldr.</i>
<i>Up Sta. [1]</i>	1.18	3.56		3.56	1.10
<i>Down Sta. [2]</i>	1.18	3.46		3.46	0.25

33. Median: No open

34. Skew: Yes A: 2.43 B: 4.98

35. Flare: No C: _ D: _

36. Traffic Safety Features:

Bridge railings: meets acceptable standards

Transitions: meets acceptable standards

Approach Guardrail: meets acceptable standards

Approach Guardrail Ends: meets acceptable standards

38. Navigation Control: [N] Not waterway

39. Navigation Vertical Clearance: _

40. Navigation Horizontal Clearance: _

42 Type Service

On Structure [1] Highway

Under Structure: [5] Waterway

43. Structure Materials: [1] Concrete Type: [19] Culvert (incl. Frame Culvert)

44. Structure Type Approach Materials: -- Type: --

45. Number of Span (main): 2

46. Number of Span (appr.) Down Station: _ Up Station: _

47. Total Horizontal Clearance (Inventory Route) L: 8.88 R: _

48. Length of Maximum Span: 4.91

49. Structure Length: 9.79 front to front abutments

50. Sidewalk L: _ R: _

51. Bridge Roadway Width: 7.02

52. Deck Width (out to out): 8.88

_ Truss or plate girders (inside face to inside face members): _____ m. _

Other: _____ m.

53. Minimum Vertical Clearance Over Bridge Roadway: Unlimited

54. Minimum Vertical Underclearances: _

55. Minimum Lateral Underclearances on Right: _

56. Minimum Lateral Underclearances on Left: _

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INFRASTRUCTURE DIRECTORY
PRHTA**

58. DECK

58.1 Wearing Surface: Material: Steel *Condition:* 5 Thickness cm.
Deterioration: 10 to 25 % Drainage: Inadequate Ponding: Yes Safety: Yes

58.2 Slab or Plate: Material: Steel *Condition:* 5
Cracking: None
Spalling: Top: No Bottom: No
Scaling: Top: No Bottom: No
Efflorescence: No Exudation: No Rust Stains: No Corrosion: Moderate

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	6		Good	Good	Good	Moderate	--	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: --

58.11 Joints: Condition: N/A

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: None Condition: N Functioning: N/A

Comments:

58.1- Ligera y moderada corrosión en superficie, algunas pequeñas perforaciones por corrosión en las tolas.

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PRHTA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed/Mov. Out of Plumb: No Paint: None

Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.

Condition: N Cracking: No Spalling: None Scaling: None

Crushing: No

59.3 Steel beams: N/A Type: N/A

Condition: N/A Corrosion: N/A Paint: N/A

59.4 Concrete: N/A Type: N/A

Condition: N Cracking: No Spalling: Small Scaling: None

59.5 Truss: Ped. Bailey Condition: 5 Paint: Fair Corrosion: Moderate Members: N/A

59.6 Drainage: Type: N/A Condition: N/A Functioning: N/A

59.7 Hinges: Condition: N/A Functioning: N/A Movement: N/A

59.8 Deflection: N/A

59.9 Vibrations: N/A

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.2.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.2.3	Columns	Steel	6	--	--	--	Moderate	--	No	No	No
	60.2.4	Footing	Concrete	7	F	Small	--	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.5- Varios de los componentes laterales de los "bracing" tienen ligera, moderada corrosión y pérdida de sección, algunas secciones en el "upper chord" tienen moderada pérdida de sección por corrosión.

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: None Scaling: None Uneven: No
Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No
Excessive deformation: No
Safety: Hazardous: No
Drainage: Inadequate: No
Movement: Pavement-approach slab: No
Embankment: Condition: N Functioning: Yes Erosion: No
Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: Jan 23, 2014

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:

Underwater:

Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Steel Plate (includes orthotropic)

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Other

Type Membrane: None

Deck protection: None

111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

AUXILIARY ITEMS

Signs: Type: Route Orientation Material: Alum. Condition: 5

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

58. DECK

58.1 Wearing Surface: Material: Steel *Condition:* 5 Thickness cm.
Deterioration: 10 to 25 % Drainage: Inadequate Ponding: Yes Safety: Yes

58.2 Slab or Plate: Material: Steel *Condition:* 5
Cracking: None
Spalling: Top: No Bottom: No
Scaling: Top: No Bottom: No
Efflorescence: No Exudation: No Rust Stains: No Corrosion: Moderate

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	6		Good	Good	Good	Moderate	--	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: --

58.11 Joints: Condition: N/A

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: None Condition: N Functioning: N/A

Comments:

58.1- Ligera y moderada corrosión en superficie, algunas pequeñas perforaciones por corrosión en las tolas.

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed/Mov. Out of Plumb: No Paint: None

Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.

Condition: N Cracking: No Spalling: None Scaling: None

Crushing: No

59.3 Steel beams: N/A Type: N/A

Condition: N/A Corrosion: N/A Paint: N/A

59.4 Concrete: N/A Type: N/A

Condition: N Cracking: No Spalling: Small Scaling: None

59.5 Truss: Ped. Bailey Condition: 5 Paint: Fair Corrosion: Moderate Members: N/A

59.6 Drainage: Type: N/A Condition: N/A Functioning: N/A

59.7 Hinges: Condition: N/A Functioning: N/A Movement: N/A

59.8 Deflection: N/A

59.9 Vibrations: N/A

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.2.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.2.3	Columns	Steel	6	--	--	--	Moderate	--	No	No	No
	60.2.4	Footing	Concrete	7	F	Small	--	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.5- Varios de los componentes laterales de los "bracing" tienen ligera, moderada corrosión y pérdida de sección, algunas secciones en el "upper chord" tienen moderada pérdida de sección por corrosión.

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: None Scaling: None Uneven: No
Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No
Excessive deformation: No
Safety: Hazardous: No
Drainage: Inadequate: No
Movement: Pavement-approach slab: No
Embankment: Condition: N Functioning: Yes Erosion: No
Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: Jan 23, 2014

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:
Underwater:
Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Steel Plate (includes orthotropic)

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Other
Type Membrane: None
Deck protection: None

111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

AUXILIARY ITEMS

Signs: Type: Route Orientation Material: Alum. Condition: 5

A.5 Standards for Photographs of Existing Bridges

Abril 2005

Por:

Manuel Coll Borgo

Estandarización de las Fotos de los Puentes Existentes

A. Alcance y Objetivos

Evaluar la condición y la toma de decisiones sobre un puente existente es una tarea muy usual y típica realizada por los Ingenieros tanto de la Oficina de Ingeniería de Puentes como de otras oficinas de la Agencia. Una de las partes más útiles del archivo de puentes son las fotografías del puente tomadas en cada inspección que este archivo incluye. Antes de que la tecnología digital estuviera disponible, las fotos que se podían tomar en un puente eran limitadas debido al costo y espacio de almacenamiento disponibles, por lo tanto, la parte visual del puente en el archivo era limitada. Con las cámaras de tecnología digital y la computadora estas barreras se han eliminado.

El objetivo de este documento es ayudar a establecer un patrón estándar de fotografía para los puentes inspeccionados de tal manera que se pueda observar todo el puente mediante las fotos tomadas durante la inspección. Esto, aprovechando lo económico y relativamente sencillo que es tomar y archivar fotos con las cámaras digitales.

B. Procedimiento

El procedimiento descrito a continuación es uno general que se debe aplicar para cada puente individualmente. Básicamente el patrón de fotos va a consistir de dos partes. Primero, se tomarán las fotos generales de tal manera que se puedan ver todas las partes del puente

con o sin daños y luego se tomaran fotos más específicas de las áreas donde se observa deterioro y otras partes importantes del puente. El procedimiento se puede describir en tres pasos generales descritos a continuación:

1. Se debe documentar en una tabla el número de la foto y la parte del puente que se está fotografiando. Debe haber espacio para poner comentarios, como por ejemplo, dónde se ubicó el fotógrafo para tomar la foto de tal manera que se tome la foto igual en la próxima inspección. Esta debe ir acompañada de un dibujo de planta donde indique donde se tomó cada foto. La tabla básicamente hay que hacerla la primera vez que se estandaricen las fotografías del puente, pero a medida que aparezcan partes deterioradas en el puente y se tomen fotos de esas áreas éstas se deben añadir a este documento. El documento se debe colocar en el archivo y también se debe mantener una copia digital de este documento en el mismo directorio donde se almacenen las fotos.
2. **Fotos Generales del Puente** - Se debe incluir por lo menos una foto de cada parte importante del puente. Si se necesitan más fotos para cubrir una parte del puente, esto se debe documentar en la tabla de la parte (1). Las partes principales que se deben incluir típicamente para cada tramo (span) del puente incluyen, entre otras cosas, lo siguiente:
 - a. Placa de identificación del puente si la tiene.
 - b. Letreros de restricciones de pesos, geometría y pórticos para limitar altura si los incluye el puente
 - c. Superficie de Rodaje o vista superior del tramo. Se deben incluir las losas de entrada del puente (Approach Slabs).
 - d. Vista Lateral del Tramo (ambos lados), por ejemplo, cuando el puente atraviesa un canal, vista desde aguas arriba y vista desde aguas abajo.
 - e. Vista inferior del tramo con énfasis en la losa y las vigas. Ésta es la más difícil de tomar y por lo general tomará más de una fotografía. Hay que tomar en cuenta que también es la más importante y es bien importante documentar todas las fotos bien para que se tomen iguales en la próxima visita.
 - f. Fotos de las pilastras y sus fundaciones si se ven. Se deben tomar por lo menos dos fotos por pilastra, una de cada lado de la pilastra para poder observar la condición de ésta en su totalidad.
 - g. Fotos de los estribos y asientos (bearings) del puente. En puentes típicos, esto tomaría al menos tres fotos. Una para cada lado incluyendo vista de la pared lateral (wing wall) y por lo menos una foto donde se vea el estribo de frente y los asientos.
3. **Fotos Particulares del Puente** - Si el puente presenta zonas de deterioro y tiene detalles importantes que se deben retratar, estas fotos se deben enumerar y documentar en la tabla de la parte (1) luego de las fotos generales del puente. De esta manera los comentarios del informe de inspección pueden hacer referencia a estas fotos. La tabla debe

documentar bien la localización de la foto para poder identificar bien en otras inspecciones el lugar, sobre todo, por si se hacen reparaciones entre inspecciones.

C. Comentarios Adicionales

Las fotos (c), (d) y (e) de las fotografías generales (parte (2)) deben tomarse para cada tramo (span), pero se debe evaluar para cada puente particular si una foto puede incluir más de un tramo con suficiente claridad. Por ejemplo, en puentes pequeños de dos tramos, a lo mejor una sola foto lateral puede incluir la vista de los dos tramos del puente y de los dos estribos, por lo que se podrían reducir cuatro fotos de las descritas arriba en una sola. De hacerlo así, esto se debe documentar en la descripción de la foto en la tabla de la parte (1).

Por otra parte, debido a la geometría y localización de cada puente particular no siempre será posible tomar una foto de las vistas que se describen en la parte (2). En ese caso, se deben tomar una o más fotos en otro ángulo pero de tal manera que se pueda ver lo descrito. No se debe dejar de tomar una foto de una parte del puente a menos que ésta esté debajo del agua.

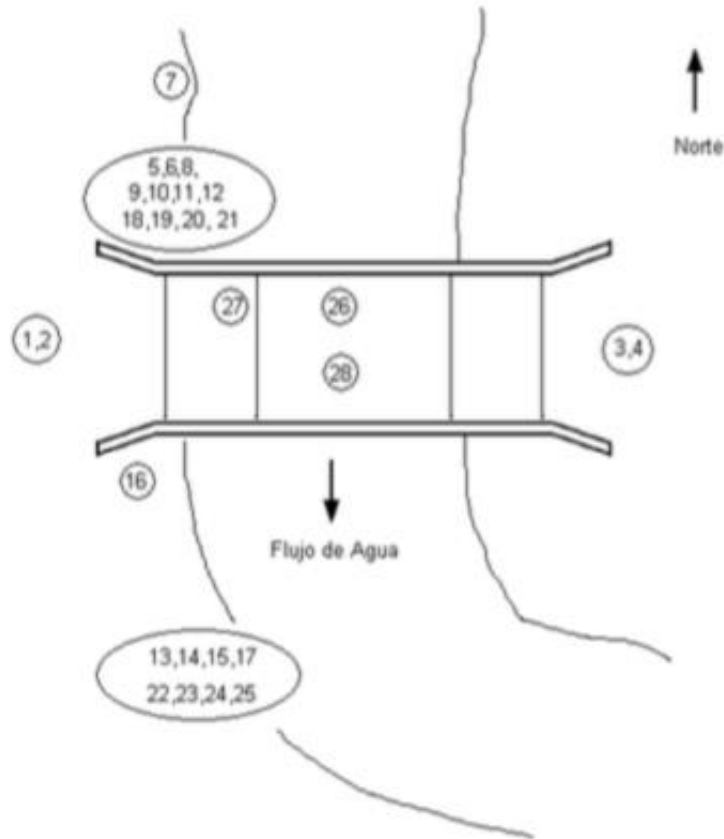
D. Ejemplo: Puente No. 1496 en Guayanilla

Tabla de Descripción de Fotos

Foto	Descripción	Comentarios
Fotos Generales del Puente		
# 1	Losa de entrada lado oeste	
# 2	Superficie de rodaje lado oeste	
# 3	Losa de entrada lado este	
# 4	Superficie de rodaje lado este	
# 5	Tramo oeste: Vista lateral desde el norte	
# 6	Tramo este: Vista lateral desde el norte	
# 7	Vista aguas arriba del puente y el canal	Aguas arriba del canal del lado norte
# 8	Muro lateral norte del estribo oeste	
# 9	Muro lateral norte del estribo este	
# 10	Estribo oeste: vista frontal	
# 11	Estribo este: vista frontal	
# 12	Pilastra: vista desde lado oeste	
# 13	Pilastra: Vista desde lado este	Se tomó la foto desde el lado oeste del río, pero en ángulo, de tal manera que se observe el lado que no se ve en la foto #13
# 14	Tramo oeste: Vista lateral desde el sur	
# 15	Tramo este: Vista lateral desde el sur	
# 16	Muro lateral sur del estribo oeste	
# 17	Muro lateral sur del estribo este	
# 18	Losa tramo oeste: Vista por debajo	
# 19	Losa tramo este: Vista por debajo	
# 20	Gaviones estribo oeste	
# 21	Gaviones estribo este	
# 22	Detalle de Zapata expuesta	
# 23	Muro caído en estribo oeste	Esta condición existía antes de la

Foto	Descripción	Comentarios
Fotos Generales del Puente		
		reparación de puente con Gaviones.
# 24	Socavación en Gaviones del estribo este	
# 25	Grieta en muro lateral sur del estribo este	
# 26	Abrasión de la superficie de rodaje Tiro central de losa	La losa se tiró en tres paños.
# 27	Abrasión de la superficie de rodaje Tiro de losa oeste	
# 28	Grietas en tiro central de losa	En los otros tiros también hay grietas, pero menos marcadas.

Diagrama de Localización desde donde se tomo la Fotografía



Nota: Para el diagrama se puede usar el boceto ("as built drawing") del Puente.



Foto #1



Foto #2



Foto #3



Foto #4

A.6 Standard Element Numbering Procedure

Elements in Longitudinal Direction: Elements that run in the direction of traffic such as Main Girders, Beams or stringers shall be numbered from left to right when pointing to the direction of traffic or when pointing to the direction of the increasing kilometer signs when traffic goes both ways.

Elements in Transverse Direction: Elements that run transverse to the direction of traffic such as Abutments Piers or Floor Beams shall be numbered in the direction of traffic or pointing to the direction of the increasing kilometer signs when traffic goes both ways. For example pier #1 will be the first one when entering the bridge, for bridges with one direction of traffic, and in the case of two way traffic pier #1 will have the lesser kilometer point.

A.7 Quality Assurance / Quality Control Forms

INSPECTION REPORT SUMMARY & QC SHEET						
BRIDGE:						
TEAM LEADER:						
INSP. DATE:						
1. Inspection Type and Dates:						
NBI	Type	Performed? (Yes / No / NA)	Freq (MONTHS)	Previous Insp. DATE (MONTH/YEAR)	Next Insp. DATE (MONTH/YEAR)	
ITEM 90	Routine Inspection					
ITEM 93 A	FC Inspection					
ITEM 93 B	Underwater Insp.					
ITEM 93 C	Other:					
2. NBI Condition Rating Summary:						
	Item 58	Item 59	Item 60	Item 61	Item 62	Item 113
Previous Inspection						
Current Inspection						
Other Checks: (Y, N, NA)			Review Comments:			
<ul style="list-style-type: none"> <input type="checkbox"/> Scour Critical (items 113 & 60) <input type="checkbox"/> AASHTO Core's & NBI CD consistent <input type="checkbox"/> Smart Flags (scour, steel plate, fire damage, etc) <input type="checkbox"/> Channel Profile/Clearance Table <input type="checkbox"/> FC & Underwater Members Tables <input type="checkbox"/> Asphalt Overlay Thickness <input type="checkbox"/> Drawings <input type="checkbox"/> Photos <input type="checkbox"/> Critical Finding <input type="checkbox"/> Inspector & Team Leader Signature 						
Reviewer: _____						
Safety Eng.: _____						

**INSPECTION REPORT SUMMARY SHEET
QUALITY ASSURANCE**

**BRIDGE:
TEAM LEADER:
INSP. DATE:**

1. Inspection Type and Dates:

NBI	Type	Performed? (Yes / No / NA)	Freq (MONTHS)	Previous Insp. DATE (MONTH/YEAR)	Next Insp. DATE (MONTH/YEAR)
ITEM 90	Routine Inspection				
ITEM 93 A	FC Inspection				
ITEM 93 B	Underwater Insp.				
ITEM 93 C	Other:				

2. NBI Condition Rating Summary:

	Item 58	Item 59	Item 60	Item 61	Item 62	Item 113
Previous Inspection						
Current Inspection Team Leader						
Current Inspection Safety Engineer						

Other Checks: (Y, N, NA)

Review Comments:

- Scour Critical (items 113 & 60)
- AASHTO Core's & NBI CD consistent
- Smart Flags (scour, steel plate, fire damage, etc)
- Channel Profile/Clearance Table
- FC & Underwater Members Tables
- Asphalt Overlay Thickness
- Drawings
- Photos
- Critical Finding
- Inspector & Team Leader Signature

Team Leader: _____

Safety Eng.: _____

Bridge No.

Date:

Rating Engineer/Consultant:

LOAD RATING REPORT

QUALITY CONTROL FORM

Rating Report Item	Yes/No, NA or Checked
Does report includes data used for rating (Drawings/Field Measurements/NDT's)	
Are Dead load calculations presented	
Are Live load distribution factors computed/presented	
Is Transverse section data and reinforcement presented	
Report explains reasons to (or not to) consider deterioration.	
Report explains reasons to rate (or not to) Substructure	
Report shows spans and explains boundary conditions (support types, continuity)	
Design Load / Legal Loads are clearly presented	
Verify Input data of model for consistency	
Report Includes Load Rating Results:	
o At inventory level	
o At Operating level	
o Legal loads	
Verify output data is consistent with report results and with the rating summary sheet report.	
Verify items 63 to 66 be reported from summary sheet.	
Verify items 41 and 70 are correctly coded in summary sheet.	

Comments:

- Write comments to rating engineer here.
- Comment example: Section data used is missing in rating report.

Reviewer Name:

Reviewer Signature:

Bridge No.

Date:

Rating Engineer/Consultant:

LOAD RATING REPORT

QUALITY ASURANCE FORM

Rating Report Item	Yes/No, Comments
Does report includes data used for rating (Drawings/Field Measurements/NDT's)	
Are Dead load calculations presented	
Are Live load distribution factors computed/presented	
Is Transverse section data and reinforcement presented	
Report explains reasons to (or not to) consider deterioration.	
Report explains reasons to rate (or not to) Substructure	
Report shows spans and explains boundary conditions (support types, continuity)	
Design Load / Legal Loads are clearly presented	
Verify Input data of model for consistency	
Report Includes Load Rating Results:	
o At inventory level	
o At Operating level	
o Legal loads	
Verify output data is consistent with report results and with the rating summary sheet report.	
Verify items 63 to 66 be reported from summary sheet.	
Verify items 41 and 70 are correctly coded in summary sheet.	

Comments:

- Write comments to rating engineer here.
- Comment example: Section data used is missing in rating report.

Reviewer Name:

Reviewer Signature:

Consultant:

Bridge No.

Date:

SCOUR EVALUATION PHASE 1

QUALITY CONTROL FORM

Scour Evaluation Report Item	Yes/No, N/A or Checked
Does report includes project description and location?	
Does report includes summary of significant findings?	
Does report includes conclusions and recommendations?	
Does report includes photos showing exiting conditions?	
Does report includes data used for scour evaluation (Drawings/Field Measurements/NDT's)?	
Does report includes sources of data collection presented (i.e. H-H report, scour report, soil survey, etc ...)?	
Does report includes stream characteristics?	
Does report includes overall stream stability (lateral, vertical, potential response)?	
Does report includes abutments characteristics and conditions?	
Does report includes piers characteristics and conditions?	
Does report includes name of engineers responsible for each discipline, engineer registration numbers, and name of reviewing engineer?	
Does report is signed and sealed by a professional engineer?	
Verify data presented is consistent with the conclusions and recommendations.	

Comments:

Reviewer Name:

Reviewer Signature:

Consultant:

Bridge No.

Date:

SCOUR EVALUATION PHASE 2

QUALITY CONTROL FORM

Scour Evaluation Report Item	Yes/No, N/A or Checked
Does report includes summary of significant findings?	
Does report includes conclusions and recommendations?	
Does H-H report include field survey and cross sections signed and sealed by a surveyor?	
Does H-H and Scour report was performed based on Scope of Work guide for H-H report (Appendix A of the SOW)?	
Does H-H report include sources of hydrologic data and/or peak flow value?	
Does Scour report include a grain size analysis?	
Does Scour report recommend at least three (3) countermeasures?	
If any, is increase in existing water surface elevation due to countermeasures within limits of the PB # 13 regulation?	
Does the scour report include analysis for the Design Scour Flood Event?	
Does the scour report include analysis for the Check Scour Flood Event?	
Does the scour report include scour results for long term, contraction, local, and total scour?	
Does the scour report include plot of bridge foundation elements and computed scour depths?	
Does plot of bridge foundation indicate if foundations are known? In the case of deep foundation, does it indicate tips elevation?	
Does report includes name of engineers responsible for each discipline, engineer registration numbers, and name of reviewing engineer?	
Does report is signed and sealed by a professional engineer?	
Verify data presented is consistent with the conclusions and recommendations.	

Comments:

Reviewer Name:

Reviewer Signature:

Consultant:

Bridge No.

Date:

SCOUR EVALUATION PHASE 3

QUALITY CONTROL FORM

Scour Evaluation Report Item	Yes/No, N/A or Checked
Does report includes summary of significant findings?	
Does report includes conclusions and recommendations?	
Does report includes data for the geotechnical and structural scour assessment (Drawings/Field Measurements/NDT's)?	
Are live loads capacities evaluated in accordance with the PRHTA's current load rating guidelines?	
Does report includes estimated capacity of each foundation unit under scour conditions defined in Phase 2?	
Does report indicates method used to estimate capacity of foundation units for unknown foundations?	
Does report includes name of engineers responsible for each discipline, engineer registration numbers, and name of reviewing engineer?	
Does report is signed and sealed by a professional engineer?	
Verify data presented is consistent with the conclusions and recommendations.	

Comments:

Reviewer Name:

Reviewer Signature:

Consultant:

Bridge No.

Date:

SCOUR EVALUATION PHASE 4

QUALITY CONTROL FORM

Scour Evaluation Report Item	Yes/No, N/A or Checked
Does report includes summary of significant findings?	
Does report includes conclusions and recommendations?	
Does report includes POA based on Scour Critical Bridge – POA format?	
Does POA include at least three (3) alternate countermeasures?	
Does POA include a conceptual plan for recommended alternative countermeasure?	
Does report includes name of engineers responsible for each discipline, engineer registration numbers, and name of reviewing engineer?	
Does report is signed and sealed by a professional engineer?	
Verify data presented is consistent with the conclusions and recommendations.	

Comments:

Reviewer Name:

Reviewer Signature:

A.8 Scope of Work of Underwater Inspection

SCOPE OF WORK

UNDERWATER INSPECTION

I. PROJECT DESCRIPTION

The purpose of this work is to perform an adequate level of underwater inspections to bridges on marine and freshwater areas in Puerto Rico. An adequate inspection level detects damaged structure members, section loss, timber decay or attack by marine borers, scour, and undermining of footings or concrete walls.

II. GENERAL DESCRIPTION

The Consultant will provide all labor, equipment and materials necessary to complete the inspections and reports.

The bridges that will require an underwater inspection are included in **Appendix A**. For each structure the PRHTA will provide, when available, site descriptions, plans or sketches, previous inspection reports, and access information.

If during the period of contract, any new procedures or regulations regarding Underwater Inspection are added or modified, they shall be added to the work performed by the Consultant with no additional cost for the PRHTA.

III. INSPECTION PERSONAL QUALIFICATIONS AND REQUIREMENTS

A. Qualification requirements for the Engineer in charge of the inspection and report preparation:

1. Be a registered professional engineer licensed to practice in Puerto Rico.
2. Have a minimum of five years experience in underwater structure inspection assignments.
3. Meet the qualifications of a bridge inspection team leader in accordance with the National Bridge Inspection Standards (NBIS), 23 CFR 650.309(b).
4. Within five years prior to the inspection completion, successfully complete an approved comprehensive bridge inspection training course or bridge inspection refresher training course in accordance with NBIS, 23 CFR 650.309(b) or 650.313(g), respectively.
5. Be on site at all times coordinating the inspections.

B. Qualification requirements for the Diver(s) performing the inspection:

1. Meet the qualifications of underwater bridge inspection diver per NBIS 23 CFR 650.309 (b).
2. Have all the required licensing to practice commercial diving in Puerto Rico.

IV. CODES, REGULATIONS, STANDARDS AND PROCEDURES

Inspect in accordance with the practices and procedures contained in the following regulations and operational guidelines. Consider the following:

- Geographical location of the site,
- Climatic conditions,
- Tides,
- Currents,
- River levels, and
- Site-specific constraints.

FHWA Guidelines

Bridge Inspector's Reference Manual, October 2002, (Report No. FHWA NHI 03-001 and 03-002)

Underwater Inspection of Bridges, (Report No. FHW A-DP-80-1)

Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges, (Report No. FHWA-PD-96-001)

Manual of Uniform Traffic Control Devices, December 2009 or latest edition.

AASHTO Specifications and Guidelines

Manual for Bridge Evaluation, 2nd Edition, with current interims.

National Bridge Inspection Standards

Title 23, Code of Federal Regulations, Part 650, subpart C, latest rule applicable.

Commercial Diving Operations

Title 29, Code of Federal Regulations, Part 1910, subpart T, latest revision.

U.S. Coast Guard Navigation Rules

U.S. Coast Guard pamphlet CG-169, May 1, 1977, Rule 27, or latest revision.

Puerto Rico Highway and Transportation Authority Specifications

Standard Specification for Road and Bridge Construction, 2005 edition.

Units

Use English units of measurement.

Bridge Member Identification

Use the member identification convention consistent with PRHTA provided documents and the Bridge Safety Inspection Manual, Department of Transportation and Public Works, Puerto Rico Highway and Transportation Authority, July 2011

V. TERMS AND DEFINITIONS

PRHTA - Puerto Rico Highway and Transportation Authority

Consultant - The individual, partnership, firm, corporation, or any acceptable combination thereof, contracting with the PRHTA for performance of the work included in this contract.

Professional Registration - Prepare all work under the direct supervision of the registered professional engineers who are in responsible charge for the services. These Engineers currently registered in Puerto Rico will sign, seal and certify the accuracy of each final work product.

Correspondence - On all correspondence include the PRHTA's assigned Project name and numbers.

Identified Substructure Unit (ISU) - Each structure description lists the substructure members for inspection.

Documents and Reports - Print reports with solid black letters that are double-spaced on white, 8.5-inch x 11-inch bond or "Xerox Copy" paper. Fold larger paper sizes to 8.5 -inch by 11-inch.

Submit a digital copy of final reports complete with color photographs and sketches. Provide this report in Adobe Acrobat (pdf) or other approved format.

Report Copies - Provide (2) hard copies and (1) digital copy.

Covers - Include the following information on the cover of all documents and reports:

- a. Name of document or report.
- b. Date.
- c. Indicate whether draft or final.
- d. Project Name.

- e. State and Federal Project Number(s).
- f. Prepared for: Puerto Rico Highway and Transportation Authority
- g. Prepared by: (Company Name)

Revisions - Modify work products in response to direction from the PRHTA. Consider corrections, adjustments, or modifications necessitated by the review/approval process, which do not substantially affect the scope, complexity, or character of the services, as a normal part of the Consultant services.

Errors and Omissions - Submit the completed work products to the PRHTA. Prior to submitting correct all significant errors or omissions within the work products. The PRHTA will not accept errors or omissions in the work product.

Comment Resolution - Provide a written response with subsequent submittals that address all written and oral PRHTA comments. Clearly explain all changes from previous submittals.

VI. PROJECT WORK TASKS

Structure Inspection

The inspections include but are not limited to the following:

1. Identify and list each member of the underwater inspection or Substructure Unit.
2. Include a drawing with the UW inspection elements (ISU's) identified at the beginning of the inspection report.
3. Document inspection procedure for UW inspection of the bridge and include the procedure as part of the UW inspection report.
4. As defined in the FHWA Manual, Underwater Inspection of Bridges: A complete Level I underwater inspection on 100% of ISU's from the mud line to the water surface.
5. Identify and document the High Watermark.
6. Visually inspect the ISUS's from the High Watermark down to the current water level of the above water portion of ISU's using the diver in the water or inspector from a workboat.

7. Assign a Condition Rating to each member according to the FHWA Recording and Coding Guide.
8. Identify the AASHTO Core elements and assign condition state quantities. Document the quantities per ISU's and in total.
9. As defined in the FHWA Manual, Underwater Inspection of Bridges: A Level II underwater inspection on 10% of ISU's below the water surface.
10. As defined in the FHWA Manual, Underwater Inspection of Bridges: A Level III underwater in depth inspection identified in the Scope of Work -or- where deemed necessary by the Engineer in Charge. Immediately notify the PRHTA Contract Manager when a Level III inspection is deemed necessary and the reasons for this determination. Based on the information provided, the PRHTA may require a more detailed inspection.
11. Assess the riprap condition at embankments and intermediate piers.
12. Document all deficiencies with photos and additional sketches.
13. Document typical conditions, with colored photographs and/or sketches including debris build up.
14. Locate, describe and quantify all structural defects with colored photographs and/or sketches including damaged members, remaining section, timber decay or attack by marine borers. Use clear water box when necessary.
15. Measure all local scour and scour undermining of footings, concrete walls or around piles by width, length and depth. Reference the scour elevations from the as-built drawings and locate in relation to adjacent substructure unit. Measure and locate the maximum water depth at each substructure unit.
16. Identify stream or channel bed material.

Documentation and Reporting

Submit draft and final reports for each structure to the PRHTA for review and approval.

VII. CONSULTANT RESPONSIBILITY

General

1. Comply with all laws, ordinances, rules, regulations and orders of any public authority bearing on the performance of the work.
2. Establish a safe environment for work site personnel.

Scheduling

Prepare a proposed schedule thirty (30) calendar days prior to starting underwater inspections. Update detailed structure inspection dates and approximate times at least two (2) weeks prior to the actual start date. When schedule revisions arise, immediately submit a revised schedule to the PRHTA Project Manager. No inspection shall begin prior to schedule approval.

Identify the optimal time for underwater inspections. Balance underwater inspection schedule between the low flow and the higher flows produced by and seasonal rains.

Debris Removal

Remove sufficient debris such as trees, brush and other miscellaneous objects that inhibit inspection efforts or cause unsafe conditions.

The Consultant may review existing inspection records to identify debris accumulation history when such data is available.

Work Zone Traffic Control

Use inspection procedures which minimize traffic disruption. Omit traffic control, when underwater inspections are completely off the roadway and behind existing railing. Develop and submit to the PRHTA for approval a TCP, when a TCP is needed.

Provide a safe and expeditious movement of traffic around and through the inspection/work site. Implement the approved TCP and utilize traffic control devices when the underwater inspection affects the movement or safety of traffic or the safety of the work force.

Employ the Traffic Maintenance in accordance to the most recent Manual on Uniform Traffic Control Devices (MUTCD). Obtain an approved and signed TCP prior to submitting or commencing work on PRHTA right of way. During the work maintain a copy of the PRHTA approved traffic control plan on site.

Inspection Coordination

Prior to performing the inspections coordinate the inspection efforts with the PRHTA, all applicable Federal, State, and Local Agencies including law enforcement.

Coordinate activities with Marine Transport Authority managers and schedule inspections to avoid conflicts with ferry vessel (*Aqua Expreso*) arrivals and departures.

Permits

Obtain all permits required for these inspections including, but not limited to, any required by the U.S Army Corps of Engineers , Natural Resources Department and U.S. Coast Guard.

Inspection Procedures

Determine how to safely access each structure. Develop an access plan for each structure. Assess site conditions prior to diving. Document unsafe diving conditions by providing color photos and a written explanation of why the site is unsafe.

Contact the PRHTA Project Manager every regular business day or immediately as necessary with inspection updates. Immediately notify the PRHTA Project Manager or on-site PRHTA Inspector of any significant inspection findings.

Do not block or restrict stream navigation. Provide adequate notification to water traffic during dive operations and provide safety vessel as necessary.

VIII. MANAGEMENT**Schedule(s)**

Adhere to the Project Duration to meet the PRHTA's long-term goals and commitments. Expend all effort necessary to stay on schedule.

Project Coordination

Coordinate all project correspondence through or with the concurrence of the PRHTA's Project Manager.

FHWA Communication

The PRHTA shall conduct all project related communications with FHWA.

PRHTA Activities

The PRHTA's Project Manager coordinates the Consultants activities with those of various functional groups within the PRHTA. These groups include Traffic, Safety & Utilities; Highway Construction; Bridge Engineering Office; and Contracts.

For quality assurance purposes, the PRHTA may have an inspector on site for some or all inspection activities.

Release of Information

The PRHTA's Project Manager must approve the release of any project related information.

IX. QUALITY CONTROL PROGRAM

The Quality Control Program shall include the following elements:

1. Quality Reviews: The Consultants shall conduct quality reviews to make certain their own organization is in compliance with the requirements cited in this scope of services. Quality reviews shall be conducted to evaluate the adequacy of materials, documentation, processes, procedures, training, guidance, and staffing included in the execution of this contract.
2. Quality Control Plan: The Consultant shall include a quality control plan in the technical proposal furnished to the PRHTA. The quality control plan shall detail the procedures, evaluation criteria, and instruction to his organization to assure conformance with the contract. No work shall be initiated until the Consultant's quality control plan is approved by the PRHTA. Significant changes to the work requirements may require the Consultant to revise the quality control plan. It shall be the responsibility of the Consultant to maintain a plan current with the work requirements. The Plan shall include, but not be limited to, the following areas.
 - a. Organization - A description is required of the Consultant's quality control organization and its functional relationship to the part of the organization performing the work under the contract. The authority, autonomy, and responsibilities of the quality control organization shall be detailed as well as the names and qualifications of personnel in the quality control organization.
 - b. Quality Control - The Consultants methods use to monitor and assure compliance with the contract requirements for services and products shall be detailed.
 - c. Quality Assurance - The Consultant's methods used to monitor and assure compliance with the quality control requirements shall be detailed.

- d. Quality Records - The types of records which will be generated and maintained by the Consultant during the execution of a quality control program shall be outlined.
- e. Control of Subcontractors and Vendors - The methods used by the Consultant to control the quality of the subcontractors shall be detailed.
- f. Quality Control Certification - An officer of the Consultant firm will be required to sign and seal a certification that will accompany each submittal stating that the drawings, reports, etc. have been prepared and checked in accordance with PRHTA Specifications.

The Consultant shall maintain adequate records of the quality control actions performed (including subcontractors and vendors) in providing services and products under this contract. All records shall indicate the nature and number of observations made, the number and type of deficiencies found, and the corrective actions taken. Deficiencies identified through the consultant's QC and/or QA processes shall be reported to the PRHTA and immediately corrected. Failure to make satisfactory and timely correction to deficiencies, identified by the consultant or by the PRHTA, shall result in withholding of payments, pending satisfactory corrections being made. These records shall be available to the PRHTA upon request during the contract term. All records shall be kept at the primary job site. All records are subject to audit review.

X. BRIDGE INSPECTION REPORTS

Requirements

Submit a concise, complete and separate report for each structure with the following:

1. Executive Summary - Identify critical findings and locations of significant observations and deficiencies. Provide critical findings and maintenance work recommendations. Include the site time, inspection date, type of dive and safety vessels used, and crew size in a summary table.
2. Introduction - Include a description of the structural members inspected, inspection method, Inspection access method, traffic control, equipment used, and list the name and date of Agencies and individuals contacted.
3. Inspection Findings - Include detailed observations and a brief narrative with an overall condition summary for the members inspected. Provide inspection findings for each member.
 - Describe, quantify, and locate all defects.
 - Discuss nondestructive testing methods. Provide all test results.

- Assign a Condition Rating to each member according to the FHWA Recording and Coding Guide for the Structure Inventory and Appraisal of the Nation's Bridges, (Report No. FHWA-PD-96-001)
4. Special Conditions - Discuss all hydrological data including debris, scour, streambed material, and water conditions. Discuss access method, traffic control, and nondestructive testing methods used. Provide all test results.
 5. Sketches - Provide plan and elevation views of piers and abutments drawn to scale. On these sketches:
 - Show the streambed spot water depth map; the flow direction; and north arrow.
 - Locate scour holes and points of maximum scour depth with approximate dimensions (length, width and depth below normal streambed); reference the horizontal location to the nearest substructure unit; and vertical distance from the deck to water surface.
 - Locate all deficiencies.
 - Show deck elevation to water surface measurement.
 6. Typed Daily Dive Reports - Include crew size and crew names, the time the diver's actually spent inspecting the structure, tide table record with time and location references, maximum dive depth at each identified substructure unit, maximum current, visibility, ambient and water temperatures. Provide typed Daily Inspection Logs.
 7. Color photos documenting the typical member condition and all defects, diver access methods include staging and boat launch areas, substructure members above and below the water, in-place traffic control, rip rap condition, and debris before and after removal.

Draft Reports

Submit all draft Structure Reports complete with color photos and sketches to the Bridge Engineering Office for review. Provide a digital copy and one hard copy.

Final Reports

Submit final Structure Reports complete with color photos, sketches and Engineers seal. Provide a digital copy and two bound paper copies.

XI. METHOD OF COMPENSATION

The method of payment for the Consultant contract will be on per bridge basis.

In the sealed Economical Proposal submitted by the Consultant a task breakdown shall be included per bridge.

A lump sum item for debris removal shall be included per bridge. This lump sum shall be negotiated on a per bridge basis when needed.

A.9 Procedure for Conducting Underwater Inspection

The Federal Highway Administration (**FHWA**), through the National Bridge Inspection Standards (**NBIS**), and through guidance provided in Technical Advisories, specifies that underwater members shall be inspected to the extent necessary to determine with certainty that their condition has not compromised the structural safety of the bridge. The NBIS recognizes seven types of bridge inspections: routine, initial, damage, in-depth, underwater, special, and fracture critical.

Underwater inspection (**UWI**) is the evaluation carried out on those bridges where the submerged portions of the structure have a history of at least four feet of water depth throughout the year, or, where the elements are submerged in less than four feet of water, but the evaluation thereof by the method of wading, would be unsafe. A summary of the procedure to perform the UWI is presented below, complying with the NBIS' minimum requirements for the safety and proper inspection of all highway bridges. Also, the UWI procedure must comply with the safety standards set by the Occupational Safety and Health Administration's (**OSHA**), as well as any other local, federal or more restrictive applicable regulation.

Procedure for Conducting the Underwater Inspection

1. Select the Underwater Inspection Team

Bridge owner must select the UWI team. The UWI team is composed of individuals who meet the requirements for bridge inspection specified by the NBIS and the minimum requirements of diving required by OSHA regulations, for commercial diving operations (29 CFR 1910 Subpart T).

Members of the team that participated in the inspection of this bridge are included in Table I: "Inspection Data" of the "Underwater Bridge Inspection Report", for Bridge No. 0542, prepared by Behar Ybarra & Associates, LLP and Bolt Underwater Services.

2. Site Reconnaissance and Data Collection

Through data collection or an on-site reconnaissance, the dive inspection team leader should:

- a. Determine the approximate water depth, from the drawings or from field measurements.
- b. Determine the approximate velocity of the water.
- c. Perform a hazard analysis.
- d. Do a pre-dive checklist.
- e. Determine if due to the work of inspection, traffic may be affected.
- f. Obtain all data related to the structure to prepare for an inspection.
- g. Prepare an UWI operation plan.
- h. Review previous reports so that they can help to determine the rate of deterioration of the previously observed defects.
- i. Review the previously established measures against scour.

3. Determine the Level of Inspection

Bridge owner and/or UWI team must determine the level of inspection required. Three levels of inspection have been adopted for UWIs, these are summarized below as:

Level I: Visual, tactile inspection.

Level II: Detailed inspection with partial cleaning.

Level III: Highly detailed, inspection with Non-Destructive Testing or Partially Destructive Testing.

A routine UWI normally includes a 100% Level I inspection and a 10% Level II inspection. It may also include additional Level II inspections and Level III inspections, as necessary.

For the evaluation of this bridge, Level I inspection were performed on 100% of the submerged portions of each identified substructure unit. Level II cleanings and Level III inspection were not required for this bridge.

4. Establish the Inspection Frequency

UWI team shall establish the inspection frequency. UWI shall be done at least every 60 months or more frequently if conditions warrant. However, the NBIS has specified certain factors that shall be considered in establishing the frequency of inspections and normal levels of inspection effort.

The UWI for Bridge No. 0542 was completed on August 15, 2012. The recommended next UWI should be done no later than 60 months from this date.

5. Bridge Data

a. Bridge Type

Cast in place concrete bridge with 6 spans

b. Location – Refer to Figure 1: "Bridge Location Map".

6. Identify the Substructure Unit Located in Water

The inspector must recognize the various types of substructure configurations, likely locations and types of commonly encountered defects, and understand the causes and mechanisms of deterioration. The types of structures include pile bents, piers, abutments, caissons, cofferdams and foundation seals, protection devices and culverts. The ISU's in water, requiring Level I inspection and a 10% of Level II, are presented below:

- a. ISU:** Columns 1 and 2 of Bents 2 through 6
- b. ISU:** Channel

7. Select the Underwater Inspection Method and Technique

The UWI team shall select the inspection method. There are currently three methods used to conduct UWI, these are:

- a. Wading** - requires only a probing rod and boots to be effective.
- b. Scuba Diving** - A method that allows a more detailed examination of substructure conditions at the mudline.
- c. Surface-Supplied Air Diving:** A diving mode in which the diver in the water is supplied from the dive location with compressed air for breathing. This method may involve the use of sophisticated diving equipment. This inspection method is well suited when adverse conditions will be encountered.

For this bridge, the Wading and Scuba Diving methods and techniques have been employed.

8. Perform the Routine Inspection and Determine the Deterioration of Structural Material

A routine inspection is a regularly scheduled inspection consisting of observations and measurements needed to determine the physical and functional condition of the bridge, to identify any changes from initial or previously recorded conditions, and to ensure that the structure continues satisfying the present service requirements. The routine inspection may indicate that an in-depth inspection is needed in some areas to confirm the Level I and Level II findings, or to gain additional data so that the structural conditions can be evaluated with certainty.

The UWI team shall determine the deterioration of the structural material, primarily concerned with ratings of the five inventory and appraisal NBI items: Item 60-Substructure, Item 61-Channel and Channel Protection, Item 62-Culverts, Item 71-Waterway Adequacy, and Item 113-Scour Critical Bridges (see section 10).

Deterioration of structural material depends mainly on the type of material. For concrete, there are four main types of deterioration: cracking, scaling, spalling, and chemical attack. Steel foundation elements located in water, can suffer distress in the form of corrosion. Typical problems found in masonry structures include cracking, scaling, and deteriorated pointing. The deterioration of structural materials found on Bridge No. 0542 includes concrete scaling, and spalling with exposed steel; for detailed deficiencies and locations refer to Table 3.

9. Underwater Inspection Equipment and Safety Measures

- a. **Vehicular Traffic** –The vehicular traffic was not impacted during the inspection. Traffic Control Devices and/or Maintenance of Traffic were not needed for UWI of this bridge.
- b. **Access** –The access to many small bridges can be accomplished from the adjacent shore; for larger waterways, a boat will be necessary. In this case the bridge was accessed by the adjacent shore.
- c. **Divers Equipment**–There are several divers equipment including: exposure suits, commercial scuba equipment, and communication equipment, among others. For the evaluation of this bridge, diving equipment was used. For additional equipment employed in the evaluation of this bridge, refer to Table 1: "Inspection Data".
- d. **Inspection Tools** – There are two types of inspection tools; hand tools and power tools. For this bridge, both types of tools have been used.
- e. **Images**–Color photographs taken during the UWI are included in the report, refer to Appendix B.

10. Inspect the Scour

UWI team must evaluate bridge for scour (Refer to NBIS-Item 113-Scour Critical Bridges). UWI team shall accomplished two main objectives in inspecting bridge for scour: To accurately record the present condition of the bridge and the stream and to identify conditions indicative of potential problems with scour and stream stability, for further review and evaluation by others. For bridges that are scour critical, bridge owners must prepare a plan of action to monitor known and potential deficiencies and to address critical findings.

A basic scour investigation should include sounding and probing the channel bottom adjacent to the structure, and determining channel cross sections in the area of the bridge.

For information related to scouring, refer to "Underwater Bridge Inspection Report", for Bridge No. 0542, made by Behar Ybarra & Associates, LLP and Bolt Underwater Services.

11. Critical Findings - Critical findings shall be identified by the UWI team and shall be addressed in a timely manner. The FHWA shall be periodically notified of the actions taken to resolve or monitor critical findings. The underwater inspection team found some incidental issues on Bridge No. 0542. There are:

- a. There are struts running between Piles No. 1 & 2, of Bents No. 3 through 5, 6 feet, approximately, below the waterline.

12. Determine the Condition Rating and Core Element Level Condition Rating

UWI team shall determine the condition rating and the core element level condition rating. The NBI rating system assigns a numerical rating from "9" to "0" for substructure units; an element based rating system breaks the substructure units into Commonly Recognized (**CoRe**) Structural Elements, and determines the quantity of submerged piles that should be classified in various condition states. In general, the condition states range from "1" to "5".

The CoRe Structural Elements are a group of structural elements endorsed by the American Association of State Highway Transportation Officials (AASHTO) as a means of providing a uniform basis for data collection for any bridge management system. Each element is also assigned an environment rating from "1" to "4" representing the aggressiveness of operating practices and the local environment in which the element is located.

For NBIS ratings refer to Table 2: "NBIS Overall Condition Rating" of the "Underwater Bridge Inspection Report", for Bridge No. 0542, made by Behar Ybarra & Associates, LLP and Bolt Underwater Services.^{1 2}

¹Bridges with fracture critical members and complex bridges shall be inspected according to those procedures.

²The Procedure for Conducting the Underwater Inspection shall comply with the NBIS and OSHA requirements and any other more restrictive local or federal requirement. The Underwater Inspection Report for Bridge No. 0542, done by Behar Ybarra & Associates, LLP/Bolt Underwater Services on July 2013, as well as, previous inspection's reports for this bridge, provided by the Puerto Rico Highway and Transportation Authority, have been used as reference also.

A.10 Failure Critical Inspection Procedures

Inspection procedures begin with proper advanced planning. Important planning aspects usually based on an office review of the structural plans, fracture critical inspection records, and aerial views of the location, include:

- Identify all fracture critical members on both the superstructure and substructure including all primary gusset plate locations.
- Note the particular members in the structure that may require special field attention, such as built-up tension members composed of few individual pieces.
- Determine equipment necessary to access the fracture critical members, such as a SNOOPER® truck, ladders, bucket truck, boom lift, boat, etc.
- Identify all personal protective equipment necessary to inspect the structure such as hard hat, vest, steel toed boots, flashlight, respirator, or eye protection.
- Many structures designed for urban situations with necessary complex alignment geometries result in FCMs. Proper inspection of these bridges may require closing a traffic lane or require a night time inspection due to high average daily traffic. Coordinate safe traffic control in advance with the local District and Area offices and their Safety Review Team.
- Identify and make available any necessary special tools and equipment that may be required in addition to the normal inspection gear.
 - Tools for cleaning:
 - Shop-vac to remove loose debris
 - Brushes – both wire and synthetic
 - Needle scaler
 - Chipping hammer
 - Compressed air

- Non-destructive test equipment
 - Penetrant Testing - Used for crack verification and crack tip location.
 - Magnetic Particle Testing – Used for crack verification and crack tip location
 - Ultrasonic Testing – Used to detect subsurface cracks and evaluate remaining thickness at locations that have experienced section loss. All pin-and-hanger connections are evaluated using this method.

The actual field inspection of all FCMs consists of a hands-on visual inspection of all fracture critical members. The inspector notes any:

- Visual cracks and records the crack length, direction, and location, describing what member the crack is in and where on the member the crack is located. Marks are made on the member documenting crack limits and the date crack observations were made. A picture is taken of each crack and placed in the picture report.
- Corrosion resulting in section loss of the fracture critical member, or resulting in pack rust between built-up members. At least one picture showing the worst location of this type of deficiency is placed in the picture report.
- Weld terminations, plug welds, or intersecting welds in a tension area.
- Interrupted back-up bars used for built-up-member fabrication.
- Arc strikes, scars from assembly cables or chains, or other physical damage.
- Cross-section changes which may cause a sudden increase in the stress pattern.
- Each type of AASHTO defined fatigue detail and all known problematic details that exists on the member. A picture of each is taken and placed in the picture report.

After the field inspection has been performed, a report is written to document the inspection plan and the current condition of the FCMs. This written report is supplemented by a picture report. The picture report contains an example of each type of fatigue detail, the overall layout of the structure, a picture of every crack, and the location of moderate to severe section loss. The format of the report is as follows:

Methods:

- The equipment used to perform hands on inspection of all FCMs, including required safety equipment.
- Who performed the inspection?
- How long was required to perform the inspection?
- When the inspection was performed (night/day – date).
- Site specific details requiring special attention (fatigue details, problematic details, primary gusset plates, dangerous conditions).

- List special equipment necessary to clean and verify cracks and quantify section loss, including non-destructive evaluation techniques used.
- Equipment setup and access points

Traffic Control:

- The agency or contract firm that provided the traffic control.
- Lane closure information.
- Advanced notification time required to obtain ROW access

Structure Description:

- Year Built, Structure Type, Length of Structure
- Structure Orientation.
- Identification of each FCM which are then grouped by the appropriate AASHTO fatigue category.
- List of fatigue details and problematic welded detail.

Inspection Summary:

- Describe the general condition of the structure.
- List all cracks, on the structure, in a table showing length, location, and direction.
- List all section loss dimension and locations. This value should be recorded as remaining thickness.
- List all pack rust locations that are causing bowing of the built-up members or causing bolts or rivet to shear.

Recommendations:

- Repair and/or preventative maintenance recommendations based on the information shown in the inspection summary.
- Items to be updated on the NBI as a result of the fracture critical inspection. ☐☐Item 59 - Superstructure Rating (0-9)
 - Item 60 - Substructure Rating (0-9)
 - Item 92A - Fracture Critical Inspection Frequency (0-24)
 - Item 93A - Fracture Critical Inspection Date (Month/Year)

Appendix A includes an example layout of a typical fracture critical inspection report outlining items that are documented from the inspection.

Maintenance and Preservation

Although rare, routine FC inspections occasionally reveal defects or deterioration in fracture critical elements. In those cases, the inspectors who observe and document the defects work closely with the engineers in the Bridge Maintenance group, located in DOT Bridge Division, to develop appropriate repair procedures and details.

When damage is limited to a few isolated locations, DOT personnel often perform repair work themselves. There are several Certified Bridge Welders on staff in the Bridge Maintenance group who are capable of performing a variety of routine repairs to steel structures. When damage is more substantial or widespread, DOT engineers develop repair procedures and details for use by a qualified steel repair contractor.

Repairing damage to fracture critical elements is given a high priority, particularly when the deterioration appears to be rapidly advancing or when it could potentially lead to failure of the element and consequently to the bridge itself. Routine maintenance is also an important aspect of preserving fracture critical elements. Repairs are typically far more effective and less expensive when a problem is addressed before it advances to the point that the load-carrying capacity of the structure is compromised. For example, cleaning and coating steel that exhibits minor surface corrosion can prevent deterioration from worsening to the point that more substantial structural repairs are necessary.

Appendix B Forms for Field Inspections

Bridge Number:	
<i>Location</i>	
<i>Team</i>	
<i>Insp. Date</i>	
<i>Previous Insp. Date</i>	
Structure Type and Materials	
<i>Number of Spans</i>	
<i>Main Span Material</i>	
<i>Deck Protection</i>	
<i>Structure Length</i>	
<i>Structure Width</i>	
Deck	
<i>Material</i>	
<i>Deterioration</i>	
<i>Drainage</i>	
<i>Safety</i>	
Slab or Plate	
<i>Material</i>	
<i>Condition</i>	
<i>Corrosion</i>	
Superstructure	
<i>Type</i>	
<i>Paint</i>	
<i>Condition</i>	

Steel beams

Type	
Condition	
Corrosion	
Paint	

Concrete

Type	
Condition	
Cracking	

Truss

Paint	
Corrosion	
Members	

Comments

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Appendix C Inspection by PRHTA of PB 0960

C.1 April 15, 2014

INSPECTION REPORT SUMMARY & QC SHEET

BRIDGE: 960

TEAM LEADER: Eng. Arturo Cáceres Febus

INSP. DATE: 4/15/14

1. Inspection Type and Dates:

NBI	Type	Performed? (Yes / No / NA)	Freq (MONTHS)	Previous Insp. DATE (MONTH/YEAR)	Next Insp. DATE (MONTH/YEAR)
ITEM 90	Routine Inspection	Y	48	12/9/08	4/15/18
ITEM 93 A	FC Inspection	N	—	—	—
ITEM 93 B	Underwater Insp.	N	—	—	—
ITEM 93 C	Other:	N	—	—	—

2. NBI Condition Rating Summary:

	Item 58	Item 59	Item 60	Item 61	Item 62	Item 113
Previous Inspection	4	4	5	N	N	N
Current Inspection	5	4	5	N	N	N

Other Checks: (Y, N, NA) Review Comments:

- Scour Critical (items 113 & 60)
- AASHTO Core's & NBI CD consistent
- Smart Flags (scour, steel plate, fire damage, etc)
- Channel Profile/Clearance Table
- FC & Underwater Members Tables
- Asphalt Overlay Thickness
- Drawings
- Photos
- Critical Finding
- Inspector & Team Leader Signature

Reviewer:

Safety Eng.:

Bridge Inspection Report

Bridge Key: 009601 Agency ID: 009601 Sufficiency Rating: -2.0

IDENTIFICATION

State 1: 72 Puerto Rico Struc Num 8: 009601
 Facility Carried 7: PR 18 Location 9: 0.5 KM N INT PR21 & PR18
 Rte.(On/Under)5A: One Route Under Rte. Signing Prefix 5B: 1 Interstate Hwy
 Level of Service 5C: 1 Mainline Rte. Number 5D: 00018
 Directional Suffix 5E: 1 North % Responsibility: NA
 SHD District 2: -1 County Code 3: SAN JUAN
 Place Code 4: SAN JUAN ZONA URBANA Kilometer Post 11: 01.7 km
 Feature Intersected 6: PEDESTRIAN WALKWAY
 Latitude 16: 18d 23' 24" Longitude 17: 066d 04' 36"
 Border Bridge Code 98: Unknown (P)
 Border Bridge Number 99: Unknown

INSPECTION

Frequency 91: 48 months Inspection Date 90: 4/15/2014 Next Inspection: 04/15/2018
 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA
 UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 48 months Element Inspection Date: 01/01/1901 Next Elem. Insp. Due: 01/01/1901

CLASSIFICATION

Defense Highway 100: 2 On Non-Interstate STRA Parallel Structure 101: Unknown (NBI)
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Not Applicable (P)
 Highway System 104: 0 Not on NHS NBIS Length 112: Unknown (NBI)
 Toll Facility 20: 3 On free road Functional Class 26: 12 Urban Fwy/Expwy
 Historical Significance 37: Not Applicable (P)
 Owner 22: -1 Unknown (P)
 Custodian 21: -1 Unknown (P)

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: -1 Number of Spans Main Unit 45: -1
 Main Span Material/Design 43A/B:
 2 Concrete Continuous 04 Tee Beam
 Deck Type 107: Unknown (NBI)
 Wearing Surface 108A: Unknown (NBI)
 Membrane 108B: Unknown (NBI)
 Deck Protection 108C: Unknown (NBI)

CONDITION

Deck 58: 5 Fair Super 59: 4 Poor Sub 60: 5 Fair
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

LOAD RATING AND POSTING

Inventory Rating Method 65: Unknown (NBI) Operating Rating Method 63: Unknown (NBI)
 Inventory Rating 66: MS-0 Operating Rating 64: MS-0.6
 Design Load 31: 7 Pedestrian Posting 70: Unknown (NBI)
 Posting status 41: Not Applicable (P)

AGE AND SERVICE

Year Built 27: 1967 Year Reconstructed 106: Unknown
 Type of Service on 42A: 3 Pedestrian-bicycle
 Type of Service under 42B: 1 Highway
 Lanes on 28A: Unknown Lanes Under 28B: 9 Detour Length 19: 0.0 km
 ADT 29: 178,900 Truck ADT 109: 4 % Year of ADT 30: 2005

APPRAISAL

Bridge Rail 36A: N N/A or not required Approach Rail 36C: N N/A or not required
 Transition 36B: N N/A or not required Approach Rail Ends 36D: N N/A or not required
 Str. Evaluation 67: N Deck Geometry 68: N Not applicable (NBI)
 Underclearance, Vertical and Horizontal 69: 4 Tolerable
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: Not Applicable
 Scour Critical 113: Not Applicable (P)

GEOMETRIC DATA

Length Max Span 48: 22.20 m Structure Length 49: 49.30 m
 Curb/Sdwk Width L 50A: Curb/Sidewalk Width R 50B:
 Width Curb to Curb 51: 2.10 m Width Out to Out 52:
 Approach Roadway Width 32: 44.30 m Median 33: Unknown (NBI)
 Deck Area:
 Skew 34: -1.00 ° Structure Flared 35: Unknown (NBI)
 Minimum Vertical Clearance Over Bridge 53:
 Minimum Vertical Underclearance Reference 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 04.64 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 03.30 m
 Minimum Lateral Underclearance L 56: 02.60 m

PROPOSED IMPROVEMENTS

Bridge Cost 94: \$ 0 Type of Work 75: 35 Rehabilitate-gen.
 Roadway Cost 95: \$ 0 Length of Improvement 76: 00 00 m
 Total Cost 96: \$ 42,115 Future ADT 114: 216,469
 Year of Cost Estimate 97: 2014 Year of Future ADT 115: 2020

NAVIGATION DATA

Navigation Control 38: Unknown (NBI)
 Vertical Clearance 39: Horizontal Clearance 40:
 Pier Protection 111: Not Applicable (P) Lift Bridge Vertical Clearance 116:

ELEMENT CONDITION STATE DATA

BRIDGE NOTES

Bridge Inspection Report

PAST INSPECTION

Inspection Date: 04/15/2014

Type: 1 Regular NBI

Inspector: -1

Pontis User Key: Pontis - Pontis Poi

Scope:

NBI: Other: Element:
Underwater: Fracture Critical:

INSPECTION NOTES

THE WEARING SURFACE SHOWS POOR FINISHING, LIGHT SCALINGS AND FINE TO MEDIUM TRANSVERSE CRACKS. INADEQUATE RAMPS FOR HANDICAPS. HOLES AND MUCH VEGETATION ALONG THE WEARING SURFACE AND RAILINGS. RAMPS RAILINGS WITH BROKEN SECTIONS FROM SUPPORTS AND MOVEMENT. LARGE SPALLINGS WITH EXPOSED REINFORCED STEEL DUE TO TRAFFIC IMPACT AT BEAMS OF SECOND SPAN OVER ROUTE TOWARDS CAGUAS AND OVER REVERSIBLE LANE. MEDIUM HORIZONTAL CRACK AT PIER CAP (EAST SIDE).

INSPECTOR WORK CANDIDATES

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

Team Leader: Arturo Cáceres

Bridge Inspector: Micky Santiago

Bridge Evaluator: Arturo Cáceres

Assistants: John Dávila _ _ _ _

Driver: Juan C. Otero

Inspection date: Abril-15-2014

Weather Conditions: Sunny

Amount of Time on Inspection: 0.5 Hours

Equipment: Bus or Van Underwater Ladders Snooper Camera Boat
 Other:

Bridge Number: **0960**

Road on Structure: N/A **Number or Name:** _ Km. _

Road Under Structure: State Highway **Number or Name:** PR-18 **Km.** 1.7

Ident. Plaque: No **Num.** _

36. Traffic Safety Features:

Bridge railings: not applicable or safety not required

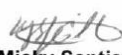
Transitions: not applicable or safety not required


Approach Guardrail: not applicable or safety not required

Approach Guardrail Ends: not applicable or safety not required

41. Posting: **Condition:** A - Open **Sign Type:** _ **Posting Load:** _

COMMENTS AND/OR RECOMMENDATIONS:

Inspection by: 
Micky Santiago
Bridge Inspector

Revised and Approved by: 
Arturo Cáceres
Bridge Evaluator

BR-0960

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**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

58. DECK

58.1 Wearing Surface: Material: Concrete **Condition:** 5 Thickness cm.

Deterioration: 0 to 10 % Drainage: Adequate Ponding: No Safety: Yes

58.2 Slab or Plate: Material: Concrete **Condition:** 6

Cracking: Fine (0.0 to 0.5 mm)

Spalling: Top: Small Bottom: No

Scaling: Top: Light Bottom: No

Efflorescence: No Exudation: No Rust Stains: Yes Corrosion: Moderate

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	4		--	Good	Poor	--	--	No	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: _

Type: Size: Safety: _

Type: Size: Safety: _

58.11 Joints: **Condition:** N/A

Type: Expansion Functioning: No Leaking: N/A Cracking: N Spalling: N Armor: No

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: Other Condition: N Functioning: N/A

Comments:

58.1- Superficie con desprendimientos del resanado, leve "scaling", grietas finas y medianas transversales. Rampas de acceso mas bajas que la superficie del puente, escalon no es apropiado para uso de impedidos. Huecos en la superficie sobre area de pilastras y mucha vegetación sobre la estructura y "railings".

58.8- "Railing" en area de las rampas con secciones partidas de sus bases e inclinadas hacia el interior de la rampa, mucha vegetación sobre el "railing".

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed/Mov. Out of Plumb: No Paint: None

Condition: 6

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.

Condition: 6 Cracking: No Spalling: None Scaling: None

Crushing: No

59.3 Steel beams: N/A Type: N/A

Condition: N Corrosion: None Paint: None

59.4 Concrete: T-beams Type: Cont.

Condition: 4 Cracking: Medium (0.5 to 1.0 mm) Spalling: Large Scaling: None

59.5 Truss: N/A Paint: None Corrosion: None Members: N/A

59.6 Drainage: Type: **Condition:** N Functioning: No

59.7 Hinges: **Condition:** N Functioning: N/A Movement: N/A

59.8 Deflection: Normal

59.9 Vibrations: Minimal

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	Concrete	5	M	Small	L	--	Fair	No	No	No
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	Concrete	5	F	Small	L	--	--	No	No	No
	60.2.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.2.3	Columns	Concrete	6	F	Small	L	--	--	No	No	No
	60.2.4	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.4- Vigas sobre ruta @ Caguas y carril reversible impactadas, "spallings" grandes y mucho acero expuesto en vigas "span" #2 (sobre carril reversible), otros "spallings" en vigas sobre carriles @ Caguas, fuertes impactos, "spallings" y acero expuesto.

60.2- Grieta mediana horizontal en "cap" de pilastra lado este.

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

61. CHANNEL AND PROTECTION Condition: N

61.1 Channel Scour: N/A

61.2 Embankment Erosion: N/A

		Type	Material	Condition Rating	Functioning
61.3	Protective Device		N/A	N	--
61.4	Fender System		N/A	N	--
61.5	Rip Rap		N/A	N	--
61.6	Spur Dikes, Jetties		N/A	N	--

61.7 Obstruction: N/A

61.8 Channel Change: N/A Detrimental: N/A

61.9 Adequate Waterway: N/A

61.10 Surrounding Area: Flooding: N/A if yes explain:

Erosion: N/A if yes explain:

61.11 Location of piers and/or abutment detrimental: N/A if yes explain:

61.12 Other features that may affect structure:

Comments:

62. CULVERT AND RETAINING WALL

	Material	Condition Rating	Functioning	Cracking	Spalling	Scaling	Corrosion	Settlement	Alignment	Scour
62.1 Barrel	N/A	N	--	--	--	--	--	--	--	--
62.2 Head Walls	N/A	N	--	--	--	--	--	--	--	--
62.3 Cut-off wall	N/A	N	--	--	--	--	--	--	--	--
62.4 Retaining wall	N/A	N	--	--	--	--	--	--	--	--

Comments:

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: None Scaling: None Uneven: No
Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No
Excessive deformation: No
Safety: Hazardous: No
Drainage: Inadequate: No
Movement: Pavement-approach slab: No
Embankment: **Condition:** N Functioning: Yes Erosion: No
Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: Abril-15-2014

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:

Underwater:

Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Concrete Cast-in-Place

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Concrete

Type Membrane: None

Deck protection: None

111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

AUXILIARY ITEMS

Signs: Type: Route Orientation Material: Alum. Condition: 6

Type: N/A

BR-0960

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**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

RATING ITEM 58 TO 60:

N: Not applicable

9: Excellent Condition

8: Very Good Condition: no problem noted.

7: Good Condition: some minor problems.

6: Satisfactory Condition: structural element show some minor deterioration.

5: Fair Condition: all primary structural elements are sound but may have minor section loss, cracking, spalling, or scour.

4: Poor condition: advanced section loss, deterioration, spalling, or scour.

3: Serious condition: loss of section, deterioration, spalling, or scour have seriously affected primary structural components. Local failures are possible. Fatigue cracks in steel or shear cracks in concrete may be present.

2: Critical condition: advanced deterioration of primary structural elements. Fatigue cracks in steel or shear cracks in concrete may be present or scour may have removed substructure support. Unless closely monitored it may be necessary to close the bridge until corrective action is taken.

1: "Imminent" failure condition: major deterioration or section loss present in critical structural components or obvious vertical or horizontal movement affecting structure stability. Bridge is closed

0: Failed condition: out of service: beyond corrective action.

RATING ITEM 61:

N: Not applicable. Use when bridge is not over a waterway (channel)

9: There are no noticeable noteworthy deficiencies which affect the condition of the channel

8: Banks are protected or well vegetated. River control devices such as spur dikes and embankment protection are not required or are in a stable condition.

7: Bank protection is in need of minor repairs. River control devices and embankment protection have a little minor damage. Banks and/or channel have minor amounts of drift.

6: Bank is beginning to slump. River control devices and embankment protection have widespread minor damage. There is minor stream bed movement evident. Debris is restricting the channel slightly.

5: Bank protection is being eroded. River control devices and/or embankment have major damage. Trees and brush restrict the channel.

4: Bank and embankment protection is severely undermined. River control devices have severe damage. Large deposits of debris are in the channel.

3: Bank protection has failed. River control devices have been destroyed. Stream bed aggradations, degradation, or lateral movement has changed the channel to now threaten the bridge and/or approach roadway.

2: The channel has changed to the extent the bridge is near a state of collapse.

1: Bridge closed because of channel failure. Corrective action may put back in light service.

0: Bridge closed because of channel failure. Replacement necessary.

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

RATING ITEM 62:

N: Not applicable. Use if structure is not a culvert.

9: No deficiencies.

8: No noticeable or noteworthy deficiencies which affect the condition of the culvert. Insignificant scrape marks caused by drift.

7: Shrinkage cracks, light scaling, and insignificant spalling which does not expose reinforcing steel. Insignificant damage caused by drift with no misalignment and not requiring corrective action. Some minor scouring has occurred near curtain walls, wingwalls, or pipes. Metal culverts have a smooth symmetrical curvature with superficial corrosion and no pitting.

6: Deterioration or initial disintegration, minor chloride contamination, cracking with some leaching, or spalls on concrete or masonry walls and slabs. Local minor scouring at curtain walls, wingwalls, or pipes. Metal culverts have a smooth curvature, non-symmetrical shape, significant corrosion, or moderate pitting.

5: Moderate to major deterioration or disintegration, extensive cracking and leaching, or spalls on concrete or masonry walls and slabs. Minor settlement or misalignment. Noticeable scouring or erosion at curtain walls, wingwalls, or pipes. Metal culverts have significant distortion and deflection in one section, significant corrosion, or deep pitting.

4: Large spalls, heavy scaling, wide cracks, considerable efflorescence, or opened construction joint permitting loss of backfill. Considerable settlement or misalignment. Considerable scouring or erosion at curtain walls, wingwalls, or pipes. Metal culverts have significant distortion and deflection throughout, extensive corrosion or deep pitting.

3: Any condition described in Code 4 but which is excessive in scope. Severe movement or differential settlement of the segments, or loss of fill. Holes may exist in walls or slabs. Integral wingwalls nearly severed from culvert. Severe scour or erosion at curtain walls, wingwalls, or pipes. Metal culverts have extreme distortion and deflection in one section, extensive corrosion, or deep pitting with scattered perforations.

2: Integral wingwalls collapsed severe settlement of roadway due to loss of fill. Section of culvert may have failed and can no longer support embankment. Complete undermining at curtain walls and pipes. Corrective action required to maintain traffic. Metal culverts have extreme distortion and deflection throughout with extensive perforations due to corrosion.

1: Bridge closed. Corrective action may put back in light service.

0: Bridge closed. Replacement necessary.

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

RATING ITEM 113:

N: Bridge not over waterway.

U: Bridge with "unknown" foundation that has not been evaluated for scour. Until risk can be determined, a plan of action should be developed and implemented to reduce the risk to users from a bridge failure during and immediately after a flood event (see HEC 23).

T: Bridge over "tidal" waters that has not been evaluated for scour, but considered low risk. Bridge will be monitored with regular inspection cycle and with appropriate underwater inspections until an evaluation is performed ("Unknown" foundations in "tidal" waters should be coded U.)

9: Bridge foundations (including piles) on dry land well above flood water elevations.

8: Bridge foundations determined to be stable for assessed or calculated scour conditions. Scour is determined to be above top of footing. (Example A) by assessment i.e., bridge foundation are on rock formations that have been determined to resist scour within the service life of the bridge), by calculation or by installation of properly designed countermeasures (see HEC 23).

7: Countermeasures have been installed to mitigate an existing problem with scour and to reduce the risk of bridge failure during a flood event. Instructions contained in a plan of action have been implemented to reduce the risk to users from a bridge failure during or immediately after a food event.

6: Scour calculation/evaluation has not been made. (Use only to describe case where bridge has not yet been evaluated for scour potential.)

5: Bridge foundations determined to be stable for assessed or calculated scour conditions. Scour is determined to be within the limits of footing or piles (Exam. B) By assessment i.e., bridge foundations are on rock formations that have been determined to resist scour within the service life of the bridge), by calculations or by installation of properly designed countermeasures (see HEC 23).

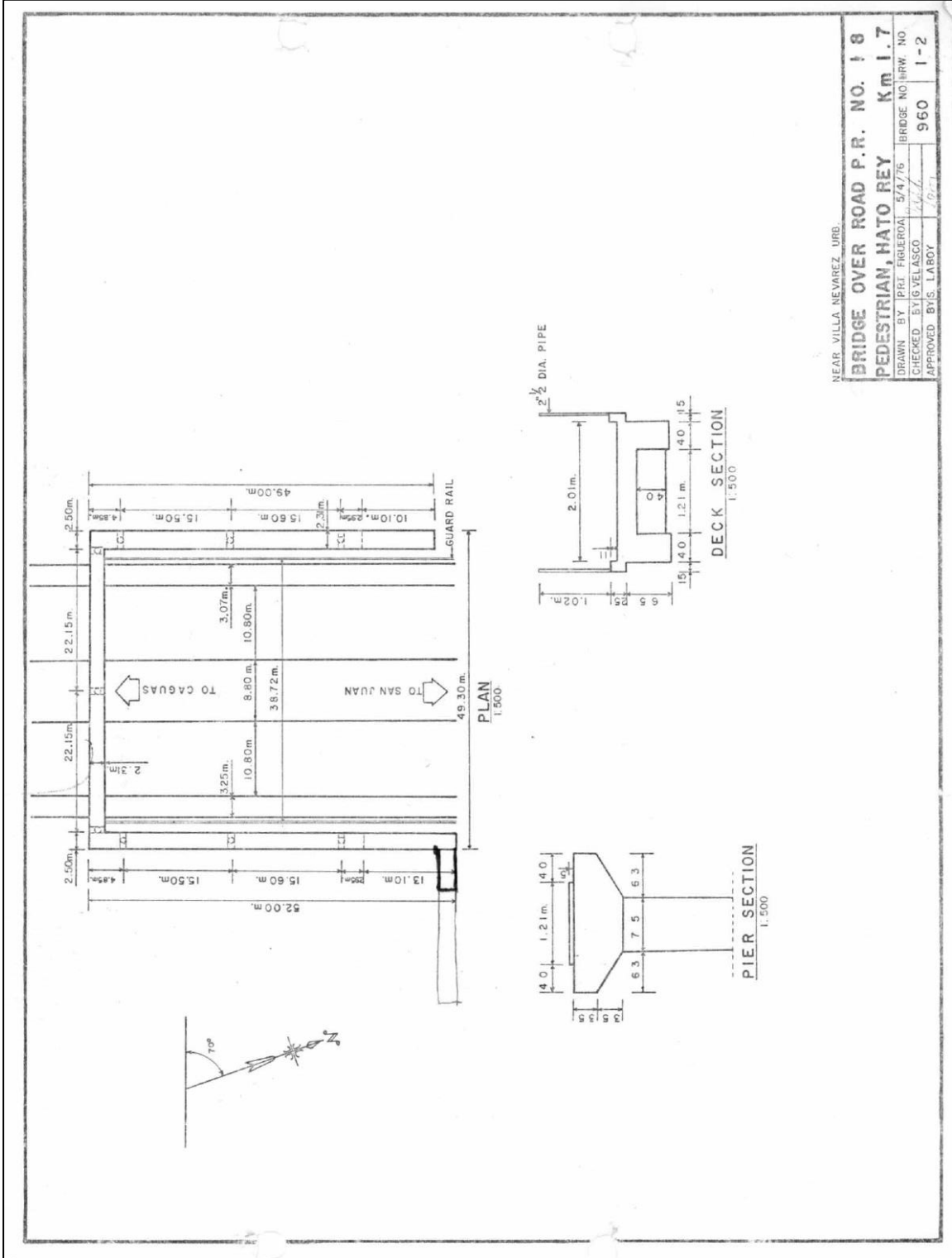
4: Bridge foundations determined to be stable for assessed or calculated scour conditions; field review indicates action is required to protect exposed foundation (see HEC 23).

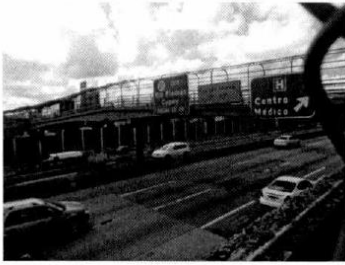
3: Bridge is scour critical; bridge foundations determined to be unstable for assessed or calculated scour conditions: Scour within limits of footing or piles. (Example B) or Scour below spread-footing base or piles tips. (Example C)

2: Bridge is scour critical; field review indicates that extensive scour has occurred at bridge foundations, which are determined to be unstable by: (a comparison of calculated scour and observed scour during the bridge inspection) or (an engineering evaluation of the observed scour condition reported by the bridge inspector in Item 60)

1: Bridge is scour critical; field review indicates that failure of piers/abutments is imminent. Bridge is closed to traffic. Failure is imminent based on: (a comparison of calculated scour and observed scour during the bridge inspection) or (an engineering evaluation of the observed scour condition reported by the bridge inspector in Item 60).

0: Bridge is scour critical. Bridge has failed and is closed to traffic.





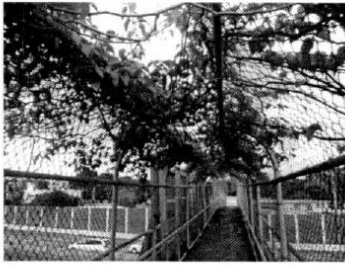
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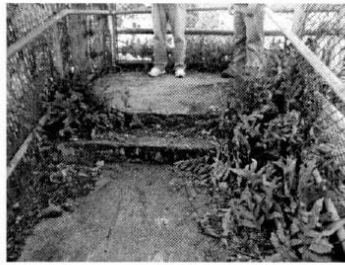
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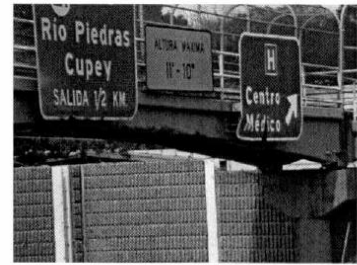
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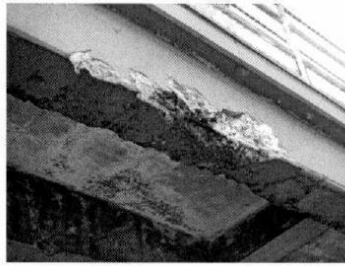
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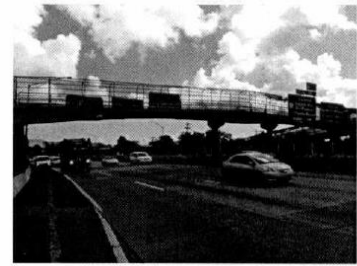
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YourState Department of Transportation **Bureau of Bridges and Structures**
Bridge Maintenance

Bridge Inspection Report

Bridge Key: 009601 Agency ID: 009601 Sufficiency Rating: -1.0

IDENTIFICATION

State 1: 72 Puerto Rico Struc Num 8: 009601
 Facility Carried 7: PR 18 Location 9: 0.5 KM N INT PR21 & PR18
 Rte (On/Under)5A: One Route Under Rte. Signing Prefix 5B: 2 U.S. Numbered
 Level of Service 5C: 1 Mainline Rte. Number 5D: 10000
 Directional Suffix 5E: 1 North % Responsibility: NA
 SHD District 2: SAN JUAN County Code 3: San Juan
 Place Code 4: 78770 Kilometer Post 11: 01.7 km
 Feature Intersected 6: PEDESTRIAN WALKWAY
 Latitude 16: 18d 23' 24" Longitude 17: 068d 04' 36"
 Border Bridge Code 98: Unknown (P)
 Border Bridge Number 99: NA

INSPECTION

Frequency 91: 24 months Inspection Date 90: 8/12/2002 Next Inspection: 08/12/2004
 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA
 UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 24 months Element Inspection Date: 08/12/2002 Next Elem. Insp. Due: 08/12/2004

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: 0 Number of Spans Main Unit 45: 2
 Main Span Material/Design 43A/B: 2 Concrete Continuous 04
 Deck Type 107: 1 Concrete-Cast-in-Place
 Wearing Surface 108A: 1 Monolithic Concrete
 Membrane 108B: 0 None
 Deck Protection 108C: None

CLASSIFICATION

Defense Highway 100: 2 Over/under STRAHNET Parallel Structure 101: No || bridge exists
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Unknown (NBI)
 Highway System 104: 0 Not on NHS NBIS Length 112: Long Enough
 Toll Facility 20: 3 On free road Functional Class 28: 12 Urban Fwy/Expwy
 Historical Significance 37: 5 Not eligible for NRHP
 Owner 22: 01 01 State Highway Agency
 Custodian 21: 01 01 State Highway Agency

AGE AND SERVICE

Year Built 27: 1967 Year Reconstructed 106: Unknown
 Type of Service on 42A: 3 Pedestrian-bicycle
 Type of Service under 42B: 1 Highway
 Lanes on 28A: Unknown Lanes Under 28B: 9 Detour Length 19: 00 km
 ADT 29: 178,900 Truck ADT 106: 4 % Year of ADT 30: 2000

CONDITION

Deck 58: 8 Satisfactory Super 59: 5 Fair Sub 60: 6 Satisfactory
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

GEOMETRIC DATA

Length Max Span 46: 22.20 m Structure Length 49: 49.30 m
 Curb/Sdwk Width L 50A: 0.00 m Curb/Sidewalk Width R 50B: 0.00 m
 Width Curb to Curb 51: 2.10 m Width Out to Out 52: 2.30 m
 Approach Roadway Width 32: 44.30 m Median 33: 0 No median (w/ shoulders)
 Deck Area:
 Skew 34: 0.00 * Structure Flared 35: 0 No flare
 Minimum Vertical Clearance Over Bridge 53: 99.99 m
 Minimum Vertical Underclearance Reference 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 04.44 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 03.30 m
 Minimum Lateral Underclearance L 55: 02.60 m

LOAD RATING AND POSTING

Inventory Rating Method 65: 2 AS Allowable Stress Operating Rating Method 63 2 AS Allowable Stress
 Inventory Rating 66: MS44.4 Operating Rating 64: MS44.4
 Design Load 31: 7 Pedestrian Posting 70: 5 At/Above Legal Loads
 Posting status 41: A Open, no restriction

ELEMNT CONDITION STATE DATA

Str Unit	Elm/Env	Description	Units	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5
2	12/3	Bare Concrete Deck	sq.m.	1,539	0 %	0	100 %	1,539	0 %	0	0 %	0	0 %	0
2	110/3	R/Conc Open Girder	m.	99	91 %	90	9 %	9	0 %	0	0 %	0	0 %	0
2	210/3	R/Conc Pier Wall	m.	18	100 %	18	0 %	0	0 %	0	0 %	0	0 %	0
2	234/3	R/Conc Cap	m.	26	100 %	26	0 %	0	0 %	0	0 %	0	0 %	0
2	358/3	Deck Cracking SmFlag	ea.	1	100 %	1	0 %	0	0 %	0	0 %	0	0 %	0

APPRAISAL

Bridge Rail 36A: N N/A or not required Approach Rail 36C: N N/A or not required
 Transition 36B: N N/A or not required Approach Rail Ends 36D: N N/A or not required
 Str. Evaluation 67: Unknown (NBI) Deck Geometry 68: Unknown (NBI)
 Underclearance, Vertical and Horizontal 69: Unknown (NBI)
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: 6 Equal Min Criteria
 Scour Critical 113: N Not Over Waterway

PROPOSED IMPROVEMENTS

Bridge Cost 94: \$ 0 Type of Work 75: 35 Rehabilitate-gen.
 Roadway Cost 95: \$ 0 Length of Improvement 78: 49.30 m
 Total Cost 96: \$ 12,000 Future ADT 114: 216,469
 Year of Cost Estimate 67: 2000 Year of Future ADT 115: 2020

NAVIGATION DATA

Navigation Control 38: N NA-no waterway
 Vertical Clearance 39: 0.00 m Horizontal Clearance 40: 0.00 m
 Pier Protection 111: Unknown (NBI) Lift Bridge Vertical Clearance 116:

INSP002_Inspect_Report_Metric Agency ID: 009601 Mon 10/7/2002 11:16:15
 Page 1 of 3

Bridge Inspection Report

Str Unit	Elem/Erv	Description	Element Notes
2	12/3	Concrete Deck - Bare	
2	110/3	Reinforced Conc Open Girder/Bea	
2	210/3	Reinforced Conc Pier Wall	
2	234/3	Reinforced Conc Cap	
2	358/3	Deck Cracking	

BRIDGE NOTES

CULVERT

PAST INSPECTION

Inspection Date: 08/12/2002 Type: 1 Regular NBI
 Inspector: Pontis Pontis User Key: Pontis - Pontis Poi
 Scope:
 NBI: Other: Element:
 Underwater: Fracture Critical:

INSPECTION NOTES

PONTIS inspection comments -
 Structure 009601 -
 Date 2002-08-12 -
 Previous comments > < none >

PAST INSPECTION

Inspection Date: 02/01/2000 Type: 1 Regular NBI
 Inspector: -1 Pontis User Key: Pontis - Pontis Poi
 Scope:
 NBI: Other: Element:
 Underwater: Fracture Critical:

INSPECTION NOTES

C.3 February 9, 2000

<p>ENGINEER: HERIBERTO GONZALEZ ASISTANTS: LUIS QUINONEZ : ALFREDO ERAZO : RAMON RODRIGUEZ : JAIME RIVAS</p> <p>EVALUATION DATE: 09-FEB-2000</p>	<p>PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REINSPECTION & EVALUATION REPORT</p> <p>BRIDGE No.: 00960 FEDERAL SYSTEM</p> <p>ROAD No.: PR 18 KM. No.: 0001.700</p>	<p style="text-align: right;">PAGE 1 OF 7</p> <p>EQUIPMENT</p> <p>BUS X LADDERS BOAT CAMERA X UNDERWATER SNOOPER # SNOOPER OPERATOR</p> <p>PLAQUE ID YES NO No. FILM No.</p>
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-----IDENTIFICATION-----

1- STATE :	Commonwealth of Puerto Rico	721
2- HIGHWAY AGENCY DISTRICT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-1-1-00001-0	221100001
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 18	
8- STRUCTURE NUMBER :	BRIDGE 960 1 OF 1	009601
9- LOCATION :	0.5 KM N INT PR21 & PR18	0444
10- INVENTORY ROUTE, MINIMUM VERTICAL CLEARANCE :	4.44 MT (14' - 7")	0001700
11- KILOMETERPOINT :	1.70	1
12- BASE HIGHWAY NETWORK :		000000001800
13- LRS INVENTORY ROUTE, SUBROUTE NUMBER :		18232400
16- LATITUDE :	18 DEGREES 23.4 MINUTES	066043600
17- LONGITUDE :	66 DEGREES 04.6 MINUTES	000
19- BYPASS, DETOUR LENGTH (NEAREST KILOMETER) :	STRUCTURE OVER HIGHWAY	

-----CLASSIFICATION-----

20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	INTERSTATE	12

-----AGE AND SERVICE-----

27- YEAR BUILT :	1967	1967
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / 9 LANES UNDER	0009
29- A.D.T. OF INVENTORY ROUTE :	169,700	169700
30- YEAR OF A.D.T. :	1998	1998

-----STRUCTURE DATA-----

31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	44.30 MT (145.30 FT)	0443
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	N/A	0000
40- NAVIGATION HORIZONTAL CLEARANCE :	N/A	00000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
43- STRUCTURE TYPE, MAIN :	CONCRETE CONTINUOUS TEE BEAMS	204
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	2	002
46- NUMBER OF APPROACH SPAN :	NONE	0000

BRIDGE No.: 00960

BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

47- INV. ROUTE, TOTAL HORIZ. CLEARANCE : 17.80mt ~~20.84~~ MT (68.36 FT) 178 JK
~~200~~
 48- LENGTH OF MAXIMUM SPAN : 22.15 MT (72.65 FT) 00222
 49- STRUCTURE LENGTH : 49.30 MT (161.70 FT) 000493
 50- CURB OR SIDEWALK WIDTHS : N/A 000000

-----STRUCTURE DATA-----

51- BRIDGE ROADWAY WIDTH, CURB TO CURB : 2.10 MT (6.59 FT) 0021
 52- DECK WIDTH, OUT TO OUT : 2.31 MT (7.58 FT) 0023
 53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY : UNLIMITED 9999
 54- MINIMUM VERTICAL UNDERCLEARANCE : 4.44 MT (14' - 7") H0444
 55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT : 3.25 MT (10.8 FT) H033
 56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT : 2.55 MT (8.36 FT) 026

-----CONDITION-----

58- DECK: FAIR CONDITION RATING: 6 DECK COMMENTS:
 58.1 WEARING SURFACE: MATERIAL: CONCRETE CONDITION : 5 DECK SURFACE HAS PLASTER DETACHMENT, SCALING AND FINE TO THICKNESS: MEDIUM TRANSVERSAL CRACKS. THERE ARE 4 PERFORATIONS (20 CMS. DIAM.), TWO OVER EACH NEW PIER THAT NEED TO BE SEALED TO PREVENT AN ACCIDENT. HANDICAP RAMP LOWER THAN DECK SURFACE MAKING IMPOSSIBLE ACCESS ON WHEELCHAIR. NORTH-EAST RAMP DISPLACED 10 CMS., ALSO RAILING INCLINED TOWARD DECK.
 DETERIORATION: X YES NO DRAINAGE: X ADEQUATE INADEQUATE
 PONDING: YES X NO SAFETY: X YES NO
 58.2 SLAB OR PLATE: MATERIAL: CONCRETE CONDITION 6
 X CRACKING: X FINE MEDIUM OPEN
 TOP BOTTOM
 SPALLING: X | S < 1 in. DEEP x 6 in. & L > 1 in. x 6 in. & |
 SCALING : X | L to 1/4 in., M 1/4 in. - 1/2 in., H 1/2 in. - 1 in., S > 1 |

58.3 MOVEMENT: DECK TO BACKWALL cm. DECK TO APPROACH SLAB cm.
 EFFLORESCENCE EXUDATION RUST STAINS CORROSION: LIGHT MODERATE SEVERE

	(58.4)	(58.5)	(58.6)	(58.7)	(58.8)
	CURBS	MEDIAN	SIDEWALKS	PARAPETS	RAILING
MATERIAL					G/STEEL
CONDITION RATING					4
HEIGHT LOSS (cm)					
JOINTS					OK
DRAINAGE					PO
ALIGNMENT					
CORROSION (L-M-S)					N
PAINT: G-GOOD, F-FAIR, P-POOR, N-NONE, U-USELESS TO PAINT					
SAFETY					NO
CRACKING (F-M-O)					
SPALLING (S-L)					
SCALING (L-M-H-S)					

58.9 LIGHTING STANDARDS: MATERIAL: CONDITION: FUNCTIONING: YES NO
 58.10 UTILITIES: TYPE: SIZE: SAFETY:
 58.11 JOINTS: EXPANSION CONSTRUCTION CONDITION: FUNCTIONING: YES NO
 LEAKING: YES NO CRACKING: FINE MEDIUM OPEN SPALLING: SMALL LARGE
 58.12 DRAINS AND SCUPPERS: MATERIAL: CONDITION: FUNCTIONING: YES NO

DGE No.: 00960

59- SUPERSTRUCTURE: FAIR CONDITION RATING: 5

59.1 BEARING DEVICES: TYPE: 6 FIXED MOVABLE OUT OF PLUMB
PAINT: GOOD FAIR POOR NONE USELESS TO PAINT
CONDITION: FUNCTIONING: YES NO

59.2 BRIDGE SEATS, PEDESTAL, GROUT PADS, ABUTMENTS OR PIER SEATS WHERE BEAMS BEAR DIRECTLY ON CONCRETE
CONDITION: 6 CRACKING: X F M O SPALLING: X S L
SCALING: L M H S CRUSHING:

59.3 STEEL-BEAMS: TYPE: DIMENSIONS (height; width; thickness):
CONDITION: CORROSION: L M S PAINT: G F P N U

59.4 CONCRETE-BOX BEAMS, I-BEAMS, SLAB, T-BEAM, TYPE: T-BEAM
CONDITION: 5 CRACKING: X F M O SPALLING: S X L
SCALING: L M H S

59.5 TRUSSES: TYPE: CONDITION: CORROSION: L M S
DAMAGE: DEFORMATION: PAINT: G F P N U
A=TRUSS B=UPPER & LOWER LATERAL BRACING C=PORTALS D=FLOOR BEAMS E=STRINGERS

59.6 DRAINAGE: TYPE: CONDITION: FUNCTIONING: YES NO

59.7 HINGES: CONDITION: FUNCTIONING: YES NO MOVEMENT: YES NO
ALIGNMENT: GOOD BAD PAINT: G F P N U

59.8 DEFLECTIONS: X NORMAL EXCESSIVE cms.

59.9 VIBRATIONS: X MINIMAL MODERATE EXCESSIVE

SUPERSTRUCTURE COMMENTS:
BEAMS TOWARD CAGUAS LANES WITH BIG DETACHMENTS OF CONCRETE DUE TO TRUCK IMPACT.

60- SUBSTRUCTURE: SATISFACTORY RATING: 6

	ABUTMENTS			PIERS OR NON PILES BENTS					PILES BENTS			
	60.1.1	60.1.2	60.1.3	60.1.4	60.2.1	60.2.2	60.2.3	60.2.4	60.2.5	60.4.1	60.4.2	60.4.3
	WINGWALL	BREAST & BACKWALL	FOOTING	PILES	CAPS	BRACING	COLUMNS	FOOTINGS	PILES	CAPS	BRACING	PILES
MATERIAL					CONCRETE		CONCRETE					
CONDITION RATING					5		6					
CRACKING (F-M-O)					S		S					
SPALLING (S-L)					L		L					
SCALING (L-M-H-S)												
CORROSION (L-M-S)					N		N					
PAINT (G-P-P-N-U)					NO		NO					
MOVEMENT					NO		NO					
EROSION					NO		NO					
UNDERMINING												

60- SUBSTRUCTURE COMMENTS:
EAST PIER CAP HAS MEDIUM HORIZONTAL CRACK.

SYSTEM

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

BRIDGE No.: 00960

61- CHANNEL & CHANNEL PROTECTION : N/A
61.1 CHANNEL SCOUR (EXTENT) :
61.2 EMBANKMENT EROSION (EXTENT) :

RATING : N

CHANNEL COMMENTS:

61.3 PROTECTIVE DEVICE
61.4 FENDER SYSTEM ----
61.5 RIP RAP
61.6 SPUR DIKES, JETTIES

TYPE	MATERIAL	CONDITION RATING	FUNCTIONING	
			YES	NO

61.7 OBSTRUCTION (DEBRIS, GROWTHS):
61.8 CHANNEL CHANGE: YES NO
DETRIMENTAL: YES NO IF YES, EXPLAIN
61.9 ADEQUATE WATERWAY: YES NO IF NO, EXPLAIN
61.10 SURROUNDING AREA: FLOODING: YES NO IF YES, EXPLAIN
EROSION: YES NO IF YES, EXPLAIN
61.11 LOCATION OF PIERS AND/OR ABUTMENTS: DETRIMENTAL: YES NO
IF YES, EXPLAIN
61.12 OTHER FEATURES THAT MAY AFFECT STRUCTURE:

62- CULVERT AND RETAINING WALLS : N/A

RATING : N

CULVERT COMMENTS:

	62.1 BARREL	62.2 HEADWALL	62.3 CUT-OFF WALL	62.4 RETAINING WALL
MATERIAL				
CONDITION RATING				
FUNCTIONING (Y-N)				
CRACKING (F-M-O)				
SPALLING (S-L)				
SCALING (L-M-H-S)				
CORROSION (L-M-S)				
SETTLEMENT (Y-N)				
ALIGNMENT (Y-N)				
PAINT (G-F-P-N-U)				

63- METHOD USED TO DETERMINE OPERATING RATING ..

64- OPERATING RATING : PEDESTRIAN

2
800

65- METHOD USED TO DETERMINE INVENTORY RATING ..

66- INVENTORY RATING : PEDESTRIAN

2
800

AL SYSTEM
BRIDGE NO. : 00960

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

-----PROPOSED IMPROVEMENT-----

75- TYPE OF WORK : REHABILITATION 352
76- LENGTH OF STRUCTURE IMPROVEMENT : 49.30 MT (161.70 FT) 000493

-----INSPECTIONS-----

89- RESERVED --
90- INSPECTION DATE : FEBRUARY-09-2000 0200
91- DESIGNATED INSPECTION FREQUENCY : 24 MONTHS 24
92- CRITICAL FEATURE INSPECTION : N/A. N N N
93- CRITICAL FEAT. INSPECT. DATE: N/A
FRACTURE CRITICAL UNDERWATER OTHER

-----IMPROVEMENT COST-----

94- BRIDGE IMPROVEMENT COST : N/A
95- ROADWAY IMPROVEMENT COST : N/A
96- TOTAL PROJECT COST : (49.30X2.31X\$1035X10%)=\$11,787 000012
97- YEAR OF IMPROVEMENT COST ESTIMATED : 2000 2000

Back to 000000

--CLASSIFICATION AND STRUCTURE DATA--

98- BORDER BRIDGE : --
99- BORDER BRIDGE STRUCTURE NUMBER : --
100- STRAHNET HIGHWAY DESIGNATION : DEFENSE HIGHWAY N
101- PARALLEL STRUCTURE DESIGNATION : NO PARALLEL STRUCTURE 0
102- DIRECTION OF TRAFFIC : TRAFFIC NOT CARRIED
103- TEMPORARY STRUCTURE DESIGNATION : N/A
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE : N.H.S. 0
105- FEDERAL LANDS HIGHWAYS : NO RECONSTRUCTION 0000
106- YEAR RECONSTRUCTION : CONCRETE CAST IN PLACE 1
107- DECK STRUCTURE TYPE : CONCRETE NONE NONE 100
108- WEARING SURF./ PROTECT. SYSTEM : 4 04
109- AVERAGE DAILY TRUCK TRAFFIC : NOT PART OF NETWORK 0
110- DESIGNATED NATIONAL NETWORK : N/A
111- PIER OR ABUT. PROTECTION (NAVIGATION) : YES Y
112- NBIS BRIDGE LENGTH : BRIDGE OVER HIGHWAY N
113- SCOUR CRITICAL BRIDGE : 239,277 239277
114- FUTURE AVERAGE DAILY TRAFFIC : 2018 2018
115- YR. OF FUTURE A.D.T. : N/A
116- MINIM. NAVIG. VERT. CLEARANCE : N/A
VERTICAL LIFT BRIDGE :
117- SUFFICIENCY RATING :
125- PRIORITY RATING: 09-FEB-2000 020900
127- EVALUATION DATE :
130- CRITICAL FRACTURE INSP. DATE :

102

ENGINEER : HERIBERTO GONZALEZ

SYSTEM

IDGE NO. :00960

AUXILIARY ITEMS

1-SIGNS

TYPE:

MATERIAL:

CONDITION:

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

PAGE 7 OF 7

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE
ITEM #67.

C.4 March 20, 1998

<p>ENGINEER: HERIBERTO GONZALEZ ASSISTANTS: LUIS QUINONEZ : ALFREDO ERAZO : : : EVALUATION DATE: MAR-20-1998</p>	<p>PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REINSPECTION & EVALUATION REPORT</p> <p>BRIDGE No.: 00960 FEDERAL SYSTEM</p> <p>ROAD No.: PR 18 KM. No.: 0001.700</p>	<p style="text-align: right;">PAGE 1 OF 7</p> <p>EQUIPMENT</p> <p>BUS X LADDERS BOAT CAMERA X UNDERWATER SNOOPER # SNOOPER OPERATOR</p> <p>PLAQUE ID YES NO No. FILM No.</p>
--	--	--

-----IDENTIFICATION-----

1- STATE :	Commonwealth of Puerto Rico	721
2- HIGHWAY AGENCY DISTRICT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-1-1-00001-0	211000010
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 18	PR 18
8- STRUCTURE NUMBER :	BRIDGE 960 1 OF 1	009601
9- LOCATION :	0.5 KM N INT PR21 & PR18	0.5 KM N INT PR21 & PR18
10- INVENTORY ROUTE, MINIMUM VERTICAL CLEARANCE :	4.44 MT (14' - 7")	0444
11- KILOMETERPOINT :	1.70	0001700
12- BASE HIGHWAY NETWORK :		1
13- LRS INVENTORY ROUTE, SUBROUTE NUMBER :		000000001800
16- LATITUDE :	18 DEGREES 23.4 MINUTES	18232400
17- LONGITUDE :	66 DEGREES 04.6 MINUTES	066043600
19- BYPASS, DETOUR LENGTH (NEAREST KILOMETER) :	STRUCTURE OVER HIGHWAY	000

-----CLASSIFICATION-----

20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	INTERSTATE	11

-----AGE AND SERVICE-----

27- YEAR BUILT :	1967	1967
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / ¹⁸ LANES UNDER	0000
29- A.D.T. OF INVENTORY ROUTE :	169,700	169700
30- YEAR OF A.D.T. :	1996	1996

-----STRUCTURE DATA-----

31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	44.30 MT (145.30 FT)	04430 <i>ja</i>
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	N/A	0000
40- NAVIGATION HORIZONTAL CLEARANCE :	N/A	00000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
43- STRUCTURE TYPE, MAIN :	CONCRETE CONTINUOUS TEE BEAMS	204
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	3	002 <i>003</i>
46- NUMBER OF APPROACH SPAN :	NONE	0000

BRIDGE No.: 00960

47- INV. ROUTE, TOTAL HORIZ. CLEARANCE : 20.84 MT (68.36 FT)
 48- LENGTH OF MAXIMUM SPAN : 22-15 MT- 080 (72.65 FT)
 49- STRUCTURE LENGTH : 49.30 MT (161.70 FT)
 50- CURB OR SIDEWALK WIDTHS : N/A

208
 0022
 00049
 000000

-----STRUCTURE DATA-----

51- BRIDGE ROADWAY WIDTH, CURB TO CURB : 2.10 MT (6.59 FT)
 52- DECK WIDTH, OUT TO OUT : 2.31 MT (7.58 FT)
 53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY : UNLIMITED
 54- MINIMUM VERTICAL UNDERCLEARANCE : 4.44 MT (14' - 7")
 55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT : 3.25 MT (10.8 FT)
 56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT : 2.55 MT (8.36 FT)

0021
 0023
 9999
 H0444
 H033
 026

-----CONDITION-----

58- DECK: SATISFACTORY RATING: 6

DECK COMMENTS:

58.1 WEARING SURFACE: MATERIAL: CONCRETE CONDITION: 6
 THICKNESS:
 DETERIORATION: X YES NO DRAINAGE: ADEQUATE INADEQUATE
 PONDING: YES X NO SAFETY: YES NO

DETACHMENTS OF CONCRETE WITH EXPOSED RE
 INFORCEMENT, FINE
 TRANSVERSAL CRACKS WITH EFFLORESCENCES

58.2 SLAB OR PLATE: MATERIAL: CONCRETE CONDITION 6
 X CRACKING: X FINE MEDIUM OPEN

TOP BOTTOM

SPALLING: X | S < 1 in. DEEP x 6 in. & L > 1 in. x 6 in. &
 SCALING: X | L to 1/4 in., M 1/4 in. - 1/2 in., H 1/2 in. - 1 in., S > 1

EFFLORESCENCE EXUDATION RUST STAINS CORROSION: LIGHT MODERATE SEVERE

58.3 MOVEMENT: DECK TO BACKWALL cm. DECK TO APPROACH SLAB cm.

	(58.4)	(58.5)	(58.6)	(58.7)	(58.8)
	CURBS	MEDIAN	SIDEWALKS	PARAPETS	RAILING
MATERIAL					G/STEEL
CONDITION RATING					7
HEIGHT LOSS (cm)					
JOINTS					OK
DRAINAGE					
ALIGNMENT					OK
CORROSION (L-M-S)					L
PAINT: G-GOOD, F-FAIR, P-POOR, N-NONE					F
U-USELESS TO PAINT					
SAFETY					YES
CRACKING (F-M-O)					
SPALLING (S-L)					
SCALING (L-M-H-S)					

58.9 LIGHTING STANDARDS: MATERIAL: CONDITION: FUNCTIONING: YES NO

58.10 UTILITIES: TYPE: SIZE: SAFETY:

58.11 JOINTS: EXPANSION CONSTRUCTION CONDITION: FUNCTIONING: YES NO

LEAKING: YES NO CRACKING: FINE MEDIUM OPEN SPALLING: SMALL LARGE

58.12 DRAINS AND SCUPPERS: MATERIAL: CONDITION: FUNCTIONING: YES NO

BRIDGE No.: 00960

59- SUPERSTRUCTURE: SATISFACTORY

RATING: 6

- 59.1 BEARING DEVICES: TYPE FIXED MOVABLE OUT OF PLUMB
 PAINT: GOOD FAIR POOR NONE USELESS TO PAINT
 CONDITION: FUNCTIONING: YES NO
- 59.2 BRIDGE SEATS, PEDESTAL, GROUT PADS, ABUTMENTS OR PIER SEATS WHERE BEAMS BEAR DIRECTLY ON CONCRETE
 CONDITION: 6 CRACKING: X F M O SPALLING: X S L
 SCALING: L M H S CRUSHING:
- 59.3 STEEL-BEAMS: TYPE: DIMENSIONS (height; width; thickness):
 CORROSION: L M S PAINT: G F P N U
- 59.4 CONCRETE-BOX BEAMS, I-BEAMS, SLAB, T-BEAM, TYPE: T-BEAM
 CONDITION: S CRACKING: X F M O SPALLING: S X L
 SCALING: L M H S
- 59.5 TRUSSES: TYPE: CONDITION: CORROSION: L M S
 DAMAGE: DEFORMATION: PAINT: G F P N U
 A=TRUSS B=UPPER & LOWER LATERAL BRACING C=PORTALS D=FLOOR BEAMS E*=STRINGERS
- 59.6 DRAINAGE: TYPE: CONDITION: FUNCTIONING: YES NO
- 59.7 HINGES: CONDITION: FUNCTIONING: YES NO MOVEMENT: YES NO
 ALIGNMENT: GOOD BAD PAINT: G F P N U
- 59.8 DEFLECTIONS: X NORMAL EXCESSIVE cms.
- 59.9 VIBRATIONS: X MINIMAL MODERATE EXCESSIVE

SUPERSTRUCTURE COMMENTS:
 BEAMS TOWARD CAGUAS LANES WITH BIG DETACHMENTS OF CONCRETE DUE TO TRUCK IMPACT.

60- SUBSTRUCTURE: SATISFACTORY

RATING: 6

	← ABUTMENTS →				← PIERS OR NON PILES BENTS →					← PILES BENTS →		
	60.1.1	60.1.2	60.1.3	60.1.4	60.2.1	60.2.2	60.2.3	60.2.4	60.2.5	60.4.1	60.4.2	60.4.3
	WINGWALL	BREAST &	FOOTING	PILES	CAPS	BRACING	COLUMNS	FOOTINGS	PILES	CAPS	BRACING	PILES
	BACKWALL											
MATERIAL					CONCRETE		CONCRETE					
CONDITION RATING					F		6					
CRACKING (F-M-O)					S		F					
SPALLING (S-L)							S					
SCALING (L-M-H-S)												
CORROSION (L-M-S)												
PAINT (G-F-P-N-U)					F		F					
MOVEMENT					NO		NO					
EROSION							NO					
UNDERMINING							NO					

60- SUBSTRUCTURE COMMENTS:

GENERAL SYSTEM
 BRIDGE No.: 00960

PUERTO RICO HIGHWAY AUTHORITY
 BRIDGE STUDIES AND EVALUATION OFFICE
 BRIDGE REINSPECTION & EVALUATION REPORT

61- CHANNEL & CHANNEL PROTECTION : N/A
 61.1 CHANNEL SCOUR (EXTENT) :
 61.2 EMBANKMENT EROSION (EXTENT) :

RATING : N

CHANNEL COMMENTS:

61.3 PROTECTIVE DEVICE
 61.4 FENDER SYSTEM ----
 61.5 RIP RAP
 61.6 SPUR DIKES, JETTIES

TYPE	MATERIAL	CONDITION RATING	FUNCTIONING	
			YES	NO

61.7 OBSTRUCTION (DEBRIS, GROWTHS):
 61.8 CHANNEL CHANGE: YES NO
 DETRIMENTAL: YES NO IF YES, EXPLAIN
 61.9 ADEQUATE WATERWAY: YES NO IF NO, EXPLAIN
 61.10 SURROUNDING AREA: FLOODING: YES NO IF YES, EXPLAIN
 EROSION: YES NO IF YES, EXPLAIN
 61.11 LOCATION OF PIERS AND/OR ABUTMENTS: DETRIMENTAL: YES NO
 IF YES, EXPLAIN
 61.12 OTHER FEATURES THAT MAY AFFECT STRUCTURE:

62- CULVERT AND RETAINING WALLS : N/A

RATING : N

CULVERT COMMENTS:

	62.1 BARREL	62.2 HEADWALL	62.3 CUT-OFF WALL	62.4 RETAINING WALL
MATERIAL				
CONDITION RATING				
FUNCTIONING (Y-N)				
CRACKING (F-M-O)				
SPALLING (S-L)				
SCALING (L-M-H-S)				
CORROSION (L-M-S)				
SETTLEMENT (Y-N)				
ALIGNMENT (Y-N)				
PAINT (G-F-P-N-U)				

63- METHOD USED TO DETERMINE OPERATING RATING ..		2
64- OPERATING RATING :	PEDESTRIAN	800
65- METHOD USED TO DETERMINE INVENTORY RATING ..		2
66- INVENTORY RATING :	PEDESTRIAN	800

PUERTO RICO HIGHWAY AUTHORITY
 BRIDGE STUDIES AND EVALUATION OFFICE
 BRIDGE REINSPECTION & EVALUATION REPORT

SYSTEM
 100960
 TYPE OF WORK :
 LENGTH OF STRUCTURE IMPROVEMENT :

-----PROPOSED IMPROVEMENT-----

REHABILITATION
 49.30 MT (161.70 FT)

352
 0004913 *ja*

-----INSPECTIONS-----

RESERVED
 INSPECTION DATE : MARCH-20-1998
 DESIGNATED INSPECTION FREQUENCY : 24 MONTHS
 CRITICAL FEATURE INSPECTION : N/A
 CRITICAL FEAT. INSPECT. DATE : N/A
 FRACTURE CRITICAL UNDERWATER OTHER

--
 0398
 24
 N N N

-----IMPROVEMENT COST-----

BRIDGE IMPROVEMENT COST : N/A
 ROADWAY IMPROVEMENT COST : N/A
 TOTAL PROJECT COST : (49.30X2.31X\$1035X10%)=\$11,787
 YEAR OF IMPROVEMENT COST ESTIMATED : 1998

000009
 000000
 000012
 1998

==CLASSIFICATION AND STRUCTURE DATA==

BORDER BRIDGE :
 BORDER BRIDGE STRUCTURE NUMBER :
 STRAHNET HIGHWAY DESIGNATION : DEFENSE HIGHWAY
 PARALLEL STRUCTURE DESIGNATION : NO PARALLEL STRUCTURE
 DIRECTION OF TRAFFIC : TRAFFIC NOT CARRIED
 TEMPORARY STRUCTURE DESIGNATION : N/A
 HIGHWAY SYSTEM OF THE INVENTORY ROUTE : N.H.S.
 FEDERAL LANDS HIGHWAYS :
 YEAR RECONSTRUCTION : NO RECONSTRUCTION
 DECK STRUCTURE TYPE : CONCRETE CAST IN PLACE
 WEARING SURF./ PROTECT. SYSTEM : CONCRETE NONE NONE
 AVERAGE DAILY TRUCK TRAFFIC : 4
 DESIGNATED NATIONAL NETWORK : NOT PART OF NETWORK
 PIER OR ABUT. PROTECTION (NAVIGATION) : N/A
 NBIS BRIDGE LENGTH : YES
 SCOUR CRITICAL BRIDGE : BRIDGE OVER HIGHWAY
 FUTURE AVERAGE DAILY TRAFFIC : 239,277
 YR. OF FUTURE A.D.T. : 2016
 MINIM. NAVIG. VERT. CLEARANCE : N/A
 VERTICAL LIFT BRIDGE :
 SUFFICIENCY RATING :
 PRIORITY RATING :
 EVALUATION DATE : MAR-20-1998
 CRITICAL FRACTURE INSP. DATE :

--
 --
 1
 N
 1
 0
 0000
 1
 100
 04
 0
 Y
 N
 239277
 2016
 032098

ENGINEER : HERIBERTO GONZALEZ

AL SYSTEM
IDGE NO. :00960

FUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

AUXILIARY ITEMS
1-SIGNS
TYPE:
MATERIAL:
CONDITION:

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE
ITEM #67.

PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REPORT -----		PAGE 1 OF 3 DATE : 03/15/96 TIME : 09:56:53
FEDERAL SYSTEM	BRIDGE NO. : 00960 - ROAD NO. : PR 18	- KM. NO. : 001.700
-----IDENTIFICATION-----		
1- STATE :	Commonwealth of Puerto Rico	721
2- STATE HIGHWAY DEPARTMENT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-1-1-00001-0	211000010
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 18	PR 18
8- STRUCTURE NUMBER :	BRIDGE 960 1 OF 1	009601
9- LOCATION :	0.5 KM N INT PR21 & PR18	0.5 KM N INT PR21 & PR18
10- INV. ROUTE, MIN. CLEAR. (0.01m) :	4.44 MT (14' - 7")	1407
11- MILEPOINT :	1.70	001700
16- LATITUDE :	18 DEGREES 23.4 MINUTES	18234
17- LONGITUDE :	66 DEGREES 04.6 MINUTES	066046
19- BYPASS, DETOUR LENGTH (NEAREST MILE) :	STRUCTURE OVER HIGHWAY	00
-----CLASSIFICATION-----		
20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	INTERSTATE	11
-----AGE AND SERVICE-----		
27- YEAR BUILT :	1967	1967
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / 9 LANES UNDER	0009
29- A.D.T. OF INVENTORY ROUTE :	222,800	222800
30- YEAR OF A.D.T. :	1993	93
-----STRUCTURE DATA-----		
31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	44.30 MT (145.30 FT)	145
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	N/A	000
40- NAVIGATION HORIZONTAL CLEARANCE :	N/A	0000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
43- STRUCTURE TYPE, MAIN :	CONCRETE CONTINUOUS TEE BEAMS	204
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	2	002
46- NUMBER OF APPROACH SPAN :	NONE	0000
47- INV. ROUTE, TOTAL HORIZ. CLEARANCE :	20.84 MT (68.36 FT)	684
48- LENGTH OF MAXIMUM SPAN :	22.15 MT (72.65 FT)	0073
49- STRUCTURE LENGTH :	49.30 MT (161.70 FT)	000162
50- CURB OR SIDEWALK WIDTHS :	N/A	000000

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE

FEDERAL SYSTEM BRIDGE NO. : 00960 - ROAD NO. : PR 18 - KM. NO. : 001.700

-----STRUCTURE DATA-----

51- BRIDGE ROADWAY WIDTH, CURB TO CURB :	2.10 MT (6.59 FT)	0066
52- DECK WIDTH, OUT TO OUT :	2.31 MT (7.58 FT)	0076
53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY :	UNLIMITED	9999
54- MINIMUM VERTICAL UNDERCLEARANCE :	4.44 MT (14' - 7")	H1407
55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT :	3.25 MT (10.8 FT)	H108
56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT :	2.55 MT (8.36 FT)	084

-----CONDITION-----

	MATERIAL	CONDITION ANALYSIS	
58- DECK	CONCRETE	SATISFACTORY	6
59- SUPERSTRUCTURE	CONCRETE	SATISFACTORY	6
60- SUBSTRUCTURE	CONCRETE	SATISFACTORY	6
61- CHANNEL & CHANNEL PROTECTION	N/A	N/A	N
62- CULVERT	N/A	N/A	N
64- OPERATING RATING :		PEDESTRIAN	800
66- INVENTORY RATING :		PEDESTRIAN	800

67- STRUCTURAL EVALUATION : 6
WEARING SURFACE HAVE CONCRETE LOSE WITH EXPOSED STEEL. BOTTOM SLAB HAVE FINE TRANSVERSAL CRACKS WITH EFFLORESCENCES. BEARING HAVE LIGHT CORROSION. BEAMS AT CAGUAS WAY HAVE BIG SPALLING WITH EXPOSED STEEL BY IMPACT. RAMP AT MEDICAL CENTER SIDE HAVE LARGE SPALLING WITH EXPOSED STEEL.

68- DECK GEOMETRY : N
N/A.

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : 4
MEETS MINIMUM TOLERABLE LIMITS TO BE LEFT IN PLACE AS IS.

70- BRIDGE POSTING : N/A.

71- WATERWAY ADEQUACY : N
N/A.

72- APPROACH ROADWAY ALIGNMENT : N/A.

-----PROPOSED IMPROVEMENT-----

75- TYPE OF WORK :	REHABILITATION	352
76- LENGTH OF STRUCTURE IMPROVEMENT :	49.30 MT (161.70 FT)	000162

Handwritten signature/initials

Handwritten signature/initials

PUERTO RICO HIGHWAY AUTHORITY
 BRIDGE STUDIES AND EVALUATION OFFICE

FEDERAL SYSTEM

BRIDGE NO. : 00960 - ROAD NO. : PR 18

- KM. NO. : 001.700

-----INSPECTIONS-----

89- RESERVED --
 90- INSPECTION DATE : MARCH 1996. 0396
 91- DESIGNATED INSPECTION FREQUENCY : 24 MONTHS 24
 92- CRITICAL FEATURE INSPECTION : N/A. N N N
 93- CRITICAL FEAT. INSP. DATE: N/A

-----IMPROVEMENT COST-----

94- BRIDGE IMPROVEMENT COST : N/A 000009
 95- ROADWAY IMPROVEMENT COST : N/A 000000
 96- TOTAL PROJECT COST : (49.30X2.31X\$1035X10%)=\$11,787 000012
 97- YEAR OF IMPROVEMENT COST ESTIMATED : 1996 96

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE : --
 99- BORDER BRIDGE STRUCTURE NUMBER : --
 100- DEFENSE HIGHWAY DESIGNATION : DEFENSE HIGHWAY 1
 101- PARALLEL STRUCTURE DESIGNATION : NO PARALLEL STRUCTURE N
 102- DIRECTION OF TRAFFIC : TRAFFIC NOT CARRIED 3
 103- TEMPORARY STRUCTURE DESIGNATION : N/A
 104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE : N.H.S. 1
 105- RESERVED --
 106- YEAR RECONSTRUCTION : NO RECONSTRUCTION 0000
 107- DECK STRUCTURE TYPE : CONCRETE CAST IN PLACE 1
 108- WEARING SURF./ PROTECT. SYSTEM : CONCRETE NONE NONE 100
 109- AVERAGE DAILY TRUCK TRAFFIC : 4 04
 110- DESIGNATED NATIONAL NETWORK : NOT PART OF NETWORK 0
 111- PIER OR ABUT. PROTECTION (NAVIGATION) : N/A
 112- NBIS BRIDGE LENGTH : YES Y
 113- SCOUR CRITICAL BRIDGE : BRIDGE OVER HIGHWAY N
 114- FUTURE AVERAGE DAILY TRAFFIC : 369,900 369900
 115- YR. OF FUTURE A.D.T. : 2012 12
 116- MINIM. NAVIG. VERT. CLEARANCE : N/A
 117- SUFFICIENCY RATING : 031396 031396
 126- FIELD INSPECTION DATE : 031396 031396
 127- INSPECTION EVAL. DATE : 031396
 130- CRITICAL FRACTURE INSP. DATE :
 131- EVALUATOR ENGINEER :

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE
 ITEM #67.

C.6 September 30, 1992

PUERTO RICO HIGHWAY AUTHORITY HIGHWAY SYSTEM ADMINISTRATION OFFICE BRIDGE REPORT		PAGE 1 OF 3 DATE : 09/30/93 TIME : 08:43:47
FEDERAL SYSTEM	BRIDGE NO. : 00960 - ROAD NO. : PR 18	- KM. NO. : 004400
=====IDENTIFICATION=====		
1- STATE :	Commonwealth of Puerto Rico	721
2- STATE HIGHWAY DEPARTMENT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-1-1-00001-0	211000010
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 18	PR 18
8- STRUCTURE NUMBER :	BRIDGE 960 1 OF 1	009601
9- LOCATION :	0.5 KM N INT PR21 & PR18	0.5 KM N INT PR21 & PR18
11- MILEPOINT :	4.40	004400
16- LATITUDE :	18 DEGREES 23.4 MINUTES	18234
17- LONGITUDE :	66 DEGREES 04.6 MINUTES	066046
98- BORDER BRIDGE :		--
99- BORDER BRIDGE STRUCTURE NUMBER :		--
=====STRUCTURE TYPE AND MATERIAL=====		
43- STRUCTURE TYPE, MAIN :	CONCRETE CONTINUOUS TEE BEAMS	204
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	2	002
46- NUMBER OF APPROACH SPAN :	NONE	0000
107- DECK STRUCTURE TYPE :	CONCRETE CAST IN PLACE	1
108- WEARINGS SURF./ PROTECT. SYSTEM :	CONCRETE NONE NONE	100
=====AGE AND SERVICE=====		
19- BYPASS, DETOUR LENGTH (NEAREST MILE) :	STRUCTURE OVER HIGHWAY	00
27- YEAR BUILT :	1967	1967
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / 9 LANES UNDER	0009
29- A.D.T. OF INVENTORY ROUTE :	200,100	200100
30- YEAR OF A.D.T. :	1990	90
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
106- YEAR RECONSTRUCTION :	NO RECONSTRUCTION	0000
109- AVERAGE DAILY TRUCK TRAFFIC :	4 %	04
=====GEOMETRIC DATA=====		
10- INV. ROUTE, MIN. CLEAR. (0.01m) :	4.44 MT (14' - 7")	1407
32- APPROACH ROADWAY WIDTH :	44.30 MT (145.30 FT)	145
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
47- INV. ROUTE, TOTAL HORIZ. CLEARANCE :	20.84 MT (68.36 FT)	684
48- LENGTH OF MAXIMUM SPAN :	22.15 MT (72.65 FT)	0073
49- STRUCTURE LENGTH :	49.30 MT (161.70 FT)	000162
50- CURB OR SIDEWALK WIDTHS :	N/A	000000
51- BRIDGE ROADWAY WIDTH, CURB TO CURB :	2.10 MT (6.59 FT)	0066
52- DECK WIDTH, OUT TO OUT :	2.31 MT (7.58 FT)	0076
53- MIN. VERT. CLEAR. OVER BRIDGE ROADWAY :	UNLIMITED	9999
54- MINIMUM VERTICAL UNDERCLEARANCE :	4.44 MT (14' - 7")	H1407
55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT :	3.25 MT (10.8 FT)	H108
56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT :	2.55 MT (8.36 FT)	084

FEDERAL SYSTEM BRIDGE NO. : 00960 - ROAD NO. : PR 18 - KH. NO. : 004400

=====CLASSIFICATION=====

20- TOLL : ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY : D.T.P.W.	01
22- OWNER : D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE : INTERSTATE	11
37- HISTORICAL SIGNIFICANCE : 5	5
100- DEFENSE HIGHWAY DESIGNATION : DEFENSE HIGHWAY	1
101- PARALLEL STRUCTURE DESIGNATION : NO PARALLEL STRUCTURE	N
102- DIRECTION OF TRAFFIC : TRAFFIC NOT CARRIED	3
103- TEMPORARY STRUCTURE DESIGNATION : N/A	
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE : N.H.S.	1
110- DESIGNATED NATIONAL NETWORK : NOT PART OF NETWORK	0
112- NBIS BRIDGE LENGTH : YES	Y

=====WATERWAY=====

38- NAVIGATION CONTROL : N/A	N
39- NAVIGATION VERTICAL CLEARANCE : N/A	000
40- NAVIGATION HORIZONTAL CLEARANCE : N/A	0000
111- PIER OR ABUT. PROTECTION (NAVIGATION) : N/A	
116- MINIM. NAVIG. VERT. CLEARANCE : N/A	

=====PROPOSED IMPROVEMENT=====

75- TYPE OF WORK : REHABILITATION	352
76- LENGTH OF STRUCTURE IMPROVEMENT : 49.30 MT (161.70 FT)	000162
94- BRIDGE IMPROVEMENT COST : N/A	000000
95- ROADWAY IMPROVEMENT COST : N/A	000000
96- TOTAL PROJECT COST : (49.30X2.31X\$825X10%)=\$9,355	000009
97- YEAR OF IMPROVEMENT COST ESTIMATED : 1993	93
114- FUTURE AVERAGE DAILY TRAFFIC : 302,148	302148
115- YR. OF FUTURE A.D.T. : 2010	10

=====LOAD RATING AND POSTING=====

31- DESIGN LOAD : PEDESTRIAN	7
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. : OPEN	A
64- OPERATING RATING : PEDESTRIAN	800
66- INVENTORY RATING : PEDESTRIAN	800
70- BRIDGE POSTING : N/A.	N

=====CONDITION=====

	MATERIAL	CONDITION ANALYSIS	
58- DECK CONCRETE	SATISFACTORY	6
59- SUPERSTRUCTURE CONCRETE	SATISFACTORY	6
60- SUBSTRUCTURE CONCRETE	SATISFACTORY	6
61- CHANNEL & CHANNEL PROTECTION N/A	N/A	N
62- CULVERT N/A	N/A	N

PUERTO RICO HIGHWAY AUTHORITY
HIGHWAY SYSTEM ADMINISTRATION OFFICE

FEDERAL SYSTEM

BRIDGE NO. : 00960 - ROAD NO. : PR 18

- KM. NO. : 004400

=====APPRAISALS=====

67- STRUCTURAL EVALUATION :	6
WEARING SURFACE HAVE CONCRETE LOSE WITH EXPOSED STEEL. BOTTOM SLAB HAVE FINE TRANSVERSAL CRACKS WITH EFFLORESCENCES. CYCLONE FENCE PROTECTION IN ALL THE STRUCTURE. BEARING HAVE LIGH CORROSION. BEAMS HAVE IMPACTS OVER THE CAGUAS WAY. RAMP AT MEDICAL CENTER SIDE HAVE LARGE SPALLING WITH EXPOSED STEEL.		
68- DECK GEOMETRY :	N
N/A.		
69- UNDERCLEARANCE, VERTICAL & HORIZONTAL :	4
MEETS MINIMUM TOLERABLE LIMITS TO BE LEFT IN PLACE AS IS.		
71- WATERWAY ADEQUACY :	N
N/A.		
72- APPROACH ROADWAY ALIGNMENT :	N
N/A.		
36- TRAFFIC SAFETY FEATURES : N-N-N-N	NNNN
113- SCOUR CRITICAL BRIDGES : BRIDGE OVER HIGHWAY	N

=====ADDITIONAL COMMENTS=====

89- RESERVED	---
90- INSPECTION DATE : SEPTEMBER 3, 1993.	0993
91- DESIGNATED INSPECTION FREQUENCY : 24 MONTHS	24
92- CRITICAL FEATURE INSPECTION : N/A.	N N N
93- CRITICAL FEAT. INSP. DATE: N/A	---
105- RESERVED	---
117- SUFFICIENCY RATING :	---
126- FIELD INSPECTION DATE : 090393	090393
127- INSPECTION EVAL. DATE : 090393	090393
130- CRITICAL FRACTURE INSP. DATE :	---
131- EVALUATOR ENGINEER :	---



Javier Cruz Rosas

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE ITEM #67.

Appendix D Inspection by PRHTA of PB 1137

D.1 May 15, 2017

INSPECTION REPORT SUMMARY & QC SHEET						
BRIDGE: 1137						
TEAM LEADER: Angel Lopez						
INSP. DATE: 15 mayo 2017						
1. Inspection Type and Dates:						
NBI	Type	Performed? (Yes / No / NA)	Freq (MONTHS)	Previous Insp. DATE (MONTH/YEAR)	Next Insp. DATE (MONTH/YEAR)	
ITEM 90	Routine Inspection	Yes	—	Jan 2019	—	
ITEM 93 A	FC Inspection					
ITEM 93 B	Underwater Insp.					
ITEM 93 C	Other:					
2. NBI Condition Rating Summary:						
	Item 58	Item 59	Item 60	Item 61	Item 62	Item 113
Previous Inspection	5	5	6	—	—	—
Current Inspection	4	5	5	—	—	—
Other Checks: (Y, N, NA)			Review Comments:			
<input checked="" type="checkbox"/> Scour Critical (items 113 & 60) <input checked="" type="checkbox"/> AASHTO Core's & NBI CD consistent <input checked="" type="checkbox"/> Smart Flags (scour, steel plate, fire damage, etc) <input checked="" type="checkbox"/> Channel Profile/Clearance Table <input checked="" type="checkbox"/> FC & Underwater Members Tables <input checked="" type="checkbox"/> Asphalt Overlay Thickness <input checked="" type="checkbox"/> Drawings <input checked="" type="checkbox"/> Photos <input checked="" type="checkbox"/> Critical Finding <input checked="" type="checkbox"/> Inspector & Team Leader Signature						
Reviewer:						
Safety Eng.:						

Scan: MAYO - 31 - 2017 (Loil)

Structure Inventory and Appraisal Sheet

Bridge Key: 011371 Agency ID: 011371 Sufficiency Rating: -1.0

IDENTIFICATION

State 1: 72 Puerto Rico Struc Num 8: 011371
 Facility Carried 7: PR 26 Location 9: BALDORIOTY DE CASTRO AVE.
 Rte (On/Under) 5A: One Route Under Rte. Signing Prefix 5B: 2 U.S. Numbered
 Level of Service 5C: 1 Mainline Rte. Number 5D: 00003
 Directional Suffix 5E: 0 N/A (NBI) % Responsibility: NA
 SHD District 2: Unknown County Code 3: San Juan
 Place Code 4: 76770 Kilometer Post 11: 04.1 km
 Feature Intersected 6: PEDESTRIAN WALKWAY
 Latitude 16: 18d 27' 18" Longitude 17: 066d 03' 18"
 Border Bridge Code 98: Unknown (P)
 Border Bridge Number 99: Unknown

INSPECTION

Frequency 91: 24 months Inspection Date 90: 5/15/2017 Next Inspection: 05/15/2019
 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA
 UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 24 months Element Inspection Date: 01/01/1901 Next Elem. Insp. Due: 01/01/1901

CLASSIFICATION

Defense Highway 100: 0 Not a STRAIGHT hwy Parallel Structure 101: No || bridge exists
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Unknown (NBI)
 Highway System 104: 0 Not on NHS NBIS Length 112: Unknown (NBI)
 Toll Facility 20: 0 On free road Functional Class 26: 14 Urban Other Prime
 Historical Significance 37: Unknown (NBI)
 Owner 22: -1 Unknown (P)
 Custodian 21: -1 Unknown (P)

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: -1 Number of Spans Main Unit 45: -1
 Main Span Material/Design 43A/B:
 3 Steel 09
 Deck Type 107: Unknown (NBI)
 Wearing Surface 106A: Unknown (NBI)
 Membrane 100B: Unknown (NBI)
 Deck Protection 108C: Unknown (NBI)

CONDITION

Deck 58: 4 Poor Super 59: 5 Fair Sub 60: 5 Fair
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

LOAD RATING AND POSTING

Inventory Rating Method 65: Unknown (NBI) Operating Rating Method 63: Unknown (NBI)
 Inventory Rating 66: MS-6 Operating Rating 64: MS-0.6
 Design Load 31: 7 Pedestrian Posting 70: Unknown (NBI)
 Posting status 41: A Open, no restriction

AGE AND SERVICE

Year Built 27: 1968 Year Reconstructed 106: Unknown
 Type of Service on 42A: 3 Pedestrian bike/s
 Type of Service under 42B: 1 Highway
 Lanes on 26A: Unknown Lanes Under 206: 5 Detour Length 19: 0.0 km
 ADT 29: 67,300 Truck ADT 109: 4 % Year of ADT 30: 2002

APPRAISAL

Bridge Rail 36A: N N/A or not required Approach Rail 36C: N N/A or not required
 Transition 35B: N N/A or not required Approach Rail Ends 35D: N N/A or not required
 Str. Evaluation 67: Unknown (NBI) Deck Geometry 68: Unknown (NBI)
 Underclearance, Vertical and Horizontal 69: Unknown (NBI)
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: 6 Equal Min Criteria
 Scour Critical 113: N Not Over Waterway

GEOMETRIC DATA

Length Max Span 48: 23.60 m Structure Length 49: 61.00 m
 Curb/Sidewalk Width L 50A: Curb/Sidewalk Width R 50B:
 Width Curb to Curb 51: 1.60 m Width Out to Out 52:
 Approach Roadway Width 32: 24.60 m Median 33: - Unknown (NBI) (w/ shoulders)
 Deck Area:
 Skew 34: -1.00 ° Structure Flared 35: Unknown (NBI)
 Minimum Vertical Clearance Over Bridge 53:
 Minimum Vertical Underclearance Reference R 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 05.11 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 01.20 m
 Minimum Lateral Underclearance L 56: 02.20 m

PROPOSED IMPROVEMENTS

Bridge Cost 94: NA Type of Work 75: Unknown (P)
 Roadway Cost 95: Unknown Length of Improvement 76:
 Total Cost 96: Unknown Future ADT 114: 92,073
 Year of Cost Estimate 97: Unknown Year of Future ADT 115: 2018

NAVIGATION DATA

Navigation Control 38: - Unknown (NBI) Horizontal Clearance 40:
 Vertical Clearance 39:
 Pier Protection 111: Unknown (NBI) Lift Bridge Vertical Clearance 116:

ELEMENT CONDITION STATE DATA

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

58. DECK

58.1 Wearing Surface: Material: Steel Condition: 4 Thickness cm.
 Deterioration: 50 to 75 % Drainage: Inadequate Ponding: No Safety: No
 58.2 Slab or Plate: Material: Steel Condition: 4
 Cracking: None
 Spalling: Top: No Bottom: No
 Scaling: Top: No Bottom: No
 Efflorescence: No Exudation: No Rust Stains: No Corrosion: Severe
 58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	5		Good	Good	Good	Severe	Poor	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A
 58.10 Utilities:
 Type: Size: Safety: _
 58.11 Joints: Condition: N/A
 Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No
 58.12 Drains and Scuppers:
 Material: None Condition: N Functioning: N/A

Comments:

58.1- Hay moderada y severa corrosión en la superficie, hay varias perforaciones grandes por corrosión en las tolas, hay una sección grande con la esquina suelta e inclinada formando un hueco, grandes parchos para reparar secciones corroídas, hay varias secciones de las tolas que están sueltas de sus puntos de soldaduras, varias de los parchos que son en aluminio tiene los tornillos de anclajes sueltos, esto puede causar accidente. Muchas de las secciones fueron pintadas con óxido rojo para protección del acero, la superficie de la estructura está en pobres condiciones.

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed/Mov. Out of Plumb: No Paint: None
Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.
Condition: N Cracking: No Spalling: None Scaling: None
Crushing: No

59.3 Steel beams: N/A Type: N/A
Condition: N/A Corrosion: N/A Paint: N/A

59.4 Concrete: N/A Type: N/A
Condition: N Cracking: No Spalling: Small Scaling: None

59.5 Truss: Ped. Bailey Condition: 5 Paint: Fair Corrosion: Moderate Members: N/A

59.6 Drainage: Type: N/A Condition: N/A Functioning: N/A

59.7 Hinges: Condition: N/A Functioning: N/A Movement: N/A

59.8 Deflection: N/A

59.9 Vibrations: N/A

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.2.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.2.3	Columns	Steel	5	--	--	--	Severe	Poor	No	No	No
	60.2.4	Footing	Concrete	7	F	Small	--	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.5- Varios de los componentes laterales de los "bracing" tienen ligera, moderada y severa corrosión y pérdida de sección, algunas secciones en el "upper chord" tienen moderada pérdida de sección por corrosión,

60.2.3- Columnas en acero en buenas condiciones, ligera corrosión.

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

61. CHANNEL AND PROTECTION Condition: N

61.1 Channel Scour: N/A

61.2 Embankment Erosion: N/A

		Type	Material	Condition Rating	Functioning
61.3	Protective Device		N/A	N	--
61.4	Fender System		N/A	N	--
61.5	Rip Rap		N/A	N	--
61.6	Spur Dikes, Jetties		N/A	N	--

61.7 Obstruction: N/A

61.8 Channel Change: N/A Detrimental: N/A

61.9 Adequate Waterway: N/A

61.10 Surrounding Area: Flooding: N/A if yes explain:

 Erosion: N/A if yes explain:

61.11 Location of piers and/or abutment detrimental: N/A if yes explain:

61.12 Other features that may affect structure:

Comments:

62. CULVERT AND RETAINING WALL

	Material	Condition Rating	Functioning	Cracking	Spalling	Scaling	Corrosion	Settlement	Alignment	Scour
62.1 Barrel	N/A	N	--	--	--	--	--	--	--	--
62.2 Head Walls	N/A	N	--	--	--	--	--	--	--	--
62.3 Cut-off wall	N/A	N	--	--	--	--	--	--	--	--
62.4 Retaining wall	N/A	N	--	--	--	--	--	--	--	--

Comments:

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: None Scaling: None Uneven: No

Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No

Excessive deformation: No

Safety: Hazardous: No

Drainage: Inadequate: No

Movement: Pavement-approach slab: No

Embankment: Condition: N Functioning: Yes Erosion: No

Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: May, 15, 2017

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:

Underwater:

Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Steel Plate (includes orthotropic)

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Other

Type Membrane: None

Deck protection: None

111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

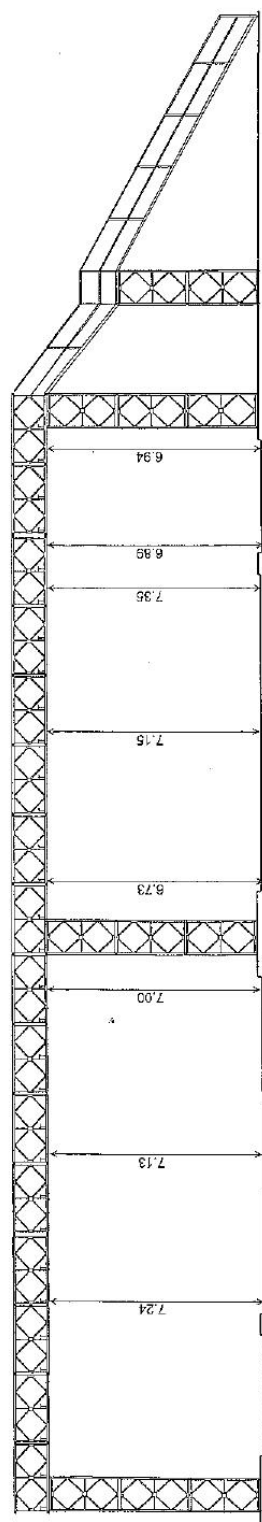
116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

AUXILIARY ITEMS

Signs: Type: Route Orientation Material: Alum. Condition: 5

BR-1137

5

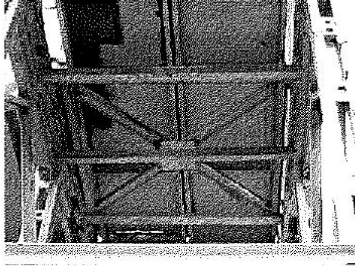


WEST ELEVATION

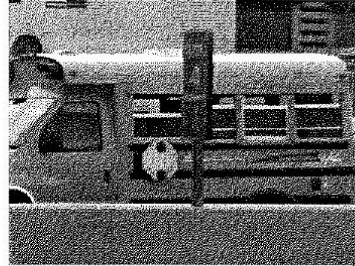
HIGHWAY AND TRANSPORTATION AUTHORITY			
BRIDGE ENGINEERING OFFICE			
PEDESTRIAN BRIDGE OVER PR-26			
ROAD PR-26, KM. 4.07, SAN JUAN			
DRAWN BY	J.R. VASQUEZ	JAN 2014	BRIDGE NO.
CHECKED BY			1137
APPROVED BY			DRAWING NO.
			1 / 1



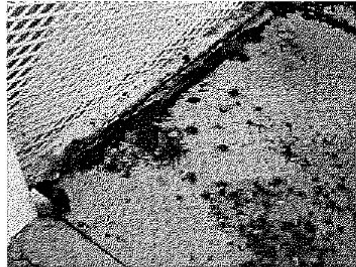
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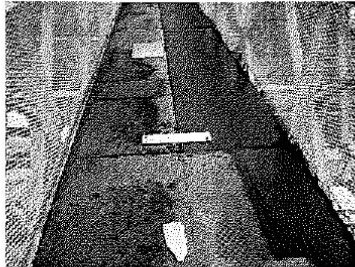
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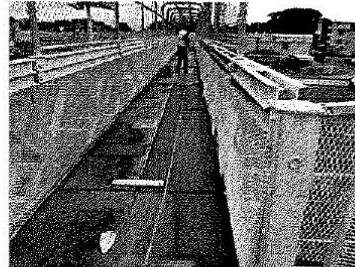
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1137-may-15-2017-030.jpg



1137-may-15-2017-032.jpg

Bridge Inspection Report

Bridge Key: 011371 Agency ID: 011371 Sufficiency Rating: -1.0

IDENTIFICATION
 State 1: 72 Puerto Rico Struc Num 8: 011371
 Facility Carried 7: PR 26 Location 9: BALDORIOY DE CASTRO AVE.
 Rte.(On/Under)5A: One Route Under Rte. Signing Prefix 5B: 2 U.S. Numbered
 Level of Service 5C: 1 Mainline Rte. Number 5D: 00026
 Directional Suffix 5E: 0 N/A (NBI) % Responsibility : NA
 SHD District 2: SAN JUAN County Code 3: SAN JUAN
 Place Code 4: SAN JUAN ZONA URBANA Kilometer Post 11: 04.1 km
 Feature Intersected 6: PEDESTRIAN WALKWAY
 Latitude 16: 18d 27' 18" Longitude 17: 066d 03' 18"
 Border Bridge Code 98: Unknown (P)
 Border Bridge Number 99: Unknown

INSPECTION
 Frequency 91: 24 months Inspection Date 90: 1/23/2014 Next Inspection: 01/23/2016
 FC Frequency 92A: NA FC Inspection Date 93A: 1/23/2014 Next FC Inspection: NA
 UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 24 months Element Inspection Date: 01/01/1901 Next Elem. Insp. Due: 01/23/2014

CLASSIFICATION
 Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: No || bridge exists
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Not Applicable (P)
 Highway System 104: 0 Not on NHS NBIS Length 112: Long Enough
 Toll Facility 20: 3 On free road Functional Class 26: 14 Urban Other Princ
 Historical Significance 37: 5 Not eligible for NRHP
 Owner 22: 01 State Highway Agency
 Custodian 21: 01 State Highway Agency

STRUCTURE TYPE AND MATERIALS
 Number of Approach Spans 46: 0 Number of Spans Main Unit 45: 2
 Main Span Material/Design 43A/B:
 3 Steel 09
 Deck Type 107: 5 Steel Plate
 Wearing Surface 108A: 9 Other
 Membrane 108B: 0 None
 Deck Protection 108C: None

CONDITION
 Deck 58: 5 Fair Super 59: 5 Fair Sub 60: 6 Satisfactory
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

LOAD RATING AND POSTING
 Inventory Rating Method 65: Unknown (NBI) Operating Rating Method 63: Unknown (NBI)
 Inventory Rating 66: MS-6 Operating Rating 64: MS-0.6
 Design Load 31: 7 Pedestrian Posting 70: Unknown (NBI)
 Posting status 41: Unknown (NBI)

AGE AND SERVICE
 Year Built 27: 1968 Year Reconstructed 106: Unknown
 Type of Service on 42A: 3 Pedestrian-bicycle
 Type of Service under 42B: 1 Highway
 Lanes on 28A: Unknown Lanes Under 28B: 5 Detour Length 19: 0.0 km
 ADT 29: 67,300 Truck ADT 109: 4 % Year of ADT 30: 2005

APPRAISAL
 Bridge Rail 36A: Unknown (NBI) Approach Rail 36C: Unknown (NBI)
 Transition 36B: Unknown (NBI) Approach Rail Ends 36D: Unknown (NBI)
 Str. Evaluation 67: Unknown (NBI) Deck Geometry 68: Unknown (NBI)
 Underclearance, Vertical and Horizontal 69: Unknown (NBI)
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: Unknown (NBI)
 Scour Critical 113: Unknown (NBI)

GEOMETRIC DATA
 Length Max Span 48: 24.37 m Structure Length 49: 53.14 m
 Curb/Sdwk Width L 50A: 0.00 m Curb/Sidewalk Width R 50B: 0.00 m
 Width Curb to Curb 51: 1.84 m Width Out to Out 52: 2.84 m
 Approach Roadway Width 32: 24.60 m Median 33: 0 No median (w/ shoulders)
 Deck Area: 150.92 m²
 Skew 34: 0.00 ° Structure Flared 35: 0 No flare
 Minimum Vertical Clearance Over Bridge 53: 99.99 m
 Minimum Vertical Underclearance Reference 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 06.73 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 01.20 m
 Minimum Lateral Underclearance L 56: 02.20 m

PROPOSED IMPROVEMENTS
 Bridge Cost 94: NA Type of Work 75: Unknown (P)
 Roadway Cost 95: Unknown Length of Improvement 76:
 Total Cost 96: Unknown Future ADT 114: 92,073
 Year of Cost Estimate 97: Unknown Year of Future ADT 115: 2020

NAVIGATION DATA
 Navigation Control 38: N NA-no waterway
 Vertical Clearance 39: 0.00 m Horizontal Clearance 40: 0.00 m
 Pier Protection 111: Unknown (NBI) Lift Bridge Vertical Clearance 116: 0.00 m

ELEMENT CONDITION STATE DATA

BRIDGE NOTES

Bridge Inspection Report

PAST INSPECTION

Inspection Date: 01/23/2014

Type: 1 Regular NBI

Inspector: -1

Pontis User Key: Pontis - Pontis Poi

Scope:

NBI: Other: Element:
Underwater: Fracture Critical:

INSPECTION NOTES

LIGHT TO MODERATE CORROSION AND SOME HOLES BY CORROSION AT STEEL PLATES. LATERAL COMPONENTS OF BRACINGS SHOWS LIGHT TO MODERATE CORROSION AND SECTION LOSS. MODERATE SECTION LOSS BY CORROSION AT UPPER CHORDS.

INSPECTOR WORK CANDIDATES

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

58. DECK

58.1 Wearing Surface: Material: Steel **Condition:** 5 Thickness cm.
Deterioration: 10 to 25 % Drainage: Inadequate Ponding: Yes Safety: Yes

58.2 Slab or Plate: Material: Steel **Condition:** 5
Cracking: None
Spalling: Top: No Bottom: No
Scaling: Top: No Bottom: No
Efflorescence: No Exudation: No Rust Stains: No Corrosion: Moderate

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	6		Good	Good	Good	Moderate	--	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: _

58.11 Joints: Condition: N/A

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: None Condition: N Functioning: N/A

Comments:

58.1- Ligera y moderada corrosión en superficie, algunas pequeñas perforaciones por corrosión en las tolas.

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed/Mov. Out of Plumb: No Paint: None
Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.

Condition: N Cracking: No Spalling: None Scaling: None
Crushing: No

59.3 Steel beams: N/A Type: N/A

Condition: N/A Corrosion: N/A Paint: N/A

59.4 Concrete: N/A Type: N/A

Condition: N Cracking: No Spalling: Small Scaling: None

59.5 Truss: Ped. Bailey Condition: 5 Paint: Fair Corrosion: Moderate Members: N/A

59.6 Drainage: Type: N/A Condition: N/A Functioning: N/A

59.7 Hinges: Condition: N/A Functioning: N/A Movement: N/A

59.8 Deflection: N/A

59.9 Vibrations: N/A

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.2.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.2.3	Columns	Steel	6	--	--	--	Moderate	--	No	No	No
	60.2.4	Footing	Concrete	7	F	Small	--	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.5- Varios de los componentes laterales de los "bracing" tienen ligera, moderada corrosión y pérdida de sección, algunas secciones en el "upper chord" tienen moderada pérdida de sección por corrosión.

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

61. CHANNEL AND PROTECTION Condition: N

61.1 Channel Scour: N/A

61.2 Embankment Erosion: N/A

		Type	Material	Condition Rating	Functioning
61.3	Protective Device		N/A	N	--
61.4	Fender System		N/A	N	--
61.5	Rip Rap		N/A	N	--
61.6	Spur Dikes, Jetties		N/A	N	--

61.7 Obstruction: N/A

61.8 Channel Change: N/A Detrimental: N/A

61.9 Adequate Waterway: N/A

61.10 Surrounding Area: Flooding: N/A if yes explain:

Erosion: N/A if yes explain:

61.11 Location of piers and/or abutment detrimental: N/A if yes explain:

61.12 Other features that may affect structure:

Comments:

62. CULVERT AND RETAINING WALL

	Material	Condition Rating	Functioning	Cracking	Spalling	Scaling	Corrosion	Settlement	Alignment	Scour
62.1 Barrel	N/A	N	--	--	--	--	--	--	--	--
62.2 Head Walls	N/A	N	--	--	--	--	--	--	--	--
62.3 Cut-off wall	N/A	N	--	--	--	--	--	--	--	--
62.4 Retaining wall	N/A	N	--	--	--	--	--	--	--	--

Comments:

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: None Scaling: None Uneven: No

Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No

Excessive deformation: No

Safety: Hazardous: No

Drainage: Inadequate: No

Movement: Pavement-approach slab: No

Embankment: Condition: N Functioning: Yes Erosion: No

Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: Jan 23, 2014

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:

Underwater:

Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Steel Plate (includes orthotropic)

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Other

Type Membrane: None

Deck protection: None

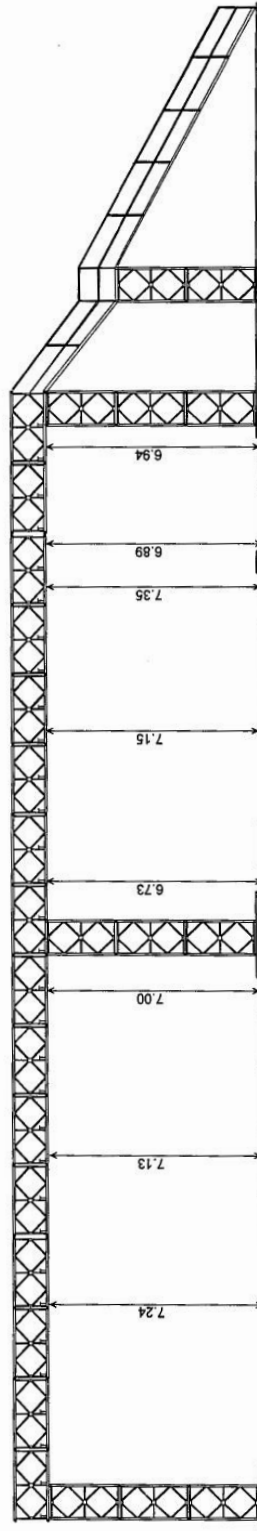
111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

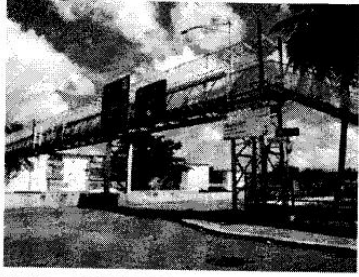
AUXILIARY ITEMS

Signs: Type: Route Orientation Material: Alum. Condition: 5



WEST ELEVATION

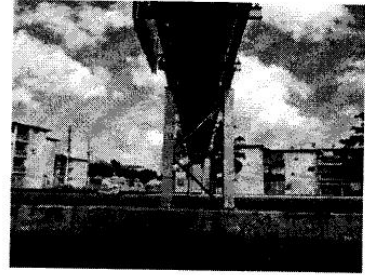
HIGHWAY AND TRANSPORTATION AUTHORITY		BRIDGE ENGINEERING OFFICE	
PEDESTRIAN BRIDGE OVER PR-26		ROAD PR-26, KM. 4.07, SAN JUAN	
DRAWN BY	J.F. VAZQUEZ	BRIDGE NO.	1137
CHECKED BY		DRAWING NO.	1 / 1
APPROVED BY			



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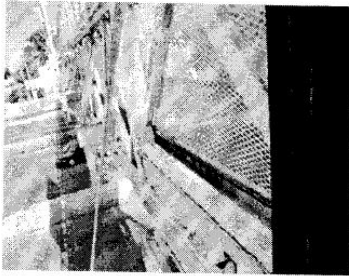
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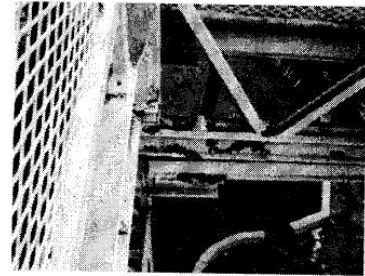
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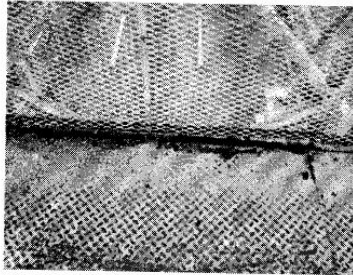
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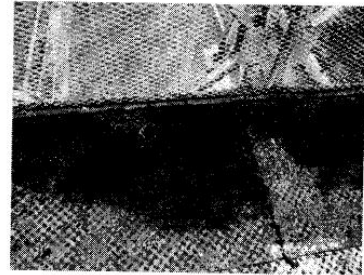
1137-Jan-23-2014-014.jpg



1137-Jan-23-2014-015.jpg



1137-Jan-23-2014-017.jpg



1137-Jan-23-2014-019.jpg

D.3 June 29, 2001

ENGINEER: EDUARDO MARQUEZ	PUERTO RICO HIGHWAY AUTHORITY	PAGE 1 OF 7
ASSISTANTS: M. SANTIAGO	BRIDGE STUDIES AND EVALUATION OFFICE	EQUIPMENT
: JAIME RIVAS	BRIDGE REINSPECTION & EVALUATION REPORT	BUS X LADDERS
: JUAN OTERO	BRIDGE No.: 01137 FEDERAL SYSTEM	BOAT CAMERA X
: JORGE VIERA	ROAD No.: PR 26 KM. No.: 0004.070	UNDERWATER SNOOPER #
EVALUATION DATE: 29/JUNE/01		SNOOPER OPERATOR
		PLAQUE ID
		YES X NO No.
		FILM No.

=====IDENTIFICATION=====

1- STATE :	Commonwealth of Puerto Rico	721
2- HIGHWAY AGENCY DISTRICT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN	76770
5- INVENTORY ROUTE :	2-2-1-00003-0	221000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	PR 26	PR 26
8- STRUCTURE NUMBER :	1137 (1 OF 1)	011371
9- LOCATION :	BALDORIOY DE CASTRO AVE.	BALDORIOY DE CASTRO AVE.
10- INVENTORY ROUTE, MINIMUM VERTICAL CLEARANCE :	5.11m (16'-9")	0510
11- KILOMETERPOINT :	4.07	0004070
12- BASE HIGHWAY NETWORK :		0
13- LRS INVENTORY ROUTE, SUBROUTE NUMBER :		
16- LATITUDE :	18 DEG 27.3 MIN	18271800
17- LONGITUDE :	66 DEG 03.3 MIN	066031800
19- BYPASS, DETOUR LENGTH (NEAREST KILOMETER) :	STRUCTURE OVER HWY.	000

=====CLASSIFICATION=====

20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE	14

=====AGE AND SERVICE=====

27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	00 ON ; 05 UNDER	0005
29- A.D.T. OF INVENTORY ROUTE :	67,300	067300
30- YEAR OF A.D.T. :	1999	1999

=====STRUCTURE DATA=====

31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	24.59m (80.65ft)	0246
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	NO	0000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO	00000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HWY.	31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000

BRIDGE No.: 01137

18.4 m

47- INV. ROUTE, TOTAL HORIZ. CLEARANCE : L=23.59m(77.39ft) R=23.59m(77.4ft) 184
 48- LENGTH OF MAXIMUM SPAN : 23.6m(77.4ft) 00236
 49- STRUCTURE LENGTH : 52.14m(171.01ft) 000521
 50- CURB OR SIDEWALK WIDTHS : NONE 000000

=====STRUCTURE DATA=====

51- BRIDGE ROADWAY WIDTH, CURB TO CURB : 1.64m(5.37ft) 0016
 52- DECK WIDTH, OUT TO OUT : 2.00m(6.56ft) 0020
 53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY : UNLIMITED 9999
 54- MINIMUM VERTICAL UNDERCLEARANCE : 5.11m(16'-9") H0511
 55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT : 1.19m(3.9ft) H012
 56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT : 2.22m(7.28ft) 022

=====CONDITION=====

58- DECK: FAIR CONDITION RATING: 5 DECK COMMENTS:
 58.1 WEARING SURFACE: MATERIAL: STEEL CONDITION : 5 58.1 LITE AND MODERATE CORROSION IN ALL
 THICKNESS: THE STRUCTURE.
 DETERIORATION: YES X NO DRAINAGE: X ADEQUATE INADEQUATE
 PONDING: YES X NO SAFETY: X YES NO

58.2 SLAB OR PLATE: MATERIAL: STEEL CONDITION 5
 CRACKING: FINE MEDIUM OPEN

	TOP	BOTTOM
SPALLING:	S < 1 in. DEEP x 6 in. & L > 1 in. x 6 in. &	
SCALING :	L to 1/2 in., M 1/4 in. - 1/2 in., H 1/2 in. - 1 in., S > 1	

58.3 MOVEMENT: DECK TO BACKWALL cm. DECK TO APPROACH SLAB cm.
 EFFLORESCENCE EXUDATION RUST STAINS XCORROSION: XLIGHT MODERATE SEVERE

	(58.4)	(58.5)	(58.6)	(58.7)	(58.8)
	CURBS	MEDIAN	SIDEWALKS	PARAPETS	RAILING
MATERIAL					STEEL
CONDITION RATING					6
HEIGHT LOSS (cm)					
JOINTS					OK
DRAINAGE					OK
ALIGNMENT					OK
CORROSION (L-M-S)					L
PAINT: G-GOOD, F-FAIR, P-POOR, N-NONE, U-USBLESS TO PAINT					F
SAFETY					YES
CRACKING (F-M-O)					
SPALLING (S-L)					
SCALING (L-M-H-S)					

58.9 LIGHTING STANDARDS: MATERIAL: NO CONDITION: FUNCTIONING: YES NO
 58.10 UTILITIES: TYPE: NO SIZE: SAFETY:
 58.11 JOINTS: EXPANSION CONSTRUCTION CONDITION: FUNCTIONING: YES NO
 LEAKING: YES NO CRACKING: FINE MEDIUM OPEN SPALLING: SMALL LARGE
 58.12 DRAINS AND SCUPPERS: MATERIAL: CONDITION: FUNCTIONING: YES NO

BRIDGE No.: 01137

59- SUPERSTRUCTURE: FAIR CONDITION RATING: 5 SUPERSTRUCTURE COMMENTS:
 59.1 BEARING DEVICES: TYPE: FIXED MOVABLE OUT OF PLUMB 59.5 LITE ABRASION BY CORROSION.
 PAINT: GOOD FAIR POOR NONE USELESS TO PAINT
 CONDITION: FUNCTIONING: YES NO
 59.2 BRIDGE SEATS, PEDESTAL, GROUT PADS, ABUTMENTS OR PIER SEATS WHERE BEAMS BEAR
 DIRECTLY ON CONCRETE
 CONDITION: CRACKING: F M O SPALLING: S L
 SCALING: L M H S CRUSHING:
 59.3 STEEL-BEAMS: TYPE: DIMENSIONS (height; width; thickness):
 CONDITION: CORROSION: L M S PAINT: G F P N U
 59.4 CONCRETE-BOX BEAMS, I-BEAMS, SLAB, T-BEAM, TYPE:
 CONDITION: CRACKING: F M O SPALLING: S L
 SCALING: L M H S
 59.5 TRUSSES: TYPE: BAILEY CONDITION: 5 CORROSION: X L X M S
 DAMAGE: DEFORMATION: PAINT: G F P N U
 A=TRUSS B=UPPER & LOWER LATERAL BRACING C=PORTALS D=FLOOR BEAMS E*=STRINGERS
 59.6 DRAINAGE: TYPE: CONDITION: FUNCTIONING: YES NO
 59.7 HINGES: CONDITION: FUNCTIONING: YES NO MOVEMENT: YES NO
 ALIGNMENT: GOOD BAD PAINT: G F P N U
 59.8 DEFLECTIONS: X NORMAL EXCESSIVE cms.
 59.9 VIBRATIONS: X MINIMAL MODERATE EXCESSIVE

60- SUBSTRUCTURE: SATISFACTORY COND. RATING: 6

	← ABUTMENTS →				← PIERS OR NON PILES BENTS →					← PILES BENTS →		
	60.1.1	60.1.2	60.1.3	60.1.4	60.2.1	60.2.2	60.2.3	60.2.4	60.2.5	60.4.1	60.4.2	60.4.3
	WINGWALL	BREAST & BACKWALL	FOOTING	PILES	CAPS	BRACING	COLUMNS	FOOTINGS	PILES	CAPS	BRACING	PILES
MATERIAL							STEEL	CONCRETE				
CONDITION RATING							6	6				
CRACKING (F-M-O)								F				
SPALLING (S-L)								S				
SCALING (L-M-H-S)								L				
CORROSION (L-M-S)												
PAINT (G-F-P-N-U)							F	F				
MOVEMENT							NO	NO				
EROSION							NO	NO				
UNDERMINING							NO	NO				

60- SUBSTRUCTURE COMMENTS:

BRIDGE No.: 01137

61- CHANNEL & CHANNEL PROTECTION : N/A

RATING : N

CHANNEL COMMENTS:

61.1 CHANNEL SCOUR(EXTENT) :

61.2 EMBANKMENT EROSION(EXTENT) :

	TYPE	MATERIAL	CONDITION	FUNCTIONING	
			RATING	YES	NO
61.3	PROTECTIVE DEVICE				
61.4	FENDER SYSTEM				
61.5	RIP RAP				
61.6	SPUR DIKES, JETTIES				

61.7 OBSTRUCTION (DEBRIS, GROWTHS):

61.8 CHANNEL CHANGE: YES NO

DETRIMENTAL: YES NO IF YES, EXPLAIN

61.9 ADEQUATE WATERWAY: YES NO IF NO, EXPLAIN

61.10 SURROUNDING AREA: FLOODING: YES NO IF YES, EXPLAIN

EROSION: YES NO IF YES, EXPLAIN

61.11 LOCATION OF PIERS AND/OR ABUTMENTS: DETRIMENTAL: YES NO
 IF YES, EXPLAIN

61.12 OTHER FEATURES THAT MAY AFFECT STRUCTURE:

62- CULVERT AND RETAINING WALLS :N/A

RATING :N

CULVERT COMMENTS:

	62.1 BARREL	62.2 HEADWALL	62.3 CUT-OFFWALL	62.4 RETAINING WALL
MATERIAL				
CONDITION RATING				
FUNCTIONING (Y-N)				
CRACKING (F-M-O)				
SPELLING (S-L)				
SCALING (L-M-H-S)				
CORROSION (L-M-S)				
SETTLEMENT (Y-N)				
ALIGNMENT (Y-N)				
PAINT (G-F-P-N-U)				

63- METHOD USED TO DETERMINE OPERATING RATING .. ALLOWABLE STRESS

2

64- OPERATING RATING : PEDESTRIAN

800

65- METHOD USED TO DETERMINE INVENTORY RATING .. ALLOWABLE STRESS

2

66- INVENTORY RATING : PEDESTRIAN

800

BRIDGE NO. :01137

=====PROPOSED IMPROVEMENT=====

75- TYPE OF WORK :	REHABILITATION NBRDRD	352
76- LENGTH OF STRUCTURE IMPROVEMENT :	52.14 MTS. (171.01 FT.)	000521

=====INSPECTIONS=====

89- RESERVED		--
90- INSPECTION DATE :	JUNE 2001	0601
91- DESIGNATED INSPECTION FREQUENCY :	EVERY-24-MONTHS	24
92- CRITICAL FEATURE INSPECTION :	NOT NEEDED	N N N
93- CRITICAL FEAT. INSPECT. DATE:	N/A	
FRACTURE CRITICAL	UNDERWATER	OTHER

=====IMPROVEMENT COST=====

94- BRIDGE IMPROVEMENT COST :	N/A	000000
95- ROADWAY IMPROVEMENT COST :	N/A	000000
96- TOTAL PROJECT COST :	(2.00) (52.14) (825) (0.15)=\$12,905	000013
97- YEAR OF IMPROVEMENT COST ESTIMATED :	2001	2001

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE :		--
99- BORDER BRIDGE STRUCTURE NUMBER :		--
100- STRAHNET HIGHWAY DESIGNATION :	IS A DH	0
101- PARALLEL STRUCTURE DESIGNATION :	N/A	N
102- DIRECTION OF TRAFFIC :	NOT CARRIED TRAFFIC	0
103- TEMPORARY STRUCTURE DESIGNATION :	N/A	
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE :	NHS	0
105- FEDERAL LANDS HIGHWAYS :		0
106- YEAR RECONSTRUCTION :	N/A	0000
107- DECK STRUCTURE TYPE :	STEEL PLATE	5
108- WEARING SURF./ PROTECT. SYSTEM :	NONE	000
109- AVERAGE DAILY TRUCK TRAFFIC :	48 (EST)	04
110- DESIGNATED NATIONAL NETWORK :	PART OF NNT	1
111- PIER OR ABUT. PROTECTION (NAVIGATION) :	N/A	
112- NBIS BRIDGE LENGTH :	YES	Y
113- SCOUR CRITICAL BRIDGE :	N/A	N
114- FUTURE AVERAGE DAILY TRAFFIC :	92,073	092073
115- YR. OF FUTURE A.D.T. :	2019	2019
116- MINIM. NAVIG. VERT. CLEARANCE :	N/A	
VERTICAL LIFT BRIDGE :		
117- SUFFICIENCY RATING :		
125- PRIORITY RATING:		
127- EVALUATION DATE :	29/JUNE/01	062901
130- CRITICAL FRACTURE INSP. DATE :		

ENGINEER : EDUARDO MARQUEZ

BRIDGE NO. :01137

AUXILIARY ITEMS

1-SIGNS

TYPE:

MATERIAL:

CONDITION:

REMARKS:

REHABILITATION CONSISTS ON REPAIRING THE DEFICIENCIES DISCUSSED ON
ITEM 67.

D.4 March 11, 1999

ENGINEER: HERIBERTO GONZALEZ ASSISTANTS: LUIS QUINONEZ : JAIME RIVAS : ALFREDO ERAZO		PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REINSPECTION & EVALUATION REPORT BRIDGE No.: 01137 FEDERAL SYSTEM		PAGE 1 OF 7 EQUIPMENT BUS X LADDERS BOAT CAMERA X UNDERWATER SNOOPER # SNOOPER OPERATOR PLAQUE ID YES X NO No. FILM No.	
ROAD No.: PR 26 KM. No. : 0004.070 EVALUATION DATE: 11-MAR-1999					
-----IDENTIFICATION-----					
1- STATE :	Commonwealth of Puerto Rico				721
2- HIGHWAY AGENCY DISTRICT :	SAN JUAN				01
3- COUNTY (PARISH) CODE :	SAN JUAN				127
4- PLACE CODE :	SAN JUAN URBAN				7 76770
5- INVENTORY ROUTE :	2-7-1-00003-0				2-1000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY				PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	PR 26				PR 26
8- STRUCTURE NUMBER :	1137 (1 OF 1)				011371
9- LOCATION :	BALDORIOY DE CASTRO AVE.				BALDORIOY DE CASTRO AVE.
10- INVENTORY ROUTE, MINIMUM VERTICAL CLEARANCE :	5.11m(16'-9")				0510
11- KILOMETERPOINT :	4.07				0004070
12- BASE HIGHWAY NETWORK :					
13- LRS INVENTORY ROUTE, SUBROUTE NUMBER :					blank 00000002500
16- LATITUDE :	18 DEG 27.3 MIN				18271800
17- LONGITUDE :	66 DEG 03.3 MIN				066031800
19- BYPASS, DETOUR LENGTH (NEAREST KILOMETER) :	STRUCTURE OVER HWY.				000
-----CLASSIFICATION-----					
20- TOLL :	ON FREE ROAD				3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.				01
22- OWNER :	D.T.P.W.				01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE				11
-----AGE AND SERVICE-----					
27- YEAR BUILT :	1968				1968
28- LANES ON AND UNDER STRUCTURE :	00 ON ; 05 UNDER				0005
29- A.D.T. OF INVENTORY ROUTE :	67,300				067300
30- YEAR OF A.D.T. :	1996				1996
-----STRUCTURE DATA-----					
31- DESIGN LOAD :	PEDESTRIAN				7
32- APPROACH ROADWAY WIDTH :	24.59m(80.65ft)				024.65ft
33- BRIDGE MEDIAN :	NONE				0
34- SKEW ANGLE :	NO				00
35- STRUCTURE FLARED :	NO				0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N				NNNN
37- HISTORICAL SIGNIFICANCE :	5				5
38- NAVIGATION CONTROL :	N/A				N
39- NAVIGATION VERTICAL CLEARANCE :	NO				0000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO				00000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN				A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HWY.				31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK				309
44- STRUCTURE TYPE APPR. :	NONE				000
45- NUMBER OF SPAN IN MAIN UNIT :	002				002
46- NUMBER OF APPROACH SPAN :	NONE				0000

BRIDGE STUDIES AND EVALUATION OFFICE

BRIDGE REINSPECTION & EVALUATION REPORT

BRIDGE No.: 01137

47- INV. ROUTE, TOTAL HORIZ. CLEARANCE : ^{18.4} ~~18.4~~ ^{mt} L=23.59m(77.39ft) R=23.6m(77.4ft)
 48- LENGTH OF MAXIMUM SPAN : 23.6m(77.4ft)
 49- STRUCTURE LENGTH : 52.14m(171.01ft)
 50- CURB OR SIDEWALK WIDTHS : NONE

JA *084*
0023
000521
000000

-----STRUCTURE DATA-----

51- BRIDGE ROADWAY WIDTH, CURB TO CURB : 1.64m(5.37ft)
 52- DECK WIDTH, OUT TO OUT : 2.00m(6.56ft)
 53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY : UNLIMITED
 54- MINIMUM VERTICAL UNDERCLEARANCE : 5.11m(16'-9")
 55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT : 1.19m(3.9ft)
 56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT : 2.22m(7.28ft)

0016
 0020
 9999
 H051
 H012
 022

-----CONDITION-----

58- DECK: FAIR CONDITION RATING: 5

DECK COMMENTS:

58.1 WEARING SURFACE: MATERIAL: STEEL CONDITION : 5
 THICKNESS:
 DETERIORATION: YES X NO DRAINAGE: X ADEQUATE INADEQUATE
 PONDING: YES X NO SAFETY: X YES NO

STRUCTURE WAS PAINTED. STEEL SHOWS LIGHT TO MODERATE CORROSION.

58.2 SLAB OR PLATE: MATERIAL: STEEL CONDITION 5

CRACKING: FINE MEDIUM OPEN

TOP BOTTOM

SPALLING: [S < 1 in. DEEP x 6 in. & L > 1 in. x 6 in. &]
 SCALING : [L to 1/4 in., M 1/4 in. - 1/2 in., H 1/2 in. - 1 in., S > 1]

EFFLORESCENCE EXUDATION RUST STAINS XCORROSION: XLIGHT MODERATE SEVERE
 58.3 MOVEMENT: DECK TO BACKWALL cm. DECK TO APPROACH SLAB cm.

	(58.4)	(58.5)	(58.6)	(58.7)	(58.8)
	CURBS	MEDIAN	SIDEWALKS	PARAPETS	RAILING
MATERIAL					STEEL
CONDITION RATING					6
HEIGHT LOSS (cm)					
JOINTS					OK
DRAINAGE					
ALIGNMENT					OK
CORROSION (L-M-S)					L
PAINT: G-GOOD, F-FAIR, P-POOR, N-NONE U-USELESS TO PAINT					F
SAFETY					YES
CRACKING (F-M-O)					
SPALLING (S-L)					
SCALING (L-M-H-S)					

58.9 LIGHTING STANDARDS: MATERIAL: NO CONDITION: FUNCTIONING: YES NO

58.10 UTILITIES: TYPE: NO SIZE: SAFETY:

58.11 JOINTS: EXPANSION CONSTRUCTION CONDITION: FUNCTIONING: YES NO
 LEAKING: YES NO CRACKING: FINE MEDIUM OPEN SPALLING: SMALL LARGE

58.12 DRAINS AND SCUPPERS: MATERIAL: CONDITION: FUNCTIONING: YES NO

BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

BRIDGE No.: 01137

59- SUPERSTRUCTURE: FAIR CONDITION RATING: 5 SUPERSTRUCTURE COMMENTS:
 59.1 BEARING DEVICES: TYPE: FIXED MOVABLE OUT OF PLUMB
 PAINT: GOOD FAIR POOR NONE USELESS TO PAINT
 CONDITION: FUNCTIONING: YES NO
 59.2 BRIDGE SEATS, PEDESTAL, GROUT PADS, ABUTMENTS OR PIER SEATS WHERE BEAMS BEAR
 DIRECTLY ON CONCRETE
 CONDITION: CRACKING: F M O SPALLING: S L
 SCALING: L M H S CRUSHING:
 59.3 STEEL BEAMS: TYPE: DIMENSIONS (height; width; thickness):
 CORROSION: L M S PAINT: G F P N U
 59.4 CONCRETE-BOX BEAMS, I-BEAMS, SLAB, T-BEAM, TYPE:
 CONDITION: CRACKING: F M O SPALLING: S L
 SCALING: L M H S
 59.5 TRUSSES: TYPE: BAILEY CONDITION: 5 CORROSION: X L X M S
 DAMAGE: DEFORMATION: PAINT: G F P N U
 A=TRUSS B=UPPER & LOWER LATERAL BRACING C=PORTALS D=FLOOR BEAMS E*=STRINGERS
 59.6 DRAINAGE: TYPE: CONDITION: FUNCTIONING: YES NO
 59.7 HINGES: CONDITION: FUNCTIONING: YES NO MOVEMENT: YES NO
 ALIGNMENT: GOOD BAD PAINT: G F P N U
 59.8 DEFLECTIONS: X NORMAL EXCESSIVE CMS.
 59.9 VIBRATIONS: X MINIMAL MODERATE EXCESSIVE

60- SUBSTRUCTURE: SATISFACTORY COND. RATING: 6

	← ABUTMENTS →				← PIERS OR NON PILES BENTS →					← PILES BENTS →		
	60.1.1	60.1.2	60.1.3	60.1.4	60.2.1	60.2.2	60.2.3	60.2.4	60.2.5	60.4.1	60.4.2	60.4.3
	WINGWALL	BREAST & BACKWALL	FOOTING	PILES	CAPS	BRACING	COLUMNS	FOOTINGS	PILES	CAPS	BRACING	PILES
MATERIAL							STEEL	CONCRETE				
CONDITION RATING	---	---	---	---	---	---	6	6	---	---	---	---
CRACKING (F-M-O)								F				
SPALLING (S-L)								S				
SCALING (L-M-H-S)								L				
CORROSION (L-M-S)												
PAINT (G-F-P-N-U)							F	F				
MOVEMENT							NO	NO				
EROSION							NO	NO				
UNDERMINING							NO	NO				

60- SUBSTRUCTURE COMMENTS:

BRIDGE No.: 01137

61- CHANNEL & CHANNEL PROTECTION : N/A

RATING : N

CHANNEL COMMENTS:

61.1 CHANNEL SCOUR (EXTENT) :

61.2 EMBANKMENT EROSION (EXTENT) :

	TYPE	MATERIAL	CONDITION RATING	FUNCTIONING	
				YES	NO
61.3 PROTECTIVE DEVICE					
61.4 FENDER SYSTEM					
61.5 RIP RAP					
61.6 SPUR DIKES, JETTIES					

61.7 OBSTRUCTION (DEBRIS, GROWTHS):

61.8 CHANNEL CHANGE: YES NO

DETRIMENTAL: YES NO IF YES, EXPLAIN

61.9 ADEQUATE WATERWAY: YES NO IF NO, EXPLAIN

61.10 SURROUNDING AREA: FLOODING: YES NO IF YES, EXPLAIN
 EROSION: YES NO IF YES, EXPLAIN

61.11 LOCATION OF PIERS AND/OR ABUTMENTS: DETRIMENTAL: YES NO
 IF YES, EXPLAIN

61.12 OTHER FEATURES THAT MAY AFFECT STRUCTURE:

62- CULVERT AND RETAINING WALLS :N/A

RATING : N

CULVERT COMMENTS:

	62.1 BARREL	62.2 HEADWALL	62.3 CUT-OFFWALL	62.4 RETAINING WALL
MATERIAL				
CONDITION RATING				
FUNCTIONING (Y-N)				
CRACKING (F-M-O)				
SPALLING (S-L)				
SCALING (L-M-H-S)				
CORROSION (L-M-S)				
SETTLEMENT (Y-N)				
ALIGNMENT (Y-N)				
PAINT (G-F-P-N-U)				

63- METHOD USED TO DETERMINE OPERATING RATING ..

2

64- OPERATING RATING : PEDESTRIAN

800

65- METHOD USED TO DETERMINE INVENTORY RATING ..

2

66- INVENTORY RATING : PEDESTRIAN

800

67- STRUCTURAL EVALUATION : 5

SEVERE CORROSION & LIGHT SECTION LOSS AT WEARING SURFACE DUE TO PONDING.
FINE CRACKS, SMALL SPALLINGS & LIGHT SCALINGS AT COLUMNS FOOTINGS. PONDING AT
STAIRS & REST AREA.

68- DECK GEOMETRY : N

N/A

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : 3

BASICALLY INTOLERABLE REQUIRING HIGH PRIORITY OF CORRECTIVE ACTION.

70- BRIDGE POSTING : N/A 5

71- WATERWAY ADEQUACY : N

N/A

REMOTE - GREATER THAN 100 YEARS. SLIGHT - 11 TO 100 YEARS. OCCASSIONAL - 3 TO 10 YEARS. FREQUENT - LESS THAN 3 YEARS.
INSIGNIFICANT - MINOR INCONVENIENCE. HIGHWAY PASSABLE IN A MATTER OF HOURS. SIGNIFICANT - TRAFFIC DELAYS OF UP TO SEVERAL DAYS
SEVERE - LONG TERM DELAYS TO TRAFFIC WITH RESULTING HARDSHIP

72- APPROACH ROADWAY ALIGNMENT : N/A

N/A

72.2 SLAB OR PAVEMENT CONDITION : MATERIAL :
CRACKING: F N O SPALLING: L S SCALING: L M H S UNNEVEN: YES NO ROUGH: YES NO
SETTLEMENT: YES NO MOVEMENT: APPROACH SLAB-BACKWALL: YES NO BREAKING UP: EXCESSIVE DEFORMATION:
SAFETY: HAZARDOUS YES NO DRAINAGE: INADEQUATE: YES NO PONDING: YES NO
MOVEMENT: PAVEMENT-APPROACH SLAB: YES NO EMBANKMENT: CONDITION:
FUNCTIONING: YES NO EROSION: YES NO IF YES, EXPLAIN
CONSTRUCTION: FLIMSAY: YES NO
INTEGRITY IMPAIRED: YES NO IF YES, EXPLAIN

72.3 UNDESIRABLE IMPACT: YES NO

72.4 JOINTS: TYPE: INADEQUATE: YES NO SATISFACTORY ALIGNMENT: YES NO

72.5 GUARDRAIL: TYPE: MATERIAL: CONDITION: FUNCTIONING: YES NO
ALIGNMENT: HORIZONTAL: GOOD BAD VERTICAL: GOOD BAD JOINTS: FUNCTIONING: YES NO
SAFETY: SECURELY ATTACHED: YES NO PEDESTRIAN HAZARDS: YES NO

72- APPROACH ROADWAY - COMMENTS:

BRIDGE NO. :01137

=====PROPOSED IMPROVEMENT=====

75- TYPE OF WORK :	REHABILITATION NEEDED	352
76- LENGTH OF STRUCTURE IMPROVEMENT :	52.14 MTS. (171.01 FT.)	000521

=====INSPECTIONS=====

89- RESERVED		--
90- INSPECTION DATE :	MARCH-11-1999	0399
91- DESIGNATED INSPECTION FREQUENCY :	EVERY-24-MONTHS	24
92- CRITICAL FEATURE INSPECTION :	NOT NEEDED	N N N
93- CRITICAL FEAT. INSP. DATE :	N/A	
FRACTURE CRITICAL	UNDERWATER	OTHER

=====IMPROVEMENT COST=====

94- BRIDGE IMPROVEMENT COST :	N/A	000000
95- ROADWAY IMPROVEMENT COST :	N/A	000000
96- TOTAL PROJECT COST :	(2.00) (52.14) (825) (0.15)=\$12,905	000016
97- YEAR OF IMPROVEMENT COST ESTIMATED :	1999	1999

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE :		--
99- BORDER BRIDGE STRUCTURE NUMBER :		--
100- STRAHNET HIGHWAY DESIGNATION :	IS A DH	1
101- PARALLEL STRUCTURE DESIGNATION :	N/A	N
102- DIRECTION OF TRAFFIC :	NOT CARRIED TRAFFIC	0
103- TEMPORARY STRUCTURE DESIGNATION :	N/A	
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE :	N/A NHS	10
105- FEDERAL LANDS HIGHWAYS :		0
106- YEAR RECONSTRUCTION :	N/A	0000
107- DECK STRUCTURE TYPE :	STEEL PLATE	5
108- WEARING SURF./ PROTECT. SYSTEM :	NONE	000
109- AVERAGE DAILY TRUCK TRAFFIC :	4% (EST)	04
110- DESIGNATED NATIONAL NETWORK :	PART OF NNT	1
111- PIER OR ABUT. PROTECTION (NAVIGATION) :	N/A	
112- NBIS BRIDGE LENGTH :	YES	Y
113- SCOUR CRITICAL BRIDGE :	N/A	N
114- FUTURE AVERAGE DAILY TRAFFIC :	92,073	092073
115- YR. OF FUTURE A.D.T. :	2016	2016
116- MINIM. NAVIG. VERT. CLEARANCE :	N/A	
VERTICAL LIPT BRIDGE :		
117- SUFFICIENCY RATING :		
125- PRIORITY RATING:		
127- EVALUATION DATE :	11-MAR-1999	031199
130- CRITICAL FRACTURE INSP. DATE :		

ENGINEER :HERIBERTO GONZALEZ

FEDERAL SYSTEM

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

PAGE 7 OF 7

BRIDGE NO. :01137

AUXILIARY ITEMS

1-SIGNS

TYPE:

MATERIAL:

CONDITION:

REMARKS:

REHABILITATION CONSISTS ON REPAIRING THE DEFICIENCIES DISCUSSED ON
ITEM 57.

D.5 August 19, 1996

PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REPORT -----		PAGE 1 OF 3 DATE : 08/19/96 TIME : 13:59:19
FEDERAL SYSTEM	BRIDGE NO. : 01137 - ROAD NO. : PR 26	- KM. NO. : 004.070
*****IDENTIFICATION*****		
1- STATE :	Commonwealth of Puerto Rico	721
2- STATE HIGHWAY DEPARTMENT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN	76770
5- INVENTORY ROUTE :	2-1-1-00003-0	211000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	PR 26	PR 26
8- STRUCTURE NUMBER :	1137 (1 OF 1)	011371
9- LOCATION :	BALDORIO DE CASTRO AVE.	BALDORIO DE CASTRO AVE.
10- INV. ROUTE, MIN. CLEAR. (0.01m) :	5.11m(16'-9")	1609
11- MILEPOINT :	4.07	004070
16- LATITUDE :	18 DEG 27.3 MIN	18273
17- LONGITUDE :	66 DEG 03.3 MIN	066033
19- BYPASS, DETOUR LENGTH (NEAREST MILE) :	STRUCTURE OVER HWY.	00
*****CLASSIFICATION*****		
20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE	11
*****AGE AND SERVICE*****		
27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	00 ON ; 05 UNDER	0005
29- A.D.T. OF INVENTORY ROUTE :	114,900	114900
30- YEAR OF A.D.T. :	1995	95
*****STRUCTURE DATA*****		
31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	24.59m(80.65ft)	081
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	NO	000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO	0000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HWY.	31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000
47- INV. ROUTE, TOTAL HORIZ. CLEARANCE :	L=23.59m(77.39ft) R=23.6m(77.4ft)	774
48- LENGTH OF MAXIMUM SPAN :	23.6m(77.4ft)	0077
49- STRUCTURE LENGTH :	52.14m(171.01ft)	000171
50- CURB OR SIDEWALK WIDTHS :	NONE	000000

PUERTO RICO HIGHWAY AUTHORITY
 BRIDGE STUDIES AND EVALUATION OFFICE

FEDERAL SYSTEM BRIDGE NO. : 01137 - ROAD NO. : PR 26 - KM. NO. : 004.070

=====STRUCTURE DATA=====

51- BRIDGE ROADWAY WIDTH, CURB TO CURB :	1.64m(5.37ft)	0054
52- DECK WIDTH, OUT TO OUT :	2.00m(6.56ft)	0066
53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY :	UNLIMITED	9999
54- MINIMUM VERTICAL UNDERCLEARANCE :	5.11m(16'-9")	H1609
55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT :	1.19m(3.9ft)	H039
56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT :	2.22m(7.28ft)	073

-----CONDITION-----

	MATERIAL	CONDITION ANALYSIS	
58- DECK	STEEL	FAIR CONDITION	5
59- SUPERSTRUCTURE	STEEL	FAIR CONDITION	5
60- SUBSTRUCTURE	STEEL	SATISFACTORY COND.	6
61- CHANNEL & CHANNEL PROTECTION	N/A	N/A	N
62- CULVERT	N/A	N/A	N
64- OPERATING RATING :		PEDESTRIAN	800
66- INVENTORY RATING :		PEDESTRIAN	800
67- STRUCTURAL EVALUATION :			5
PEDESTRIAN HAS LIGHT CORROSION & LIGHT SECTION LOSS. BRIDGE WAS PAINTED.			

68- DECK GEOMETRY :
 N/A

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : 3
 BASICALLY INTOLERABLE REQUIRING HIGH PRIORITY OF CORRECTIVE ACTION.

70- BRIDGE POSTING : N/A

71- WATERWAY ADEQUACY : N
 N/A

72- APPROACH ROADWAY ALIGNMENT : N
 N/A

=====PROPOSED IMPROVEMENT=====

75- TYPE OF WORK :	REHABILITATION NEEDED	352
76- LENGTH OF STRUCTURE IMPROVEMENT :	52.14 MTS. (171.01 FT.)	000171

JMS

PUERTO RICO HIGHWAY AUTHORITY
 BRIDGE STUDIES AND EVALUATION OFFICE

FEDERAL SYSTEM BRIDGE NO. : 01137 - ROAD NO. : PR 26 - KM. NO. : 004.070

=====INSPECTIONS=====

89- RESERVED	---
90- INSPECTION DATE : AUGUST 16,1996	0896
91- DESIGNATED INSPECTION FREQUENCY : EVERY-24-MONTHS	24
92- CRITICAL FEATURE INSPECTION : NOT NEEDED	N N N
93- CRITICAL FEAT. INSP. DATE : N/A	

=====IMPROVEMENT COST=====

94- BRIDGE IMPROVEMENT COST : N/A	000000
95- ROADWAY IMPROVEMENT COST : N/A	000000
96- TOTAL PROJECT COST : (2.00) (52.14) (1035) (0.15)=\$16,189	000016
97- YEAR OF IMPROVEMENT COST ESTIMATED : 1996	96

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE :	---
99- BORDER BRIDGE STRUCTURE NUMBER :	---
100- DEFENSE HIGHWAY DESIGNATION : IS A DH	1
101- PARALLEL STRUCTURE DESIGNATION : N/A	N
102- DIRECTION OF TRAFFIC : NOT CARRIED TRAFFIC	0
103- TEMPORARY STRUCTURE DESIGNATION : N/A	
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE : NHS	1
105- RESERVED	---
106- YEAR RECONSTRUCTION : N/A	0000
107- DECK STRUCTURE TYPE : STEEL PLATE	5
108- WEARING SURF./ PROTECT. SYSTEM : NONE	000
109- AVERAGE DAILY TRUCK TRAFFIC : 43 (EST)	04
110- DESIGNATED NATIONAL NETWORK : PART OF NNT	1
111- PIER OR ABUT. PROTECTION (NAVIGATION) : N/A	
112- NBIS BRIDGE LENGTH : YES	Y
113- SCOUR CRITICAL BRIDGE : N/A	N
114- FUTURE AVERAGE DAILY TRAFFIC : 136,100	136100
115- YR. OF FUTURE A.D.T. : 2012	12
116- MINIM. NAVIG. VERT. CLEARANCE : N/A	
117- SUFFICIENCY RATING :
126- FIELD INSPECTION DATE : 081696	081696
127- INSPECTION EVAL. DATE : 081696	081696
130- CRITICAL FRACTURE INSP. DATE :	
131- EVALUATOR ENGINEER :	

REMARKS:

REHABILITATION CONSISTS ON REPAIRING THE DEFICIENCIES DISCUSSED ON
 ITEM 67.

D.6 December 22, 1994

PUERTO RICO HIGHWAY AUTHORITY HIGHWAY SYSTEM ADMINISTRATION OFFICE BRIDGE REPORT -----		PAGE 1 OF 3 DATE : 12/22/94 TIME : 18:46:21
FEDERAL SYSTEM	BRIDGE NO. : 01137 ~ ROAD NO. : PR 26	- KM. NO. : 004.070
=====IDENTIFICATION=====		
1- STATE :	Commonwealth of Puerto Rico	721
2- STATE HIGHWAY DEPARTMENT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN	76770
5- INVENTORY ROUTE :	2-1-1-00003-0	211000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	PR 26	PR 26
8- STRUCTURE NUMBER :	1137 (1 OF 1)	011371
9- LOCATION :	BALDORIOTY DE CASTRO AVE.	BALDORIOTY DE CASTRO AVE.
10- INV. ROUTE, MIN. CLEAR. (0.01m) :	5.11m(16'-9")	1609
11- MILEPOINT :	4.07	004070
16- LATITUDE :	18 DEG 27.3 MIN	18273
17- LONGITUDE :	66 DEG 03.3 MIN	066033
19- BYPASS, DETOUR LENGTH (NEAREST MILE) :	STRUCTURE OVER HWY.	00
=====CLASSIFICATION=====		
20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE	11
=====AGE AND SERVICE=====		
27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	00 ON ; 05 UNDER	0005
29- A.D.T. OF INVENTORY ROUTE :	109,500	109500
30- YEAR OF A.D.T. :	1993	93
=====STRUCTURE DATA=====		
31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	24.59m(80.65ft)	081
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	NO	000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO	0000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HWY.	31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000
47- INV. ROUTE, TOTAL HORIZ. CLEARANCE :	L=23.59m(77.39ft) R=23.6m(77.4ft)	774
48- LENGTH OF MAXIMUM SPAN :	23.6m(77.4ft)	0077
49- STRUCTURE LENGTH :	52.14m(171.01ft)	000171
50- CURB OR SIDEWALK WIDTHS :	NONE	000000

PUERTO RICO HIGHWAY AUTHORITY
HIGHWAY SYSTEM ADMINISTRATION OFFICE

FEDERAL SYSTEM BRIDGE NO. : 01137 - ROAD NO. : PR 26 - KM. NO. : 004.070

=====STRUCTURE DATA=====

51- BRIDGE ROADWAY WIDTH, CURB TO CURB :	1.64m(5.37ft)	0054
52- DECK WIDTH, OUT TO OUT :	2.00m(6.56ft)	0066
53- MIN. VERT. CLEAR. OVER BRIDGE ROADWAY :	UNLIMITED	9999
54- MINIMUM VERTICAL UNDERCLEARANCE :	5.11m(16'-9")	H1609
55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT :	1.19m(3.9ft)	H039
56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT :	2.22m(7.28ft)	073

=====CONDITION=====

	MATERIAL	CONDITION ANALYSIS	
58- DECK	STEEL	FAIR CONDITION	5
59- SUPERSTRUCTURE	STEEL	FAIR CONDITION	5
60- SUBSTRUCTURE	STEEL	SATISFACTORY COND.	6
61- CHANNEL & CHANNEL PROTECTION	N/A	N/A	N
62- CULVERT.....	N/A	N/A	N
64- OPERATING RATING :		PEDESTRIAN	800
66- INVENTORY RATING :		PEDESTRIAN	800

67- STRUCTURAL EVALUATION : 5
 SEVERE CORROSION & LIGHT SECTION LOSS AT WEARING SURFACE DUE TO PONDING.
 FINE CRACKS, SMALL SPALLINGS & LIGHT SCALINGS AT COLUMNS FOOTINGS, PONDING AT STAIRS & REST AREA.

68- DECK GEOMETRY : N
 N/A

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : 3
 BASICALLY INTOLERABLE REQUIRING HIGH PRIORITY OF CORRECTIVE ACTION.

70- BRIDGE POSTING : N/A N

71- WATERWAY ADEQUACY : N
 N/A

72- APPROACH ROADWAY ALIGNMENT : N
 N/A

=====PROPOSED IMPROVEMENT=====

75- TYPE OF WORK :	REHABILITATION NEEDED	352
76- LENGTH OF STRUCTURE IMPROVEMENT :	52.14 MTS. (171.01 FT.)	000171

PUERTO RICO HIGHWAY AUTHORITY
HIGHWAY SYSTEM ADMINISTRATION OFFICE

PAGE 3 OF 3

FEDERAL SYSTEM BRIDGE NO. : 01137 - ROAD NO. : PR 26 - KM. NO. : 004.070

=====INSPECTIONS=====

89- RESERVED	---
90- INSPECTION DATE : DECEMBER 1994	1294
91- DESIGNATED INSPECTION FREQUENCY : EVERY-24-MONTHS	24
92- CRITICAL FEATURE INSPECTION : NOT NEEDED	N N N
93- CRITICAL FEAT. INSP. DATE : N/A	

=====IMPROVEMENT COST=====

94- BRIDGE IMPROVEMENT COST : N/A	000000
95- ROADWAY IMPROVEMENT COST : N/A	000000
96- TOTAL PROJECT COST : (2.00) (52.14) (825) (0.15)=\$12,905	000013
97- YEAR OF IMPROVEMENT COST ESTIMATED : 1994	94

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE :	--
99- BORDER BRIDGE STRUCTURE NUMBER :	---
100- DEFENSE HIGHWAY DESIGNATION : IS A DH	1
101- PARALLEL STRUCTURE DESIGNATION : N/A	N
102- DIRECTION OF TRAFFIC : NOT CARRIED TRAFFIC	0
103- TEMPORARY STRUCTURE DESIGNATION : N/A	
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE : NHS	1
105- RESERVED	---
106- YEAR RECONSTRUCTION : N/A	0000
107- DECK STRUCTURE TYPE : STEEL PLATE	5
108- WEARING SURF./ PROTECT. SYSTEM : NONE	000
109- AVERAGE DAILY TRUCK TRAFFIC : 4%(EST)	04
110- DESIGNATED NATIONAL NETWORK : PART OF NNT	1
111- PIER OR ABUT. PROTECTION (NAVIGATION) : N/A	
112- NBIS BRIDGE LENGTH : YES	Y
113- SCOUR CRITICAL BRIDGE : N/A	N
114- FUTURE AVERAGE DAILY TRAFFIC : 136,100	136100
115- YR. OF FUTURE A.D.T. : 2012	12
116- MINIM. NAVIG. VERT. CLEARANCE : N/A	
117- SUFFICIENCY RATING :	
126- FIELD INSPECTION DATE : 121294	121294
127- INSPECTION EVAL. DATE : 121294	121294
130- CRITICAL FRACTURE INSP. DATE :	
131- EVALUATOR ENGINEER :	

REMARKS:

REHABILITATION CONSISTS ON REPAIRING THE DEFICIENCIES DISCUSSED ON
ITEM 67.

D.7 February 24, 1993

PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REPORT		PAGE 1 OF 3 DATE : 02/24/93 TIME : 12:16:09
FEDERAL SYSTEM	BRIDGE NO. : 01137- ROAD NO. : PR 26	KM. NO. : 004070
1- STATE :	Commonwealth of Puerto Rico	721
2- STATE HIGHWAY DEPARTMENT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN	76770
5- INVENTORY ROUTE :	2-1-1-00003-0	21100030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	PR 26	PR 26
8- STRUCTURE NUMBER :	1137 (1 OF 1)	011371
9- LOCATION :	BALDORIOY DE CASTRO AVE.	BALDORIOY DE CASTRO AVE.
10- INV. ROUTE, MIN. VERT. CLEAR. (0.01#) :	5.11m (16'-9")	1609
11- MILEPOINT :	4.07	004870
16- LATITUDE :	18 DEG 27.3 MIN	18273
17- LONGITUDE :	66 DEG 03.3 MIN	066033
19- BYPASS, DETOUR LENGTH (NEAREST MILE) :	STRUCTURE OVER HWY.	00
20- TOLL :	UN FREE ROAD	3
21- MAINTENANCE RESPONSIBILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE	11
27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	00 ON ; 03 UNDER	0005
29- A. D. T. OF INVENTORY ROUTE :	95,400	095400
30- YEAR OF A. D. T. :	1990	90
31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	24.59m (80.65ft)	081
33- BRIDGE MEDIAN :	NONE	0
34- SKEW :	NO	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY PERFORMED :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	NO	000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO	0000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HWY.	31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000
47- INV. ROUTE, TOTAL HORIZ. CLEARANCE :	L=23.59m (77.39ft) R=23.6m (77.4ft)	774
48- LENGTH OF MAXIMUM SPAN :	23.6m (77.4ft)	0077
49- STRUCTURE LENGTH :	52.14m (171.01ft)	000171
50- CURB OF SIDEWALK WIDTHS :	NONE	000000
51- BRIDGE ROADWAY WIDTH, CURB-TO-CURB :	1.64m (5.37ft)	0054
52- DECK WIDTH, OUT-TO-OUT :	2.00m (6.56ft)	0066
53- MIN. VERT. CLEAR. OVER BRIDGE ROADWAY :	UNLIMITED	9999
54- MINIMUM VERTICAL UNDERCLEARANCE :	5.11m (16'-9")	41609
55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT :	1.19m (3.94ft)	0039
56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT :	2.22m (7.29ft)	073

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REPORT

PAGE 2 OF 3

FEDERAL SYSTEM BRIDGE NO. : 01137- ROAD NO. : PR 26 -KM. NO. : 094070

	MATERIAL	CONDITION ANALYSIS	
58- DECK	STEEL	SATISFACTORY COND.	3
59- SUPERSTRUCTURE	STEEL	SATISFACTORY COND.	5
60- SUBSTRUCTURE	STEEL	SATISFACTORY COND.	6
61- CHANNEL & CHANNEL PROTECTION	N/A	N/A	N
62- CULVERT	N/A	N/A	N
64- OPERATING RATING	PEDESTRIAN	PEDESTRIAN	800
65- INVENTORY RATING	PEDESTRIAN	PEDESTRIAN	500
67- STRUCTURAL EVALUATION :	THIS BRIDGE WAS REHABILITATED IN 1992 AND ITS CON- DITION IS SATISFACTORY. SOUTH STAIR HAS A LITTLE DEFLECTION IN FIRST LEVEL. ON WALKWAY THERE'S RONDING. OTHER PARTS OF STRUC- TURE ARE PAINTED AND IN ACCEPTABLE CONDITION.		6
68- DECK GEOMETRY :			N
	N/A		
69- UNDERCLEARANCE, VERTICAL & HORIZONTAL :			3
	BASICALLY INTOLERABLE REQUIRING HIGH PRIORITY OF CORRECTIVE ACTION.		
70- BRIDGE POSTING :			N
	N/A		
71- WATERWAY ADEQUACY :			N
	N/A		
72- APPROACH ROADWAY ALIGNMENT :			N
	N/A		
75- TYPE OF WORK :	NO IMPROVEMENT NEEDED		000
76- LENGTH OF STRUCTURE IMPROVEMENT :	N/A		000000
87- RESERVE :			---
89- INSPECTION DATE :	FEBRUARY-24-1993		0293
91- DESIGNATED INSPECTION FREQUENCY :	EVERY-24-MONTHS		24
92- CRITICAL FEATURE INSPECTION :	NOT NEEDED		N N N
93- CRITICAL FEATURE INSPECTION DATE :	N/A		

PUERTO RICO HIGHWAY AUTHORITY
 BRIDGE STUDIES AND EVALUATION OFFICE
 BRIDGE REPORT

PAGE 3 OF 3

FEDERAL SYSTEM BRIDGE NO. : 01137- ROAD NO. : PR 26 -RM. NO. : 004070

94- BRIDGE IMPROVEMENT COST :	---	00000
95- ROADWAY IMPROVEMENT COST :	---	00000
96- TOTAL PROJECT COST :	---	00000
97- YEAR OF IMPROVEMENT COST ESTIMATED :	---	99
98- BORDER BRIDGE :		---
99- BORDER BRIDGE STRUCTURE NUMBER :		---
100- DEFENSE HIGHWAY DESIGNATION :	IS A DM	1
101- PARALLEL STRUCTURE DESIGNATION :	N/A	N
102- DIRECTION OF TRAFFIC :	NOT CARRIED TRAFFIC	0
103- TEMPORARY STRUCTURE DESIGNATION :	N/A	-
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE :	INTERSTATE	1
105- RESERVED :		---
106- YEAR RECONSTRUCTED :	REHABILITATE IN 1992	1992
107- DECK STRUCTURE TYPE :	STEEL PLATE	5
108- WEARING SURF./ PROTECT. SYSTEM :	NONE	000
109- AVERAGE DAILY TRUCK TRAFFIC :	64	04
110- DESIGNATED NATIONAL NETWORK :	PART OF NN	1
111- PIER OR ABUT. PROTECTION (NAVIGATION) :	N/A	-
112- NBIS BRIDGE LENGTH :	YES	7
113- SCOUR CRITICAL BRIDGES :	N/A	N
114- FUTURE AVERAGE DAILY TRAFFIC :	145,970	145970
115- YR. OF FUTURE A.D.T. :	2010	10
116- MINIM. NAVIG. VERT. CLEARANCE :	N/A	---
117- SUFFICIENCY RATINGS :		
126- FIELD INSPECTION DATE :	021093	021093
127- INSPECTION EVAL. DATE :	022693	022693
130- CRITICAL FRACTURE IMP. DATE :		
131- EVALUATOR ENGINEER :		



REMARKS:

THIS BRIDGE WAS REHABILITATED LAST YEAR (1992) AND NO IMPROVEMENT IS NEEDED.

COMMONWEALTH OF PUERTO RICO
HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE

APPRAISAL OF ITEM 68

BRIDGE NO. 1137

ROAD NO. PR-26

KM. NO. 4.07

Year	ADT	Sec.	TW	TW Adequate or Inadequate	TW Inadequate By	TW + SH	TW + SH Adequate or Inadequate	TW + SH Inadequate By
1990	95,400	<input checked="" type="checkbox"/>	Actual =	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT =					
			N/A		PE DESTRIAN			
2010	143,990		Reqd. For ADT =			1		
			Rec. PRHA =					

✓ Adequate
x Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE
 PUERTO RICO HIGHWAY AUTHORITY

1137 0026 004.07
 Bridge No. Road No. Km.No.

BRIDGE INVENTORY

IDENTIFICATION

Code Position

25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	---	---	---	---	---	---	---	---	---

1. State Code _____ Commonwealth of Puerto Rico 7 2 1
2. State Highway Department District San Juan 0 1
3. County (Parish) Code San Juan 1 2 7
4. Place Code Urban 7 6 7 7 0
5. Inventory Route 2-1-1-00013-0 2 1 1 0 0 0 0 3 0
6. Features Intersected Pedestrian Walkway
P E D E S T R I A N W A L K W A Y
7. Facility Carried by Structure PR-26
P R 2 6
8. Structure Number 1137 1st fl. 0 0 0 0 0 0 0 0 0 0 1 1 3 7 1
9. Location Baldorioty de Castro Avenue
B A L D O R I O T Y D E C A S T R O A V E.
10. Inventory Route, Minimum Vertical Clearance (0.01 m) 5.11m(16.76) 1 6 0 9
11. Milepoint 4.07 0 0 4 0 7 0
16. Latitude _____ 18 Degrees 27.3 Minutes 1 8 2 7 3
17. Longitude _____ 66 Degrees 03.3 Minutes 0 6 6 0 3 3
19. By Pass, Detour Length (Nearest mile) Structure over Highway 0 0

CLASSIFICATION

20. Toll Toll Free 3
21. Maintenance Responsibility D. H. P. W. 0 1
22. Owner State Highway Dept. 0 1
26. Functional Classification of Inventory Route Urban Interstate 1 1

6	5	4	3	2	1
---	---	---	---	---	---

AGE AND SERVICE

27. Year Built 1968 1 9 6 8

28. Lanes on the Structure 0 and Under the Structure 5 0 0 0 5

29. Average Daily Traffic 93,500 0 9 3 5 0 0

30. Year of Average Daily Traffic 1989 8 9

STRUCTURE DATA

31. Design Load Pedestrian 7

32. Approach Roadway Width 24.59m (80.65') 0 2 4 5 9

33. Bridge Median None Open Closed Closed NM 0

34. Skew 0 0 0

35. Structure Flared Yes No 0

36. Traffic Safety Features M N M N M M N M N M

37. Historical Significance 5 5

38. Navigation Control Yes No NA N A

39. Navigation Vertical Clearance Yes No 0 0 0

40. Navigation Horizontal Clearance Yes No 0 0 0 0

41. Structure, Open, Posted, or Closed to Traffic open A

42. Type of Service Pedestrian over Highway 3 1

43. Structure Type, Main Steel Truss Deck 3 0 9

44. Structure Type, Approach Spans None 0 0 0

45. Number of Spans in Main Unit 2 0 0 2

46. Number of Approach Spans None 0 0 0 0

47. Inventory Route, Total Horizontal Clearance L = 23.89m (77.29') R = 23.60m (77.40') 7 7 4

48. Length of Maximum Span 23.60m (77.40') 0 0 7 7

49. Structure Length 52.14m (171.01') 0 0 0 1 7 1

50. Curb or Sidewalk Widths I = None R = None 0 0 0 0 0 0

51. Bridge Roadway Width, Curb-to-Curb 1.64m (5.37') 0 0 5 4

52. Deck Width, Out-to-Out 2.00m (6.56') 0 0 6 6

53. Minimum Vertical Clearance Over Bridge Roadway Unlimited 9 9 9 9

54. Minimum Vertical Underclearance H 3.11m (10.20') R N H 3 1 1

55. Minimum Lateral Underclearance on Right H 1.19m (3.90') R N H 1 1 9

56. Minimum Lateral Underclearance on Left 2.22m (7.28') 0 7 2

3	2	1
---	---	---

CONDITIONS RATINGS

	Material	Condition	
58. Deck	Steel	Fair condition	<input checked="" type="checkbox"/>
59. Superstructure	Steel	Fair condition	<input checked="" type="checkbox"/>
60. Substructure	Steel & Conc.	Fair condition	<input checked="" type="checkbox"/>
61. Channel and Channel Protection	N/A	N/A	<input type="checkbox"/>
62. Culverts	N/A	N/A	<input type="checkbox"/>
64. Operating Rating	Probation		<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>
66. Inventory Rating	Probation		<input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>

APPRAISAL RATING

Deficiencies

67. Structural Evaluation	Good Place as described and located. Some corrosion on steel deck and girders, beams with some corrosion. Vertical columns have some abrasion.	<input checked="" type="checkbox"/>
68. Deck Geometry	N/A	<input type="checkbox"/>
69. Underclearances, Vertical and Horizontal	Condition is adequate for traffic. No signs of any problems at present.	<input type="checkbox"/>
70. Bridge Posting		<input type="checkbox"/>
71. Waterway Adequacy	N/A	<input type="checkbox"/>
72. Approach Roadway Alignment	N/A	<input type="checkbox"/>

6	5	4	3	2	1
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CLASSIFICATION AND STRUCTURE DATA (Cont.)

106. Year Reconstructed	<u>1921</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>
107. Deck Structure Type	<u>Steel Plate</u>			<u>5</u>	
108. Wearing Surface/Protective System	<u>None</u>	<u>2</u>	<u>0</u>	<u>0</u>	
109. Average Daily Truck Traffic	<u>4%</u>	<u>0</u>	<u>4</u>		
110. Designated National Network	<u>PDNN</u>			<u>1</u>	
111. Pier or Abutment Protection (for Navigation)	<u>-</u>			<u>-</u>	
112. NBIS Bridge Length	<u>90</u>			<u>9</u>	
113. Scour Critical Bridges	<u>N/A</u>			<u>N</u>	
114. Future Average Daily Traffic	<u>100,000</u>	<u>1</u>	<u>0</u>	<u>0</u>	<u>0</u>
115. Year of Future Average Daily Traffic	<u>2010</u>			<u>1</u>	<u>0</u>
116. Minimum Navigation Vertical Clearance	<u>N/A</u>	<u>-</u>	<u>-</u>	<u>-</u>	
Vertical Lift Bridge					

Remarks:

The rehab of deck of this bridge was done in 1992 - see notes in 106008 107000 108000

BRIDGE NO. 1137

APPRAISAL OF ITEM 58

ROAD NO. 20 KM. 4.07

BRIDGE STUDIES AND
EVALUATION OFFICE

YEAR	ADT	Sec.	TW	TW Adequate or Inadequate	TW Inadequate By	TW + SH	TW + SH Adequate or Inadequate	TW + SH Inadequate By
1985		X	Actual =	X	X		X	X
			For ADT =					
1995			Reqd. For ADT =					
			Rec. PRHA =					

- ✓ Adequate
- X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE
PUERTO RICO HIGHWAY AUTHORITY

GENERAL EVALUATION

This bridge is basically in 2 structural condition [2]

excellent	4	fair	2	very poor	0
adequate	3	poor	1		

The load carrying capacity is 0 [0]

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
----------	---	--------------------	---	------------	---	----------------------------	---

The recommended modifications will make it [1]

adequate	2	minimally adequate	1	no required (don't print)	0
				0 when previous is	2

The bridge should be replaced. [0]

Yes (print)	1	not required (don't print)	0
-------------	---	----------------------------	---

It requires 2 0 0 0 [2][0][0][0]

routine inspection every <u>24</u> months	(A) = 2	(B) = 24
frequent inspection every <u>(B)</u> months to monitor abnormal and/or suspected deficiencies	(A) = 1	(B) = months
frequent inspection every <u>(B)</u> months to determine the cause of and remedies for existing defects	(A) = 0	(B) = months

It requires 2 [0]

routine maintenance	[3]
minor repairs and routine maintenance	[2]
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	[1]
emergency repairs to eliminate danger to the public and routine maintenance	[0]

The deck geometry is 0

For present ADT, the travelled way is 0

For future (2010) ADT, the travelled way is 0

For PRHA (2010) recommendations, the travelled way is 0

For present ADT, the shoulders are 0

The future (2010) ADT, the shoulders are 0

For PRHA (2010) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
	0	0	0	0	0	0
	Travelled way			Shoulders		

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print)		1			

The (A) approach alignment is (B) [0][0]

<u>(A)</u> = vertical	1	horizontal	0	horizontal and vertical	2
<u>(B)</u> = satisfactory	3	minimally tolerable	2	poor and represents a danger to the public	1
not required (don't print)		0			

Load Post for inventory rating shown in Item 64 [0]

Yes	1	No (don't print)	0
-----	---	------------------	---

Post for vertical underclearance shown in Items 54 and 69 [0]

Yes	1	No (don't print)	0
-----	---	------------------	---

FOR "REMARKS" SEE BRIDGE FILE.

PERSONNEL	By	Date
Structural Inspection	<u>Randy Rojas</u>	<u>6/22/91</u>
Review of Field Data	<u>[Signature]</u>	
Transfers of Data		
Condition Analysis	<u>[Signature]</u>	
Appraisal	<u>[Signature]</u>	<u>6/19/91</u>

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM-370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

Road No. Bridge No. Road No.

BRIDGE INVENTORY

Km. No. 4.07

IDENTIFICATION

Code Positions

	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1																	
1. State	Commonwealth of Puerto Rico																							10	7	2	1															
2. Highway District	San Juan																							13	0	1																
3. Municipality	San Juan																							15	0	6	3															
4. City/Town	San Juan																							18	0	0	6	3														
5. Inventory Route	7-4-1-00003-0																							22	2	1	1	0	0	0	0	3	0									
6. Features Intersected	Pedestrian Walkway																							31	P	E	D	E	A	S	T	R	I	A	N	W	A	L	K	W	A	Y
7. Facility Carried by Structure ⁵⁶	PR → 76																							74	0	1	1	3	7	1												
8. Structure No.	1137																							74	0	1	1	3	7	1												
9. Location ¹⁰	Balderrío de Castro Avenue																																									
10. Inventory Route, Minimum Vertical Clearance (0.01 m)	5.11																							10	0	5	1	1														
11. Kilometer Point (0.01 km)	4.07																							14	0	0	4	0	7													
12. Road Section Number (DOD)	25																							19	0	0	2	5	-													
13. Bridge Description	WH NTS PS																							24	-	-																
14. Defense Milepoint (0.01 ml)	2.62																							25	0	2	6	2														
15. Defense Section Length (miles) (0.1 mile)	8.10																							30	0	8	1															
16. Latitude: 18 Degrees 27.3 Minutes																								33	1	8	2	4	3													
17. Longitude: 66 Degrees 03.3 Minutes																								38	0	1	6	6	0	3	3											
18. Physical Vulnerability	Steel Truss Structure over highway																							44	4																	
19. By Pass, Detour Length (Nearest mile)	Toll Free																							45	0	0																
20. Toll	W. J.P.W.																							47	3																	
21. Custodian	W. J.P.W.																							48	1																	
22. Owner	State Highway Department																							49	1																	
23. F.A.P. No. ⁵⁰	Unknown																																									

CLASSIFICATION

24. Fed. Aid System	Interstate, Urban																							10	0	2
25. Administrative	State																							12	1	
26. Functional	Interstate, Urban																							13	4	1

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 10 0800

28. Lanes on Str. 0 Under 5 19 0005

29. ADT - Inventory Route 100,000 (estimated) 23 100000

30. Year 1985 20 85

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) 24.59m 32 246

33. Median None Open Closed 35 0

34. Skew none 36 00

35. Structure Flared Yes No 38 0

36. Traffic Safety Features W M M M M 39 W M M M M

37. Historical Significance 45 5

38. Navigation Control Yes No 46 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 000

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 00000

41. Structure, Open or Closed to Traffic Open 54 4

42. Type Service Pedestrian over Highway 55 31

43. Structure Type-Main Steel Truss Deck 57 309

44. Structure Type-Approach Spans None 58 000

45. No. of Spans-Main 7 10 007

46. No. of Spans-Approaches None 13 0000

47. Total Horizontal Clearance (0.01 m) L = 73.59 R = 23.60m 17 236

48. Max. Span Length (0.01 m) 23.60m 20 0236

49. Structure Length (0.01 m) 52.14m 24 000521

50. Sidewalk Widths (0.01 m) Left none Right none 30 000000

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.64m 36 0016

52. Deck Width (out-out) (0.01 m) 2.00m 40 0020

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) Unlimited 44 9999

54. Vertical Underclearance - Minimum (0.01 m) 5.11m 46 0511

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.27 R = 1.19 52 0127

56. Lateral Underclearance on Left (Inner) Sides - Minimum (0.01 m) L = 2.22 R = 2.19 55 0222

57. Wearing Surface steel 58 7

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION	Material	Condition Analysis	9 8 7 6 5 4 3 2 1										
			58. Deck	Steel	Fair condition								
59. Superstructure	Steel	"										60	6
60. Substructure	Steel	"										61	6
61. Channel & Channel Protection	N/A	N/A										62	N
62. Culvert & Retaining Walls	N/A	N/A										63	N
63. Estimated Remaining Life		10 years										64	10
64. Operating Rating		Pedestrian										66	800
65. Approach Alignment		N/A										69	N
66. Inventory Rating		Pedestrian										70	800

APPRAISAL

	Deficiencies		
67. Structural Condition	Ponding, holes & rust stains at deck steel plates. Severe corrosion at angles. Moderate corrosion at some structural members. Moderate corrosion at columns. Fine cracks, small spallings & light scuffings at footings. Stairs railings are broken due to moderate corrosion.	10	2
68. Deck Geometry	N/A	11	N
69. Underclearances-Vert. & Lateral	(5.1m) Criteria. Condition equal to present desirable	12	8
70. Safe Load Capacity	N/A	13	N
71. Waterway Adequacy	N/A	14	N
72. Approach Alignment	N/A	15	N

PROPOSED IMPROVEMENTS

73. Year Needed	1989	16	8	9				
Completed								
Described								
74. Type of Service	Pedestrian	18	3					
75. Type of Work	Rehabilitation	19	3	9	2			
76. Improvement Length (0.1 m)	N/A	22	0	0	0	0	0	0
77. Design Loading	N/A	28	0					

BRIDGE STUDIES AND EVALUATION OFFICE

9 8 7 6 5 4 3 2 1

- 78. Roadway Width (0.01m) *R/A* "0000"
- 79. Number of Lanes *R/A* "00"
- 80. ADT *130,080* "130580"
- 81. Year of Estimated ADT *1995* "95"
- 82. Year of Proposed Adjacent Roadway Improvements *R/A* "00"
- 83. Prop. Adj. Rdwy Improvements-Type *R/A* "0"

COST OF IMPROVEMENTS

- 84 Total (dollars)\$ *\$ 5,000* "00005"
- Estimated Design Time (months)

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars)..... *-* "000"
- 86. Demolition Cost (Dollars)..... *-* "000"
- 87. Substructure (Dollars)..... *-* "00000"
- 88. Superstructure (Dollars)..... *-* "00000"
- 89. Blank..... *-* "
- 90. Date of Last Inspection *August 18, 1989* "881889"
- 91. Rehabilitate Existing Structure (Dollars)..... *5,000* "005"
- 92. Detour and Traffic Maintenance (Dollars)..... *-* "000"
- 93. Approaches (Dollars)..... *-* "000"
- Approach Embankment (Dollars).....
- Approach Pavement (Dollars).....
- Approach Guardrail (Dollars).....
- *(Code to nearest thousand dollars).....

REMARKS *The rehabilitation consists on repair all deficiencies discussed on item 67*

**BRIDGE STUDIES AND EVALUATION OFFICE
GENERAL EVALUATION**

This bridge is basically in 2 structural condition 16

excellent	4	fair	2	very poor	0
good	3	poor	1		

The load carrying capacity is 0 17

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
----------	---	--------------------	---	------------	---	----------------------------	---

The recommended modifications will make it 1 18

adequate	2	minimally adequate	1	not required (don't print)	0
				0 when previous is	2

The bridge should be replaced. 19

Yes (print)	1	Not required (don't print)	0
-------------	---	----------------------------	---

It requires 20 24 20

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 2 23

routine maintenance	3
minor repairs and routine maintenance	2
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	1
emergency repairs to eliminate danger to the public and routine maintenance	0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1995) ADT, the travelled way is 0

For PRHA (1995) recommendations, the travelled way is 0 24

Deck	Present	Future	PRHA	Present	Future	PRHA
Travelled way	0	0	0	0	0	0
Shoulders						

For present ADT, the shoulders are 0

For future (1995) ADT, the shoulders are 0

For PRHA (1995) recommendations, the shoulders are 0

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print)					0

The (A) approach alignment is (B) 31

(A) vertical	1	horizontal	0	horizontal and vertical	2
(B) satisfactory	3	minimally tolerable	2	poor and represents a danger to the public	1
not required (don't print)					0

Load Post for inventory rating shown in Item 64 33

Yes	1	No (don't print)	0
-----	---	------------------	---

Post for vertical underclearance shown in Items 54 & 69 34

Yes	1	No (don't print)	0
-----	---	------------------	---

For "Remarks" see bridge file.

PERSONNEL	By <u>J. J. [Signature]</u>	Date <u>6/23/89</u>	Condition Analysis	By <u>[Signature]</u>	Date <u>8/18/89</u>
Structural Inspection			Appraisal		
Review of Field Data			IBM MT/SC file		
Transfer of Data			IBM-370 file		

BRIDGE NO. 1137

APPRAISAL OF ITEM **88**

ROAD NO. 26 KM. 4.07

BRIDGE STUDIES AND
EVALUATION OFFICE

YEAR	ADT	Sec.	TW	TW Adequate or Inadequate	TW Inadequate By	TW + SH	TW+SH Adequate or Inadequate	TW + SH Inadequate By
1985	N/A	<input checked="" type="checkbox"/>	Actual =	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	—	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT =	—	—	—	—	—
1995	N/A	<input type="checkbox"/>	Reqd. For ADT =	—	—	—	—	—
			Req. PRHA =	—	—	—	—	—

✓ Adequate
X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM--370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

4 No.

Bridge No.

Road No.

BRIDGE INVENTORY

Km. No. 4.07

IDENTIFICATION

Code Positions

25 24 23 22 21 20 19 18 17 16 15 14 13 12 11 10 9 8 7 6 5 4 3 2 1

1. State Commonwealth of Puerto Rico ¹⁰

2. Highway District *San Juan* ¹³

3. Municipality *San Juan* ¹⁵

4. City/Town *San Juan* ¹⁶

5. Inventory Route *2-1-1-00003-0* ²²

6. Features Intersected *Pedestrian Walkway* ³¹

7. Facility Carried by Structure ³⁶ *PR-26*

8. Structure No. *1137* ⁷⁴

9. Location ¹⁰ *Baldorioty de Castro Avenue*

10. Inventory Route, Minimum Vertical Clearance (0.01 m) *5.11* ¹⁰

11. Kilometer Point (0.01 km) *4.07* ¹⁴

12. Road Section Number (DOD) *25* ¹⁹

13. Bridge Description *WH NSTS* ²⁴

14. Defense Milepoint (0.01 ml) *2.62* ²⁶

15. Defense Section Length (miles) (0.1 mile) *8.10* ³⁰

16. Latitude: ¹⁸ Degrees *27.3* Minutes ³³

17. Longitude: ⁶⁶ Degrees *03.3* Minutes ³⁸

18. Physical Vulnerability *Steel Truss* ⁴⁴

19. By Pass, Detour Length (Nearest mile) *Structure over highway* ⁴⁵

20. Toll *Toll Free* ⁴⁷

21. Custodian *W.T.P.W.* ⁴⁹

22. Owner *State Highway Department* ⁴⁹

23. F.A.P. No. ⁶⁰ *Unknown*

CLASSIFICATION

24. Fed. Aid System *Interstate, Urban* ¹⁰

25. Administrative *State* ¹²

26. Functional *Interstate, Urban* ¹³

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 16 0800

28. Lanes on Str. 0 Under 0 18 0005

29. ADT - Inventory Route 100,000 (estimated) 23 000000

30. Year 1985 28 85

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) 24.59m 32 2450

33. Median None Open Closed 35 0

34. Skew none 36 00

35. Structure Flared Yes No 38 0

36. Traffic Safety Features NNNN 39 NNNN

37. Historical Significance 43 5

38. Navigation Control Yes No 46 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 000

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 0000

41. Structure, Open or Closed to Traffic open 54 A

42. Type Service Pedestrian over highway 55 31

43. Structure Type-Main Steel truss deck 57 309

44. Structure Type-Approach Spans None 6d 000

45. No. of Spans-Main 2 10 002

46. No. of Spans-Approaches None 13 0000

47. Total Horizontal Clearance (0.01 m) L = 23.59m R = 23.60m 17 236

48. Max. Span Length (0.01 m) 23.60m face to face 20 0236

49. Structure Length (0.01 m) 52.14m 24 000527

50. Sidewalk Widths (0.01 m) Left none Right none 50 000000

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.64m 30 0016

52. Deck Width (out-out) (0.01 m) 2.00m 40 0020

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) Unlimited 44 9999

54. Vertical Underclearance - Minimum (0.01 m) 5.11m 48 0517

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.27m R = 1.19m 52 012

56. Lateral Underclearance on Left (Inner) Sides - Minimum (0.01 m) L = 2.22m R = 2.19m 53 022

57. Wearing Surface steel 56 7

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION	Material	Condition Analysis	9 8 7 6 5 4 3 2 1											
58. Deck	Steel	Fair condition											59	<input type="checkbox"/>
59. Superstructure	Steel	Fair condition											60	<input type="checkbox"/>
60. Substructure	Steel	Generally Good Condition											61	<input type="checkbox"/>
61. Channel & Channel Protection	NA	NA											62	<input type="checkbox"/>
62. Culvert & Retaining Walls	NA	NA											63	<input type="checkbox"/>
63. Estimated Remaining Life		10 years											64	<input type="checkbox"/>
64. Operating Rating		Pedestrian											66	<input type="checkbox"/>
65. Approach Alignment		NA											69	<input type="checkbox"/>
66. Inventory Rating		Pedestrian											70	<input type="checkbox"/>

APPRAISAL

		Deficiencies	
67. Structural Condition	Light corrosion at steel plate recently. Fine crackings, small spallings and light corrosion at footings and columns respectively.	The structure was painted	10 <input type="checkbox"/>
68. Deck Geometry	NA		11 <input type="checkbox"/>
69. Underclearances-Vert. & Lateral	(5.11m) Criteria	Condition equal to present desirable	12 <input type="checkbox"/>
70. Safe Load Capacity			13 <input type="checkbox"/>
71. Waterway Adequacy			14 <input type="checkbox"/>
72. Approach Alignment	at applicable		15 <input type="checkbox"/>

PROPOSED IMPROVEMENTS

73. Year Needed	1987		16 <input type="checkbox"/>
Completed			
Described			
74. Type of Service	Pedestrian		18 <input type="checkbox"/>
75. Type of Work	Rehabilitation		19 <input type="checkbox"/>
76. Improvement Length (0.1 m)			22 <input type="checkbox"/>
77. Design Loading	NA		28 <input type="checkbox"/>

BRIDGE STUDIES AND EVALUATION OFFICE

9 8 7 6 5 4 3 2 1

78. Roadway Width (0.01m) *N.A.* *0 0 0 0
 79. Number of Lanes *N.A.* *0 0
 80. ADT *130,080* *1 3 0 0 8 0
 81. Year of Estimated ADT *1995* *9 5
 82. Year of Proposed Adjacent Roadway Improvements *N.A.* *0 0
 83. Prop. Adj. Rdwy Improvements-Type *N.A.* *0

COST OF IMPROVEMENTS

84. Total (dollars)\$ *\$3,000.00* *0 0 0 0 3
 Estimated Design Time (months)

SUMMARY OF IMPROVEMENT COSTS

85. Preliminary Engineering (Dollars) *0 0 0
 86. Demolition Cost (Dollars) *0 0 0
 87. Substructure (Dollars) *0 0 0 0 0
 88. Superstructure (Dollars) *0 0 0 0 0
 89. Blank *
 90. Date of Last Inspection *August 25, 1987* *0 8 2 5 8 7
 91. Rehabilitate Existing Structure (Dollars) *\$3,000.00* *0 0 3
 92. Detour and Traffic Maintenance (Dollars) *0 0 0
 93. Approaches (Dollars) *0 0 0
 Approach Embankment (Dollars)
 Approach Pavement (Dollars)
 Approach Guardrail (Dollars)
 *(Code to nearest thousand dollars)

REMARKS

The rehabilitation consists in repair the deficiencies described on item no. 67

BRIDGE STUDIES AND EVALUATION OFFICE
GENERAL EVALUATION

This bridge is basically in 3 structural condition

16 3

excellent	4	fair	3	very poor	0
good	3	poor	1		

The load carrying capacity is 0

17 0

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
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The recommended modifications will make it 0

18 0

adequate	2	minimally adequate	1	not required (don't print)	0
				if when previous is	2

The bridge should be replaced.

19 0

Yes (print)	1	Not required (don't print)	0
-------------	---	----------------------------	---

It requires 2024

20 2 24

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 2

23 2

routine maintenance	3
minor repairs and routine maintenance	2
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	1
emergency repairs to eliminate danger to the public and routine maintenance	0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1995) ADT, the travelled way is 0

For PRHA (1995) recommendations, the travelled way is 0

For present ADT, the shoulders are 0

For future (1995) ADT, the shoulders are 0

For PRHA (1995) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
Travelled way	0	0	0	0	0	0
Shoulders						

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print) 0					

The (B) approach alignment is (B)

31 0 0

(A) vertical 1 horizontal 0 horizontal and vertical 0

(B) satisfactory 3 minimally tolerable 2 poor and represents a danger to the public 1

not required (don't print) 0

Load Post for inventory rating shown in Item 64

33 0

Yes	1	No (don't print)	0
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Post for vertical underclearance shown in Items 54 & 69

34 0

Yes	1	No (don't print)	0
-----	---	------------------	---

For "Remarks" see bridge file.

PERSONNEL
Structural Inspection
Review of Field Data
Transfer of Data

By Recca Pyzore Date 7/28/07
30/07

Condition Analysis
Appraisal
IBRD MT/SC file
IBRD-370 file

By A. Ciccardi Date 8/25/07

BRIDGE NO. 1137
 ROAD NO. 26 KM. 4.07

APPRAISAL OF ITEM 88

BRIDGE STUDIES AND
 EVALUATION OFFICE

YEAR	ADT	Sec.	T W	TW	TW	TW + SH	TW + SH	TW + SH
				Adequate or Inadequate	Inadequate By		Adequate or Inadequate	Inadequate By
1985	N.A.	<input checked="" type="checkbox"/>	Actual \equiv	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT \equiv					
	N.A.							
1995	N.A.		Reqd. For ADT \equiv					
			Rec. PRHA \equiv					

✓ Adequate
 X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM-370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

1137 0026
 No. Bridge No. Road No.

BRIDGE INVENTORY

Km. No. 4.07

IDENTIFICATION

Code Positions

		25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1	
1. State	Commonwealth of Puerto Rico																										
2. Highway District	SAN JUAN																										
3. Municipality	SAN JUAN																										
4. City/Town	SAN JUAN																										
5. Inventory Route	R-1-1 0002-0																										
6. Features Intersected	PEDESTRIAN SIDEWALKWAY																										
7. Facility Carried by Structure ⁵⁸	RR-26																										
8. Structure No.	1137 1 of 1																										
9. Location ¹⁰	RAZONAMIENTO DE CARRETERA AVENUE																										
10. Inventory Route, Minimum Vertical Clearance (0.01 m)	5.11																										
11. Kilometer Point (0.01 km)																											
12. Road Section Number (DOD)	25																										
13. Bridge Description	DH NOTE																										
14. Defense Milepoint (0.01 ml)	2.62																										
15. Defense Section Length (miles) (0.1 mile)	0.1																										
16. Latitude: 18 Degrees 27.3 Minutes																											
17. Longitude: 66 Degrees 03.3 Minutes																											
18. Physical Vulnerability	Steel Truss																										
19. By Pass, Detour Length (Nearest mile)	STRUCTURE OVER HIGHWAY																										
20. Toll	TOLL FREE																										
21. Custodian	D.T.A.W.																										
22. Owner	State Highway Department																										
23. F.A.P. No. ⁵⁰	UNKNOWN																										

CLASSIFICATION

24. Fed. Aid System	Interstate, Urban																										
25. Administrative	State																										
26. Functional	INTERSTATE																										

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 15 6 8 0 0

28. Lanes on Str. 0 Under 5 19 0 0 0 5

29. ADT - Inventory Route 70,000 ESTIMATE 23 0 7 0 0 0

30. Year 1985 28 7 5

31. Design Load TEDESTRATION 31 7

32. Approach Roadway width including shoulders (0.01 m) 24.52 m 32 2 4 5 2

33. Median None Open Closed 35 0

34. Skew None 36 0 0

35. Structure Flared Yes No 38 0

36. Traffic Safety Features 39

37. Historical Significance 43 5

38. Navigation Control Yes No 46 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 0 0 0

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 0 0 0 0

41. Structure, Open or Closed to Traffic OPEN 54

42. Type Service RECREATION over highway 56 7

43. Structure Type-Main Steel Truss with 57 3 6 9

44. Structure Type-Approach Spans None 60 0 0 0

45. No. of Spans-Main 2 10 0 0 0

46. No. of Spans-Approaches None 13 0 0 0 0

47. Total Horizontal Clearance (0.01 m) L = 23.57 R = 23.60 17 2 3 2

48. Max. Span Length (0.01 m) 23.60 face to face 20 0 0 3 6

49. Structure Length (0.01 m) 57.14 m 24 0 0 0 5 2 7

50. Sidewalk Widths (0.01 m) Left Right 30 0 0 0 0 0 0

51. Bridge Roadway Width (curb-curb) (0.01 m) 16.8 38 0 0 7 6

52. Deck Width (out-out) (0.01 m) 2.00 40 0 0 2 0

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) UNLIMITED 44 0 0 0 0

54. Vertical Underclearance - Minimum (0.01 m) 5.11 48 0 5 1 1

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.11 R = 1.19 52 0 7 2

56. Lateral Underclearance on Left (Inner) Sides - Minimum (0.01 m) L = 2.22 R = 2.19 54 0 2 2

57. Wearing Surface Steel 58 7

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION

9 8 7 6 5 4 3 2 1

CONDITION	Material	Condition Analysis	
58. Deck	STEEL	GENERALLY Good Condition	59 <input checked="" type="checkbox"/>
59. Superstructure	STEEL	" " " "	60 <input checked="" type="checkbox"/>
60. Substructure	STEEL	" " " "	61 <input checked="" type="checkbox"/>
61. Channel & Channel Protection	NA	" " " "	62 <input checked="" type="checkbox"/>
62. Culvert & Retaining Walls	NA	" " " "	63 <input checked="" type="checkbox"/>
63. Estimated Remaining Life		15 YEARS	64 <input checked="" type="checkbox"/>
64. Operating Rating		PERMANENT	66 <input checked="" type="checkbox"/>
65. Approach Alignment		NA	69 <input checked="" type="checkbox"/>
66. Inventory Rating		PERMANENT	70 <input checked="" type="checkbox"/>

APPRAISAL

APPRAISAL	Deficiencies	
67. Structural Condition	MINOR MAINTENANCE IS REQUIRED. SAND BLASTING AND PAINTING.	10 <input checked="" type="checkbox"/>
68. Deck Geometry	N/A	11 <input checked="" type="checkbox"/>
69. Underclearances-Vert. & Lateral	Condition equal to present clearance criteria	12 <input checked="" type="checkbox"/>
70. Safe Load Capacity		13 <input checked="" type="checkbox"/>
71. Waterway Adequacy		14 <input checked="" type="checkbox"/>
72. Approach Alignment		15 <input checked="" type="checkbox"/>

PROPOSED IMPROVEMENTS

73. Year Needed	1986	16 <input checked="" type="checkbox"/>
Completed		
Described		
74. Type of Service	PERMANENT	18 <input checked="" type="checkbox"/>
75. Type of Work	REHABILITATION	19 <input checked="" type="checkbox"/>
76. Improvement Length (0.1 m)		22 <input checked="" type="checkbox"/>
77. Design Loading		28 <input checked="" type="checkbox"/>

BRIDGE STUDIES AND EVALUATION OFFICE

9 8 7 6 5 4 3 2 1

- 78. Roadway Width (0.01m)..... *NA* 10 0 1 0
- 79. Number of Lanes..... *NA* 0 0
- 80. ADT..... *130000* 7 0 0 0 0 0
- 81. Year of Estimated ADT..... *1995* 9 5
- 82. Year of Proposed Adjacent Roadway Improvements..... *NA* 0 0
- 83. Prop. Adj. Rdwy Improvements-Type..... *NA* 0

COST OF IMPROVEMENTS

- 84. Total (dollars)\$..... *10,000.00* 0 0 0 0 0 0
- Estimated Design Time (months).....

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars)..... 0 0 0
- 86. Demolition Cost (Dollars)..... 0 0 0
- 87. Substructure (Dollars)..... *NA* 0 0 0 0 0 0
- 88. Superstructure (Dollars)..... *NA* 0 0 0 0 0 0
- 89. Blank..... *Blank* 0
- 90. Date of Last Inspection..... 0 4 2 7 8 6 *04-28-86*
- 91. Rehabilitate Existing Structure (Dollars)..... *10,000* 0 0 0
- 92. Detour and Traffic Maintenance (Dollars)..... *NA* 0 0 0
- 93. Approaches (Dollars)..... *NA* 0 0 0
- Approach Embankment (Dollars).....
- Approach Pavement (Dollars).....
- Approach Guardrail (Dollars).....
- *(Code to nearest thousand dollars).....

REMARKS

REHABILITATION consist of MAINTENANCE, SAND BLASTING and painting

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BRIDGE NO. _____
 ROAD NO. _____ KM. _____

APPRAISAL OF ITEM 68

BRIDGE STUDIES AND
 EVALUATION OFFICE

YEAR	ADT	Sec.	T W	T W	T W	T W + S H	T W + S H	T W + S H
				Adequate of Inadequate	Inadequate By		Adequate of Inadequate	Inadequate By
1985	<i>na</i>	<input checked="" type="checkbox"/>	Actual ≡	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT ≡					
1995	<i>na</i>		Reqd. For ADT ≡					
			Rec. PRHA ≡					

✓ Adequate
 X Inadequate

RIDGE STUDIES AND EVALUATION OFFICE
GENERAL EVALUATION

This bridge is basically in 3 structural condition 16

excellent	4	fair	2	very poor	0
good	3	poor	1		

The load carrying capacity is 17

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
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The recommended modifications will make it 18

adequate	2	minimally adequate	1	not required (don't print)	0
				0 when previous is	2

The bridge should be replaced. 19

Yes (print)	1	Not required (don't print)	0
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It requires 20

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 23

routine maintenance 3

minor repairs and routine maintenance 2

urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance 1

emergency repairs to eliminate danger to the public and routine maintenance 0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1995) ADT, the travelled way is 0

For PRHA (1995) recommendations, the travelled way is 0 24

For present ADT, the shoulders are 0

For future (1995) ADT, the shoulders are 0

For PRHA (1995) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
	0	0	0	0	0	0
Travelled way			Shoulders			

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print)		0			

The (A) approach alignment is (B) 31

<u>(A)</u> vertical	1	horizontal	0	horizontal and vertical	2
<u>(B)</u> satisfactory	3	minimally tolerable	2	poor and represents a danger to the public	1
not required (don't print)		0			

(A) (B)

Load Post for inventory rating shown in Item 64 33

Yes	1	No (don't print)	0
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Post for vertical underclearance shown in Items 54 & 69 34

Yes	1	No (don't print)	0
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For "Remarks" see bridge file.

<p>PERSONNEL</p> <p>Structural Inspection _____ By _____ Date _____</p> <p>Review of Field Data _____</p> <p>Transfer of Data _____</p>	<p>Condition Analysis</p> <p>Appraisal _____ By _____ Date _____</p> <p>IBM MT/SC file _____</p> <p>IBM-370 file _____</p>
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BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 15 6 8 0 0

28. Lanes on Str. 0 Under 8 19 0 0 0 8

29. ADT - Inventory Route N/A 23 0 0 0 0 0 0

30. Year N/A 29 0 0

31. Design Load Pedestrian 31

32. Approach Roadway width including shoulders (0.01 m) 11.162 32 0 1 1 6

33. Median None Open Closed 35 0

34. Skew None 36 0 0

35. Structure Flared Yes No 38 0

36. Traffic Safety Features 39 N N N N 43 0 0 0 0

37. Blank 46 0

38. Navigation Control Yes No 47 0 0 0

39. Navigation Vertical Clearance (0.1 m) Yes No 50 0 0 0 0

40. Navigation Horizontal Clearance (0.1 m) Yes No 54 0

41. Structure, Open or Closed to Traffic Open 55 3 1

42. Type Service Pedestrian over Highway 57 3 0 0

43. Structure Type-Main Steel Truss Deck 60 0 0 0

44. Structure Type-Approach Spans None 10 0 0 0

45. No. of Spans-Main Two 13 0 0 0 0

46. No. of Spans-Approaches None 17 0 1 0

47. Total Horizontal Clearance (0.01 m) L = 1.62 R = 20 0 2 1 3

48. Max. Span Length (0.01 m) 24.37 24 0 0 0 5 2 1

49. Structure Length (0.01 m) 52.14 30 0 0 0 0 0 0

50. Sidewalk Widths (0.01 m) Left Right 36 0 0 1 0

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.62 40 0 0 1 0

52. Deck Width (out-out) (0.01 m) 1.62 44 0 0 0 0

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) Unlimited 48 0 5 1 0

54. Vertical Underclearance - Minimum (0.01 m) 5.19 52 0 1 2

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.27 R = 1.19 55 0 1 3

56. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.30 R = 1.45 58 0

57. Wearing Surface Steel Plate

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION	Material	Condition Analysis	9	8	7	6	5	4	3	2	1
58. Deck	steel	Good Cond. N/A									8
59. Superstructure	steel	Minor T.P.P.F.									7
60. Substructure	steel	Minor T.P.P.F.									7
61. Channel & Channel Protection		N.A.									N
62. Culvert & Retaining Walls		N.A.									N
63. Estimated Remaining Life		1.5 years									5
64. Operating Rating		Pedestrian Walkway	8	0	0						0
65. Approach Alignment		N.A.									N
66. Inventory Rating		Pedestrian Walkway	8	0	0						0

APPRAISAL

	Deficiencies	
67. Structural Condition	Moderate corrosion at whole bridge	10 <input checked="" type="checkbox"/>
68. Deck Geometry	N.A.	11 <input checked="" type="checkbox"/>
69. Underclearances-Vert. & Lateral	(5.19m) Superior to present desirable criteria	12 <input type="checkbox"/>
70. Safe Load Capacity	N.A.	13 <input checked="" type="checkbox"/>
71. Waterway Adequacy	N.A.	14 <input checked="" type="checkbox"/>
72. Approach Alignment	N.A.	15 <input checked="" type="checkbox"/>

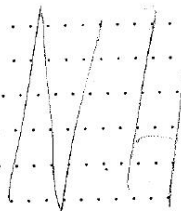
PROPOSED IMPROVEMENTS

73. Year Needed	1990	16 <input checked="" type="checkbox"/>
Completed		
Describe		
74. Type of Service	Pedestrian over highway	18 <input checked="" type="checkbox"/>
75. Type of Work	Rehabilitation	19 <input checked="" type="checkbox"/> <input checked="" type="checkbox"/> <input checked="" type="checkbox"/>
76. Improvement Length (0.1 m)	N.A.	22 <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/> <input type="checkbox"/>
77. Design Loading		28 <input type="checkbox"/>

BRIDGE STUDIES AND EVALUATION OFFICE

9 8 7 6 5 4 3 2 1

- 78. Roadway Width (0.01 m) 29 0 0 0 0
- 79. Number of Lanes 33 0 0
- 80. ADT 35 0 0 0 0 0 0 0
- 81. Year of Estimated ADT 41 0 0
- 82. Year of Proposed Adjacent Roadway Improvements 43 0 0
- 83. Prop. Adj. Rdwy Improvements-Type 45



COST OF IMPROVEMENTS

- 84. Total (dollars) \$ 3,000 46 0 0 0 0 0 *
- Estimated Design Time (months)

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars) 51 0 0 0 *
- 86. Demolition Cost (Dollars) 54 0 0 0 *
- 87. Substructure (Dollars) 57 0 0 0 0 *
- 88. Superstructure (Dollars) 62 0 0 0 0 *
- 89. Blank 67 *
- 90. Rehabilitate Existing Structure (Dollars) 3,000 68 0 0 0 *
- 91. Detour and Traffic Maintenance (Dollars) 71 0 0 0 *
- 92. Approaches (Dollars) 74 0 0 0 *
- Approach Embankment (Dollars)
- Approach Pavement (Dollars)
- Approach Guardrail (Dollars)
- (* Code to nearest thousand dollars)
- 93. Date of Last Inspection July 16, 1975 10 0 0 0 0 0 *

REMARKS

1) Rehabilitation (Item 90) should include; a) Removal of rust from structural steel by means of wire brush or any other acceptable method and the protection of it with paint; b) The replacement of any structural element, if necessary.

BRIDGE STUDIES AND EVALUATION OF
GENERAL EVALUATION

This bridge is basically in 3 structural condition

16 3

excellent	4	fair	2	very poor	0
good	3	poor	1		

The load carrying capacity is _____

17 0

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
----------	---	--------------------	---	------------	---	----------------------------	---

The recommended modifications will make it _____

18 0

adequate	2	minimally adequate	1	not required (don't print)	0
				0 when previous is	2

The bridge should be replaced.

19 0

Yes (print)	1	Not required (don't print)	0
-------------	---	----------------------------	---

It requires 1; 12

20 1 12

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 2

23 2

routine maintenance	3
minor repairs and routine maintenance	2
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	1
emergency repairs to eliminate danger to the public and routine maintenance	0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1990) ADT, the travelled way is 0

For PRHA (1990) recommendations, the travelled way is 0

For present ADT, the shoulders are 0

For future (1990) ADT, the shoulders are 0

For PRHA (1990) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>
Travelled way			Shoulders			

satisfactory 3 unsatisfactory 2 cannot be evaluated because of lack of information 1

not required (don't print) 0

The (A) approach alignment is (B)

31 0 0

(A) vertical 1 horizontal 0 horizontal and vertical 2

(A) (B)

(B) satisfactory 3 minimally tolerable 2 poor and represents a danger to the public 1

not required (don't print) 0

Load Post for inventory rating shown in Item 66

33 0

Yes	1	No (don't print)	0
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Post for vertical underclearance shown in Items 54 & 69

34 0

Yes	1	No (don't print)	0
-----	---	------------------	---

For "Remarks" see bridge file.

PERSONNEL

Structural Inspection	By <u>L. Pérez</u>	Date <u>Jul 16/75</u>
Review of Field Data	By <u>J. Capero</u>	Date <u>Jul 16/75</u>
Transfer of Data	By <u>J. Domio</u>	Date <u>Jan 19/76</u>

Condition Analysis	By <u>N.Z.</u>	Date <u>1-15-76</u>
Appraisal	By <u>G. Velasco</u>	Date <u>1-26-76</u>
IBM MT/SC file	By <u>R. Pando</u>	Date <u>3-30-76</u>
IBM-370 file		

BRIDGE NO. _____

APPRAISAL OF ITEM 68

ROAD NO. _____ KM. _____

**BRIDGE STUDIES AND
EVALUATION OFFICE**

YEAR	ADT	Sec.	T W	T W Adequate or Inadequate	T W Inadequate By	TW + SH	TW+SH Adequate or Inadequate	TW + SH Inadequate By
1970		X	Actual ≡	X	X		X	X
			For ADT ≡					
1990			Reqd. For ADT ≡					
			Rec. PRHA ≡					

✓ Adequate
X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM - 370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

Card No.

Bridge No.

Road No.

ROUTE UNDER STRUCTURE

Km. No. 4.07

Code Positions

IDENTIFICATION

25|24|23|22|21|20|19|18|17|16|15|14|13|12|11|10|9|8|7|6|5|4|3|2|1

- 5. Inventory Route . . . PR 26 7.5.3.5.1.5.0026.0.0. ¹⁰
- 6. Features Intersected . . . Pedestrian Walkway Bridge 1137.
 ¹⁹
- 8. Structure No. 1137. one of one. ⁴⁴
- 10. Inventory Route, Minimum Vertical Clearance (0.01 m) 5.19. ⁵⁰
- 11. Kilometer Point (0.01 km) 4.07 ⁵⁴
- 12. Road Section Number (DOD) 25 ⁵⁹
- 13. Bridge Description NDA. DRH. NSTS.P. ⁶⁴
- 14. Defense Milepoint (0.01 m) 2.62 ⁶⁸
- 15. Defense Section Length (miles) (0.1 mile) 8.1 ⁷⁰
- 19. By Pass, Detour Length (Nearest mile) Marginal Road ⁷³

CLASSIFICATION

- 24. Fed. Aid System Fed. aid. urban ⁷⁵
- 25. Administrative STATE ⁷⁷
- 26. Functional Urban. Principal Arterial ⁷⁸

STRUCTURE DATA

- 29. ADT - Inventory Route 36,550. ¹⁰
- 30. Year 1970 ¹⁶
- 47. Total Horizontal Clearance (0.01 m) L = 10.61 R = 7.30 ¹⁸

B7 - 1137K

9 8 7 6 5 4 3 2 1

STRUCTURE DATA

27. Year Built 1968 15 6800

28. Lanes on Str. 0 Under 8 19 0008

29. ADT - Inventory Route. NA 23 000000

30. Year NA 29 00

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) NA 32 5615

33. Median None Open Closed 35

34. Skew No 36 00

35. Structure Flared Yes No 38 0

36. Hydraulic Structure Yes No 39 0

37. Report Available Yes No 40 0

38. Navigation Control NA Yes No 41 0

39. Navigation Vertical Clearance (0.1 m) NA Yes No 42 000

40. Navigation Horizontal Clearance (0.1 m) NA Yes No 45 0000

41. Relief Structures None 49 60

42. Type Service Pedestrian, area highway 51 31

43. Structure Type—Main Steel deck Truss 53 309

44. Structure Type—Approach Spans None 56 000

45. No. of Spans—Main 2 10 002

46. No. of Spans—Approaches None 13 0000

47. Total Horizontal Clearance (0.01 m) L = 1.62 R = 17 016

48. Max. Span Length (0.01 m) 24.37 20 0244

49. Structure Length (0.01 m) 48.74 24 000487

50. Sidewalk Widths (0.01 m) Left 0.00 Right 0.00 30 000000

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.62 36 0016

52. Deck Width (out-out) (0.01 m) 1.62 40 0016

53. Vertical Clearance over Bridge Roadway—Minimum (0.01 m) Unlimited 44 9999

54. Vertical Underclearance - Minimum (0.01 m) 5.19 48 0519

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.27 R = 1.19 52 013

56. Lateral Underclearance on Left (Median) Sides - Minimum (0.01 m) L = 1.30 R = 1.45 55 013

57. Wearing Surface steel plate 58 7



CONDITION

9 8 7 6 5 4 3 2 1

	Material	Condition Analysis	
58. Deck	Steel	Minor r.r.m.f.	59 <input type="checkbox"/> 7
59. Superstructure	Steel	Minor r.r.m.f.	60 <input type="checkbox"/> 7
60. Substructure	Steel	G.C.N.R.N.	61 <input type="checkbox"/> 8
61. Channel & Channel Protection	NA		62 <input type="checkbox"/> W
62. Culvert & Retaining Walls	NA		63 <input type="checkbox"/> W
63. Estimated Remaining Life	30 years		64 <input type="checkbox"/> 2 0
64. Operating Rating	PEDESTRIAN LOADING		66 <input type="checkbox"/> 8 0 0
65. Approach Alignment	NA		69 <input type="checkbox"/> W
66. Inventory Rating	PEDESTRIAN LOADING		70 <input type="checkbox"/> 8 0 0

APPRAISAL

Deficiencies

67. Structural Condition	Needs painting Some joint bolt safety pins missing	10 <input type="checkbox"/> 7
68. Deck Geometry	NA	11 <input type="checkbox"/> W
69. Underclearances—Vert. & Lateral	V = 5.19m; Lateral underclearance width equals recommended (P.R.H.A.) (1.9.9.0) section width	12 <input type="checkbox"/> 5
70. Safe Load Capacity	NA	13 <input type="checkbox"/> 6
71. Waterway Adequacy	NA	14 <input type="checkbox"/> W
72. Approach Alignment	NA	15 <input type="checkbox"/> W

PROPOSED IMPROVEMENTS

73. Year Needed	1973	16 <input type="checkbox"/> 7 3
Completed		
Describe		
74. Type of Service	Pedestrian	18 <input type="checkbox"/> 3
75. Type of Work	Rehabilitation	19 <input type="checkbox"/> 3 7 1
76. Improvement Length (0.1 m)	52.14	22 <input type="checkbox"/> 0 0 0 5 2 2
77. Design Loading	V. Pedestrian	28 <input type="checkbox"/> 7



9 8 7 6 5 4 3 2 1

- 78. Roadway Width (0.01 m) 1.62 29 0 0 1 6
- 79. Number of Lanes N.A. 33 0 0
- 80. ADT N.A. 35 0 0 0 0 0 0
- 81. Year of Estimated ADT N.A. 41 0 0
- 82. Year of Proposed Adjacent Roadway Improvements N.A. 43 0 0
- 83. Prop. Adj. Rdwy Improvements—Type N.A. 45 0

COST OF IMPROVEMENTS

- 84. Total (dollars) \$ 4,000 46 0 0 0 0 4 *

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars) 51 [] [] [] *
- 86. Demolition Cost (Dollars) 54 [] [] [] *
- 87. Substructure (Dollars) 57 [] [] [] [] [] *
- 88. Superstructure (Dollars) 62 [] [] [] [] [] *
- 89. Priority Letter 67 [] *
- 90. Rehabilitate Existing Structure (Dollars) 68 [] [] [] *
- 91. Detour and Traffic Maintenance (Dollars) 71 [] [] [] *
- 92. Approaches (Dollars) 74 [] [] [] *

Approach Embankment (Dollars) _____

Approach Pavement (Dollars) _____

Approach Guardrail (Dollars) _____

(*Code to nearest thousand dollars)

Remarks

Rehabilitation should include replacing missing bolts & safety pins and also painting.

PERSONNEL	By	Date		By	Date
Structural Inspection	S.M. Rodgers	11/13/72	Condition Analysis	K. Shah	3/29/73
Topographic Survey			Appraisal	"	"
Review of Field Data	K. Shah	3/27/73	IBM MT/SC file		
Transfer of Data	K. Shah	"	IBM-370 file		

REMARKS

B. 1137

~~Travelled way narrow for present (1970) ADT by PRHA recommended section by~~
~~Roadway (including shoulders) narrow for present (1970) ADT by~~
~~Narrow for future (1990) ADT by and for future PRHA recommended section by~~

GENERAL EVALUATION

This bridge is basically in ~~excellent~~ good structural condition.
~~fair~~
~~poor~~
~~very poor~~

~~The load-carrying capacity is minimally adequate inadequate~~

~~The recommended modifications will make it minimally adequate.~~

~~The bridge should be replaced.~~

It requires ~~routine inspection (every 2 years).~~
~~frequent inspection (every months) to determine the cause of~~
~~and remedies for existing defects.~~

~~frequent inspection (every months) to monitor abnormal~~
~~and/or suspected deficiencies.~~

It requires ~~routine maintenance.~~
~~minor repairs and routine maintenance.~~
~~emergency repairs to eliminate danger to the public.~~
~~urgent repairs to prevent further costly deterioration and/or~~
~~the development of a dangerous condition.~~
~~Non-urgent programmed repairs.~~

~~The deck geometry is unsatisfactory. The travelled way is unsatisfactory for present requirements. The shoulders are unsatisfactory for present requirements.~~

~~The travelled way is unsatisfactory for future (1990) () requirements.~~

~~The shoulders are unsatisfactory for future (1990) () requirements.~~

~~The approach alignment is satisfactory minimally adequate.~~
~~poor and represents a danger to the public.~~



ROUTE UNDER STRUCTURE

Km. No. 4.07

Code Positions

IDENTIFICATION	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1							
5. Inventory Route	PR 26 . . . 2.6 3.6 1.6 0.0 0.2 6 6 0 . 10.																2	3	1	0	0	0	2	6	0							
6. Features Intersected	Pedestrian walkway . . . C. Palacia																19	P	E	D	W	A	L	K	C.	P	A	L	A	C	I	O
8. Structure No.	1137 . . . 1.0P1																44	0	1	1	3	7	1									
10. Inventory Route, Minimum Vertical Clearance (0.01 m)	5.19																50	0	5	1	9											
11. Kilometer Point (0.01 km)	4.07																54	0	0	4	0	7										
12. Road Section Number (DOD)	62																59	0	0	6	2	5										
13. Bridge Description	N.D.H.O.D.H. . . . N.S.T.S. . . . P																69	6	6													
14. Defense Milepoint (0.01 m)	1.09																66	0	1	0	9											
15. Defense Section Length (miles) (0.1 mile)	1.7																70	0	1	7												
19. By Pass, Detour Length (Nearest mile)	Marginal road																73	0	1													

CLASSIFICATION

24. Fed. Aid System	Other Federal Aid, primary urban																75	0	4
25. Administrative	State																77	1	
26. Functional	Urban principal arterial																78	4	3

Card 2

STRUCTURE DATA

29. ADT - Inventory Route	361.550																10	0	3	6	5	5	0
30. Year	1970																16	7	0				
47. Total Horizontal Clearance (0.01 m)	L 10.61 . . . R 7.30																18	0	7	3			

Appendix E Inspection by PRHTA of PB 1307

E.1 May 15, 2017

INSPECTION REPORT SUMMARY & QC SHEET

BRIDGE: 1307

TEAM LEADER: Angel Lopez

INSP. DATE: 15 mayo 2017

1. Inspection Type and Dates:


NBI	Type	Performed? (Yes / No / NA)	Freq (MONTHS)	Previous Insp. DATE (MONTH/YEAR)	Next Insp. DATE (MONTH/YEAR)
ITEM 90	Routine Inspection	<u>Yes</u>	<u>—</u>	<u>Jan 2014</u>	<u>—</u>
ITEM 93 A	FC Inspection				
ITEM 93 B	Underwater Insp.				
ITEM 93 C	Other:				


2. NBI Condition Rating Summary:

	Item 58	Item 59	Item 60	Item 61	Item 62	Item 113
Previous Inspection	<u>5</u>	<u>5</u>	<u>5</u>	<u>—</u>	<u>—</u>	<u>—</u>
Current Inspection	<u>5</u>	<u>5</u>	<u>5</u>	<u>—</u>	<u>—</u>	<u>—</u>

Other Checks: (Y, N, NA) Review Comments:

- Scour Critical (items 113 & 60)
- AASHTO Core's & NBI CD consistent
- Smart Flags (scour, steel plate, fire damage, etc)
- Channel Profile/Clearance Table
- FC & Underwater Members Tables
- Asphalt Overlay Thickness
- Drawings
- Photos
- Critical Finding
- Inspector & Team Leader Signature

Reviewer: 

Safety Eng.: 

Scan: MAYO - 31 - 2017 (Karl)

MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA

Team Leader: Angel T. López

Bridge Inspector: Angel T. López

Bridge Evaluator: Manuel Coll

Assistants: José R. Vázquez _ _ _ _

Driver: José A. Ortiz

Inspection date: _ May, 15, 2017

Weather Conditions: Sunny

Amount of Time on Inspection: 1.5 Hours

Equipment: X Bus or Van _ Underwater _ Ladders _ Snooper X Camera _ Boat

_ Other:

Bridge Number: 1307

Road on Structure: N/A Number or Name: _ Km. _

Road Under Structure: State Highway Number or Name: 26 Km. 4.6

Ident. Plaque: No Num. _

36-Traffic Safety Features:

Bridge railings: not applicable or safety not required


Transitions: not applicable or safety not required


Approach Guardrail: not applicable or safety not required

Approach Guardrail Ends: not applicable or safety not required

COMMENTS AND/OR RECOMMENDATIONS:

(50.25mts largo)


Inspection by: Angel T. López
Bridge Inspector


Revised and Approved by: Manuel Coll
Bridge Evaluator

30 may 2017

BR-1307

1

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

58. DECK

58.1 Wearing Surface: Material: Steel Condition: 5 Thickness cm.
Deterioration: 50 to 75 % Drainage: Adequate Ponding: No Safety: Yes

58.2 Slab or Plate: Material: Steel Condition: 5

Cracking: None

Spalling: Top: No Bottom: No

Scaling: Top: No Bottom: No

Efflorescence: No Exudation: No Rust Stains: Yes Corrosion: Severe

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	5		Fair	--	Fair	Severe	Poor	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: _

Type: Size: Safety: _

Type: Size: Safety: _

58.11 Joints: Condition: N/A

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: None Condition: N Functioning: N/A

Comments:

58.1- Superficie de acero con corrosión severa, pérdidas leves de secciones y leves perforaciones en los laterales de la superficie de las tolas. Varias tolas están sueltas de sus puntos de soldadura, grandes secciones fueron reparadas aunque hay secciones en pobres condiciones, varios huecos pequeños por corrosión y secciones de las de tolas soldadas que dejan un ligero hueco entre ellas.

58.8- Corrosión severa con mallas de protección, varias de ellas están sueltas de sus puntos de apoyo. En el área de las escaleras hay varios verticales que están sueltos de sus soldaduras.

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed Out of Plumb: No Paint: None
Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.
Condition: 5 Cracking: No Spalling: None Scaling: None
Crushing: No

59.3 Steel beams: N/A Type: N/A
Condition: N Corrosion: None Paint: None

59.4 Concrete: N/A Type: N/A
Condition: N Cracking: No Spalling: None Scaling: None

59.5 Truss: Bailey Pedestrian Paint: Poor Corrosion: Severe Members: Stringers Condition: 5

59.6 Drainage: Type: Condition: N Functioning: No

59.7 Hinges: Condition: N Functioning: N/A Movement: N/A

59.8 Deflection: Normal

59.9 Vibrations: Minimal

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.2	Bracing	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.3	Columns	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.4	Footing	Concrete	6	F	Small	L	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.5- Corrosión moderada y severa en todos los miembros inferiores de la estructura. Hay perforaciones con pérdida de secciones. Las escaleras tienen corrosión severa y perforaciones con pérdidas de secciones, Hay "railings" que tienen corrosión severa en los anclajes ó bases, tienen movimientos laterales.

**BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

61. CHANNEL AND PROTECTION Condition: N

61.1 Channel Scour: N/A

61.2 Embankment Erosion: N/A

		Type	Material	Condition Rating	Functioning
61.3	Protective Device		N/A	N	--
61.4	Fender System		N/A	N	--
61.5	Rip Rap		N/A	N	--
61.6	Spur Dikes, Jetties		N/A	N	--

61.7 Obstruction: N/A

61.8 Channel Change: N/A

61.9 Adequate Waterway: N/A

61.10 Surrounding: Flooding: N/A if yes explain:

Erosion: N/A if yes explain:

61.11 Location of piers and/or abutment detrimental: N/A if yes explain:

61.12 Other features that may affect structure:

Comments:

62. CULVERT AND RETAINING WALL

	Material	Condition Rating	Functioning	Cracking	Spalling	Scaling	Corrosion	Settlement	Alignment	Scour
62.1 Barrel	N/A	N	--	--	--	--	--	--	--	--
62.2 Head Walls	N/A	N	--	--	--	--	--	--	--	--
62.3 Cut-off wall	N/A	N	--	--	--	--	--	--	--	--
62.4 Retaining wall	N/A	N	--	--	--	--	--	--	--	--

Comments:

BRIDGE INVENTORY MANAGEMENT OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: Small Scaling: None Uneven: No
Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No
Excessive deformation: No
Safety: Hazardous: No
Drainage: Inadequate: No
Movement: Pavement-approach slab: No
Embankment: Condition: N Functioning: No Erosion: No
Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: _ May, 15, 2017

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:

Underwater:

Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Steel Plate (includes orthotropic)

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Steel

Type Membrane: None

Deck protection: None

111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

AUXILIARY ITEMS

Signs: Type: Height Limits 18'-6" Material: Alum. Condition: 7

Type: N/A

BR-1307

5



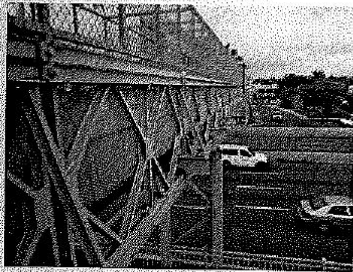
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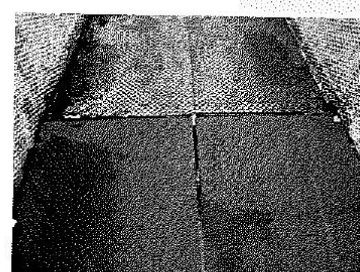
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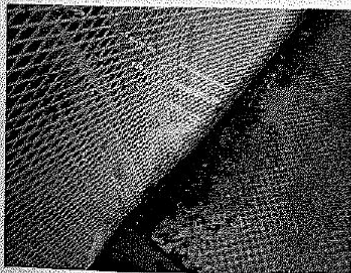
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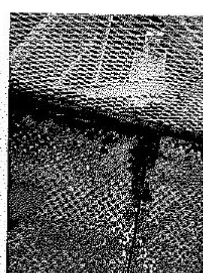
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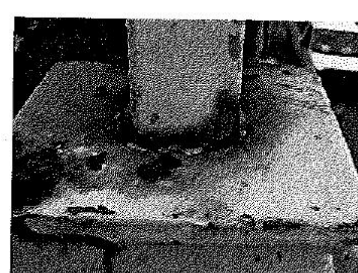
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1307-may-15-2017-030.jpg

E.2 January 24, 2014

INSPECTION REPORT SUMMARY & QC SHEET

BRIDGE: 1307 (Pedestrian)*

TEAM LEADER: Eng. Arturo Cáceres Febus

INSP. DATE: 1/24/14

1. Inspection Type and Dates:

NBI	Type	Performed? (Yes / No / NA)	Freq (MONTHS)	Previous Insp. DATE (MONTH/YEAR)	Next Insp. DATE (MONTH/YEAR)
ITEM 90	Routine Inspection	Yes	24	6/30/2008	1/24/14
ITEM 93 A	FC Inspection	N	—	—	—
ITEM 93 B	Underwater Insp.	N	—	—	—
ITEM 93 C	Other:	N	—	—	—

2. NBI Condition Rating Summary:

	Item 58	Item 59	Item 60	Item 61	Item 62	Item 113
Previous Inspection	5	5	5	N	N	Unknown
Current Inspection	5	5	5	N	N	N

Other Checks: (Y, N, NA)

Review Comments:

- Scour Critical (items 113 & 60)
- AASHTO Core's & NBI CD consistent
- Smart Flags (scour, steel plate, fire damage, etc)
- Channel Profile/Clearance Table
- FC & Underwater Members Tables
- Asphalt Overlay Thickness
- Drawings
- Photos
- Critical Finding
- Inspector & Team Leader Signature

Reviewer: [Signature]

Safety Eng.: [Signature]

Bridge Inspection Report

Bridge Key: 013071 Agency ID: 013071 Sufficiency Rating: -1.0

IDENTIFICATION

State 1: 72 Puerto Rico Struc Num 8: 013071
 Facility Carried 7: PR 26 Location 9: BALDORIOY DE CASTRO AVE.
 Rte.(On/Under)5A: One Route Under Rte. Signing Prefix 5B: 3 State Hwy
 Level of Service 5C: 1 Mainline Rte. Number 5D: 00026
 Directional Suffix 5E: 0 N/A (NBI) % Responsibility: NA
 SHD District 2: SAN JUAN County Code 3: SAN JUAN
 Place Code 4: SAN JUAN ZONA URBANA Kilometer Post 11: 04.5 km
 Feature Intersected 6: PEDESTRIAN WALKWAY
 Latitude 16: 18d 27' 18" Longitude 17: 066d 03' 06"
 Border Bridge Code 98: Not Applicable (P)
 Border Bridge Number 99: Unknown

INSPECTION

Frequency 91: 24 months Inspection Date 90: 1/24/2014 Next Inspection: 01/24/2016
 FC Frequency 92A: NA FC Inspection Date 93A: 1/24/2014 Next FC Inspection: NA
 UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 24 months Element Inspection Date: 01/01/1901 Next Elem. Insp. Due: 01/24/2016

CLASSIFICATION

Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: No || bridge exists
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Not Applicable (P)
 Highway System 104: 0 Not on NHS NBIS Length 112: Long Enough
 Toll Facility 20: 3 On free road Functional Class 25: 12 Urban Fwy/Expwy
 Historical Significance 37: 5 Not eligible for NRHP
 Owner 22: 01 State Highway Agency
 Custodian 21: 01 State Highway Agency

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: -1 Number of Spans Main Unit 45: 2
 Main Span Material/Design 43A/B:
 3 Steel 09
 Deck Type 107: 5 Steel Plate
 Wearing Surface 105A: 0 None
 Membrane 108B: 0 None
 Deck Protection 108C: None

CONDITION

Deck 58: 5 Fair Super 59: 5 Fair Sub 60: 5 Fair
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

LOAD RATING AND POSTING

Inventory Rating Method 65: Unknown (NBI) Operating Rating Method 63: Unknown (NBI)
 Inventory Rating 66: MS-6 Operating Rating 64: MS-0.6
 Design Load 31: 7 Pedestrian Posting 70: Unknown (NBI)
 Posting status 41: A Open, no restriction

AGE AND SERVICE

Year Built 27: 1968 Year Reconstructed 106: Unknown
 Type of Service on 42A: 3 Pedestrian-bicycle
 Type of Service under 42B: 1 Highway
 Lanes on 28A: Unknown Lanes Under 28B: 8 Detour Length 19: 0.0 km
 ADT 29: 65,300 Truck ADT 109: 5 % Year of ADT 30: 2000

APPRAISAL

Bridge Rail 36A: N N/A or not required Approach Rail 36C: N N/A or not required
 Transition 36B: N N/A or not required Approach Rail Ends 36D: N N/A or not required
 Str. Evaluation 67: Unknown (NBI) Deck Geometry 68: Unknown (NBI)
 Underclearance, Vertical and Horizontal 69: Unknown (NBI)
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: 6 Equal Min Criteria
 Scour Critical 113: N Not Over Waterway

GEOMETRIC DATA

Length Max Span 48: 21.70 m Structure Length 49: 43.01 m
 Curb/Sdwik Width L 50A: 0.00 m Curb/Sidewalk Width R 50B: 0.00 m
 Width Curb to Curb 51: 1.62 m Width Out to Out 52: 2.82 m
 Approach Roadway Width 32: 29.30 m Median 33: Unknown (NBI) (w/ shoulders)
 Deck Area: 121.29 m²
 Skew 34: 0.00 ° Structure Flared 35: 0 No flare
 Minimum Vertical Clearance Over Bridge 53: 99.99 m
 Minimum Vertical Underclearance Reference 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 07.16 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 01.70 m
 Minimum Lateral Underclearance L 56: 01.50 m

PROPOSED IMPROVEMENTS

Bridge Cost 94: NA Type of Work 75: Unknown (P)
 Roadway Cost 95: Unknown Length of Improvement 76:
 Total Cost 96: Unknown Future ADT 114: 92,703
 Year of Cost Estimate 97: Unknown Year of Future ADT 115: 2020

NAVIGATION DATA

Navigation Control 38: N NA-no waterway
 Vertical Clearance 39: 0.00 m Horizontal Clearance 40: 0.00 m
 Pier Protection 111: 1 Not Required Lift Bridge Vertical Clearance 116: 0.00 m

ELEMENT CONDITION STATE DATA

BRIDGE NOTES

CULVERT

Bridge Inspection Report

PAST INSPECTION

Inspection Date: 01/24/2014

Type: 1 Regular NBI

Inspector: -1

Pontis User Key: Pontis - Pontis Poi

Scope:

NBI:

Other:

Element:

Underwater:

Fracture Critical:

INSPECTION NOTES

SEVERE CORROSION, SECTION LOSS AND LIGHT HOLES AT LATERALS OF STEEL PLATE. SOME STEEL PLATES ARE LOOSE FROM WELDING SUPPORTS. SEVERE CORROSION AT WIRE MESH AND LOOSE FROM WELDING SUPPORTS. LOOSE VERTICALS, SEVERE COOROSION, HOLES AND SECTION LOSS AT APPROACHES LADDERS.

INSPECTOR WORK CANDIDATES

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

Team Leader: Arturo Cáceres

Bridge Inspector: Angel T. López

Bridge Evaluator: Arturo Cáceres

Assistants: Juan C. Otero José R. Vázquez _ _ _

Driver: Gabriel Díaz

Inspection date: _ Jan, 24, 2014

Weather Conditions: Sunny

Amount of Time on Inspection: 1.5 Hours

Equipment: X Bus or Van _ Underwater _ Ladders _ Snooper X Camera _ Boat
 _ Other:

Bridge Number: 1307

Road on Structure: N/A Number or Name: _ Km. _

Road Under Structure: State Highway Number or Name: 26 Km. 4.5

Ident. Plaque: No Num. _

Traffic Safety Features:

 Bridge railings: not applicable or safety not required

 Transitions: not applicable or safety not required

 Approach Guardrail: not applicable or safety not required

 Approach Guardrail Ends: not applicable or safety not required

COMMENTS AND/OR RECOMMENDATIONS:



Inspection by: Angel T. López

Bridge Inspector



Revised and Approved by: Arturo Cáceres

Team Leader

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

58. DECK

58.1 Wearing Surface: Material: Steel Condition: 5 Thickness cm.
 Deterioration: 50 to 75 % Drainage: Adequate Ponding: No Safety: Yes
 58.2 Slab or Plate: Material: Steel Condition: 5
 Cracking: None
 Spalling: Top: No Bottom: No
 Scaling: Top: No Bottom: No
 Efflorescence: No Exudation: No Rust Stains: Yes Corrosion: Severe
 58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	5		Fair	--	Fair	Severe	Poor	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: _

Type: Size: Safety: _

Type: Size: Safety: _

58.11 Joints: Condition: N/A

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: None Condition: N Functioning: N/A

Comments:

58.1- Superficie de acero con corrosión severa, pérdidas leves de secciones y leves perforaciones en los laterales de la superficie de las tolas. Varias tolas están sueltas de sus puntos de soldadura.

58.8- Corrosión severa con mallas de protección, varias están sueltas de sus puntos de apollo. En el área de las escaleras hay varios verticales que están sueltos de sus soldaduras.

**BRIDGE ENGINEERING OFFICE
INFRASTRUCTURE DIRECTORY
PRHTA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed Out of Plumb: No Paint: None
Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.
Condition: 5 Cracking: No Spalling: None Scaling: None
Crushing: No

59.3 Steel beams: N/A Type: N/A
Condition: N Corrosion: None Paint: None

59.4 Concrete: N/A Type: N/A
Condition: N Cracking: No Spalling: None Scaling: None

59.5 Truss: Bailey Pedestrian Paint: Poor Corrosion: Severe Members: Stringers Condition: 5

59.6 Drainage: Type: Condition: N Functioning: No

59.7 Hinges: Condition: N Functioning: N/A Movement: N/A

59.8 Deflection: Normal

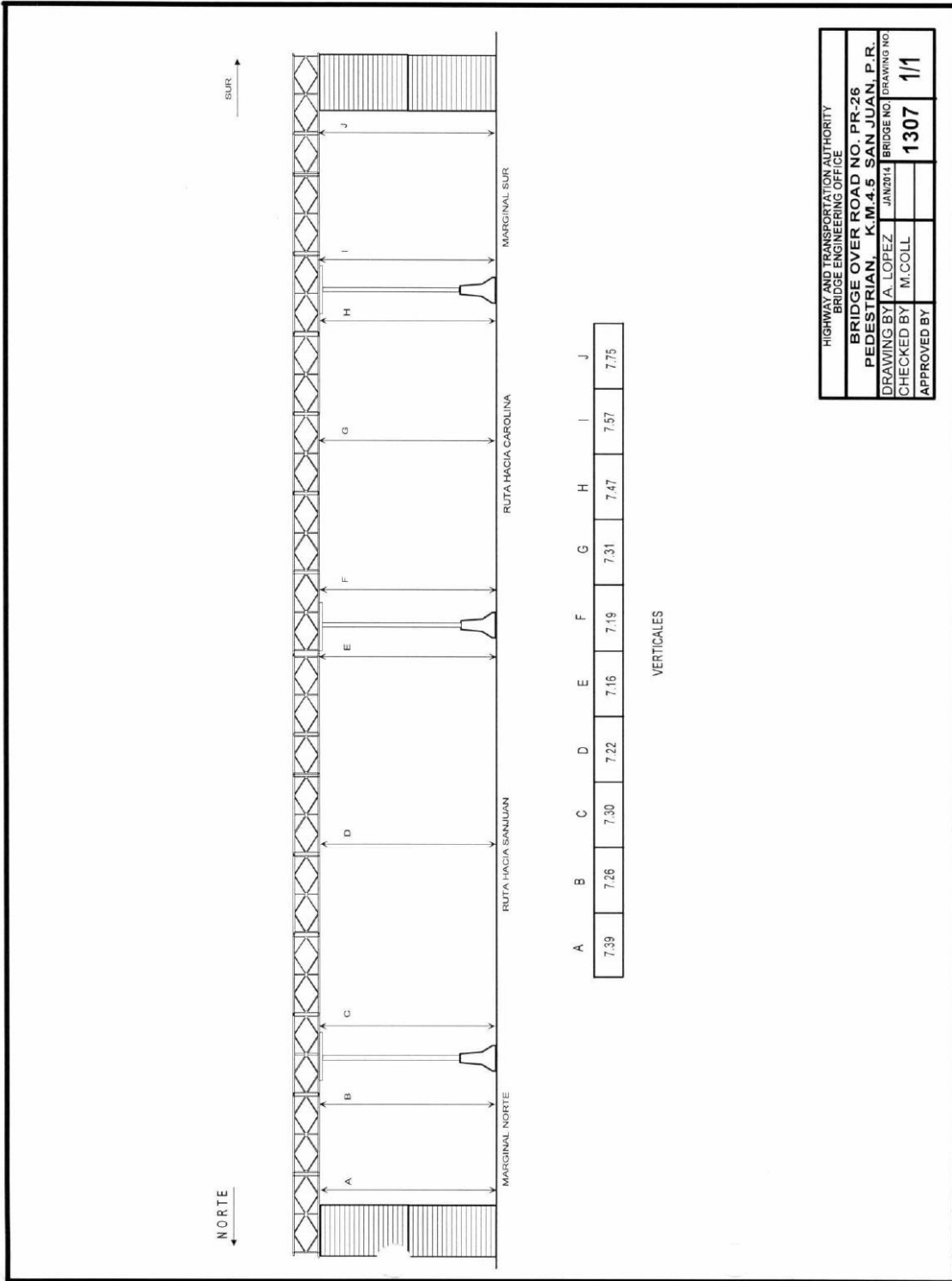
59.9 Vibrations: Minimal

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.2	Bracing	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.3	Columns	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.4	Footing	Concrete	6	F	Small	L	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

59.5- Corrosión moderada y severa en todos los miembros inferiores de la estructura. Hay perforaciones con pérdida de secciones. Las escaleras tienen corrosión severa y perforaciones con pérdidas de secciones.



A	B	C	D	E	F	G	H	I	J
7.39	7.26	7.30	7.22	7.16	7.19	7.31	7.47	7.57	7.75

VERTICALES

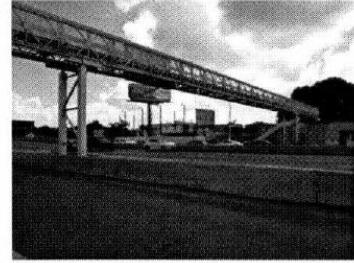
HIGHWAY AND TRANSPORTATION AUTHORITY	
BRIDGE ENGINEERING OFFICE	
BRIDGE OVER ROAD NO. PR-26	
PEDESTRIAN, K.M.4.5 SAN JUAN, P.R.	
DRAWING BY	JAN 2014
CHECKED BY	BRIDGE NO.
APPROVED BY	DRAWING NO.
M. COLL	1307
	1/1



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1307-jan-24-2014-002.JPG



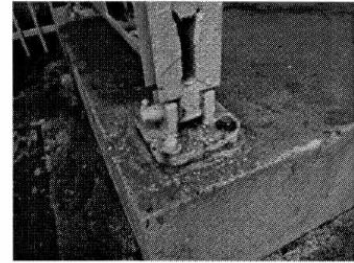
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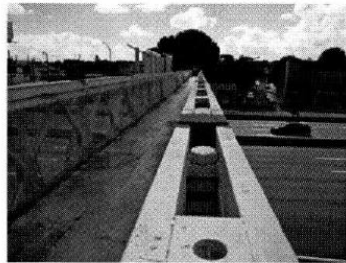
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1307-jan-24-2014-017.JPG



1307-jan-24-2014-036.JPG



1307-jan-24-2014-037.JPG

E.3 June 30, 2008

**BRIDGE ENGINEERING OFFICE
ENGINEERING SERVICE AREA**

Team Leader: Mayra I. Zayas

Bridge Inspector: Angel T. López

Bridge Evaluator: Mayra I. Zayas

Assistants: Ramón Rodríguez José R. Vázquez _ _ _

Driver: Jorge L. Viera

Inspection date: _ 30 JUNIO 2008

Weather Conditions: Sunny

Amount of Time on Inspection: 1.5 Hours

Equipment: Bus or Van Underwater Ladders Snooper Camera Boat
 Other:

Bridge Number: _ 1307

Road on Structure: N/A **Number or Name:** _ Km. _

Road Under Structure: State Highway **Number or Name:** _26 Km. _4.5

Ident. Plaque: No **Num.** _

Traffic Safety Features:

Bridge railings: not applicable or safety not required

Transitions: not applicable or safety not required

Approach Guardrail: not applicable or safety not required

Approach Guardrail Ends: not applicable or safety not required

COMMENTS AND/OR RECOMMENDATIONS:

Inspection by: **Angel T. López**
Bridge Inspector

Revised and Approved by: **Mayra I. Zayas**
Team Leader

**BRIDGE ENGINEERING OFFICE
ENGINEERING SERVICE AREA**

58. DECK

58.1 Wearing Surface: Material: Steel **Condition:** 5 Thickness cm.
 Deterioration: 50 to 75 % Drainage: Adequate Ponding: No Safety: Yes

58.2 Slab or Plate: Material: Steel **Condition:** 5
 Cracking: None
 Spalling: Top: No Bottom: No
 Scaling: Top: No Bottom: No
 Efflorescence: No Exudation: No Rust Stains: No Corrosion: Severe

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	N/A	--		--	--	--	--	--	--	--	--	--
58.8 Railing	Steel	5		Fair	--	Fair	Severe	Poor	Yes	--	--	--

58.9 Lighting Standards: Material: N/A Condition: N/A Functioning: N/A

58.10 Utilities:

Type: Size: Safety: __
 Type: Size: Safety: __
 Type: Size: Safety: __

58.11 Joints: Condition: N/A

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No
 Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: None Condition: N Functioning: N/A

Comments:

58.1- Corrosión severa con pérdidas de secciones y perforaciones en los laterales de la superficie de las tolas. Varias tolas están sueltas de sus puntos de apollo.

58.8- Corrosión severa con mallas de protección sueltas de sus puntos de apollo.

**BRIDGE ENGINEERING OFFICE
ENGINEERING SERVICE AREA**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Fixed Out of Plumb: No Paint: None

Condition: 5

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.

Condition: 5 Cracking: No Spalling: None Scaling: None

Crushing: No

59.3 Steel beams: N/A Type: N/A

Condition: N Corrosion: None Paint: None

59.4 Concrete: N/A Type: N/A

Condition: N Cracking: No Spalling: None Scaling: None

59.5 Truss: Bailey Pedestrian Paint: Poor Corrosion: Severe Members: Stringers **Condition:** 5

59.6 Drainage: Type: **Condition:** N Functioning: No

59.7 Hinges: **Condition:** N Functioning: N/A Movement: N/A

59.8 Deflection: Normal

59.9 Vibrations: Minimal

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.2	Bracing	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.3	Columns	Steel	5	--	--	--	Moderate	Poor	No	No	No
	60.2.4	Footing	Concrete	6	F	Small	L	--	--	No	No	No
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments: 59.5- Corrosión moderada y severa en todos los miembros inferiores de la estructura. Hay perforaciones con mucha pérdida de secciones. Las escaleras tienen corrosión severa y perforaciones con pérdidas de secciones.

**BRIDGE ENGINEERING OFFICE
ENGINEERING SERVICE AREA**

61. CHANNEL AND PROTECTION Condition: N

61.1 Channel Scour: N/A

61.2 Embankment Erosion: N/A

		Type	Material	Condition Rating	Functioning
61.3	Protective Device		N/A	N	--
61.4	Fender System		N/A	N	--
61.5	Rip Rap		N/A	N	--
61.6	Spur Dikes, Jetties		N/A	N	--

61.7 Obstruction: N/A

61.8 Channel Change: N/A

61.9 Adequate Waterway: N/A

61.10 Surrounding: Flooding: N/A if yes explain:

Erosion: N/A if yes explain:

61.11 Location of piers and/or abutment detrimental: N/A if yes explain:

61.12 Other features that may affect structure:

Comments:

62. CULVERT AND RETAINING WALL

	Material	Condition Rating	Functioning	Cracking	Spalling	Scaling	Corrosion	Settlement	Alignment	Scour
62.1 Barrel	N/A	N	--	--	--	--	--	--	--	--
62.2 Head Walls	N/A	N	--	--	--	--	--	--	--	--
62.3 Cut-off wall	N/A	N	--	--	--	--	--	--	--	--
62.4 Retaining wall	N/A	N	--	--	--	--	--	--	--	--

Comments:

Bridge Inspection Report

Bridge Key: 013071 Agency ID: 013071 Sufficiency Rating: -1.0

IDENTIFICATION
 State 1: 72 Puerto Rico Struc Num 8: 013071
 Facility Carried 7: PR 20 Location 9: BALDORIO DE CASTRO AVE.
 Rte.(On/Under)5A: One Route Under Rte. Signing Prefix 5B: 2 U.S. Numbered
 Level of Service 5C: 1 Mainline Rte. Number 5D: 00003
 Directional Suffix 5E: 0 N/A (NBI) % Responsibility: NA
 SHD District 2: SAN JUAN County Code 3: San Juan
 Place Code 4: 79770 Kilometer Post 11: 04.5 km
 Feature Intersected 8: PEDESTRIAN WALKWAY
 Latitude 16: 18d 27' 18" Longitude 17: 066d 03' 05"
 Border Bridge Code 9B: Unknown (P)
 Border Bridge Number 9B: NA

INSPECTION
 Frequency 91: 24 months Inspection Date 90: 3/14/2003 Next Inspection: 03/14/2005
 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA
 U/W Frequency 92B: NA U/W Inspection Date 93B: NA Next U/W Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 24 months Element Inspection Date: 03/14/2003 Next Elem. Insp. Due: 03/14/2005

STRUCTURE TYPE AND MATERIALS
 Number of Approach Spans 48: 0 Number of Spans Main Unit 46: 2
 Main Span Material/Design 43A/B:
 3 Steel 00
 Deck Type 107: 5 Steel Plate
 Wearing Surface 108A: 9 Other
 Membrane 108B: 0 None
 Deck Protection 108C: None

CLASSIFICATION
 Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: No || bridge exists
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Unknown (NBI)
 Highway System 104: 0 Not on NHS NBIS Length 112: Long Enough
 Toll Facility 20: 3 On free road Functional Class 28: 12 Urban Fwy/Expwy
 Historical Significance 37: 5 Not eligible for NRHP
 Owner 22: 0101 State Highway Agency
 Custodian 21: 01 01 State Highway Agency

AGE AND SERVICE
 Year Built 27: 1958 Year Reconstructed 106: Unknown
 Type of Service under 42A: 3 Pedestrian-bicycle
 Type of Service under 42B: 1 Highway
 Lanes on 28A: Unknown Lanes Under 28B: 8 Delour Length 19: 00 km
 ADT 29: 65,300 Truck ADT 109: 5 % Year of ADT 30: 2000

CONDITION
 Deck 58: 7 Good Super 59: 6 Satisfactory Sub 60: 6 Satisfactory
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

GEOMETRIC DATA
 Length Max Span 48: 21.70 m Structure Length 49: 55.60 m
 Curb/Sidewalk Width L 50A: 0.00 m Curb/Sidewalk Width R 50B: 0.00 m
 Width Curb to Curb 51: 1.60 m Width Out to Out 52: 2.80 m
 Approach Roadway Width 32: 29.30 m Median 33: 0 No median (w/ shoulders)
 Deck Area:
 Skew 34: 0.00 ° Structure Flared 35: 0 No flare
 Minimum Vertical Clearance Over Bridge 53: 99.99 m
 Minimum Vertical Underclearance Reference 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 05.21 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 01.70 m
 Minimum Lateral Underclearance L 56: 01.50 m

LOAD RATING AND POSTING
 Inventory Rating Method 65: 2 AS Allowable Stress Operating Rating Method 63: 2 AS Allowable Stress
 Inventory Rating 66: MS44.4 Operating Rating 64: MS44.4
 Design Load 31: 7 Pedestrian Posting 70: 5 At/Above Legal Loads
 Posting status 41: A Open, no restriction

APPRAISAL
 Bridge Rail 36A: N N/A or not required Approach Rail 36C: N N/A or not required
 Transition 36B: N N/A or not required Approach Rail Ends 36D: N N/A or not required
 Str. Evaluation 67: Unknown (NBI) Deck Geometry 68: Unknown (NBI)
 Underclearance, Vertical and Horizontal 69: Unknown (NBI)
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: 6 Equal Min Criteria
 Scour Critical 113: N Not Over Waterway

PROPOSED IMPROVEMENTS
 Bridge Cost 94: \$ 0 Type of Work 75: 36 Rehabilitate-gen.
 Roadway Cost 95: \$ 0 Length of Improvement 76: 55.60 m
 Total Cost 96: \$ 21,000 Future ADT 114: 92,703
 Year of Cost Estimate 97: 2003 Year of Future ADT 115: 2020

NAVIGATION DATA
 Navigation Control 38: N NA-no waterway
 Vertical Clearance 39: 0.00 m Horizontal Clearance 40: 0.00 m
 Pier Protection 111: Unknown (NBI) Lift Bridge Vertical Clearance 116:

ELEMENT CONDITION STATE DATA

Str Unit	Elm/Env	Description	Units	Total Qty	% in 1	Qty. St. 1	% in 2	Qty. St. 2	% in 3	Qty. St. 3	% in 4	Qty. St. 4	% in 5	Qty. St. 5
2	115/3	P/S Conc Stringer	m.	223	0 %	0	60 %	133	0 %	0	40 %	90	0 %	0
2	121/3	P/Stl Thru Truss/Bot	m.	112	0 %	0	70 %	78	0 %	0	30 %	34	0 %	0
2	126/3	P/Stl Thru Truss/Top	m.	112	0 %	0	91 %	102	0 %	0	9 %	10	0 %	0
2	152/3	Paint Stl Floor Beam	m.	46	0 %	0	26 %	12	0 %	0	74 %	34	0 %	0
2	202/3	Paint Stl Column	ea.	3	0 %	0	100 %	3	0 %	0	0 %	0	0 %	0

Bridge Inspection Report

Str Unit	Elem/Env	Description	Element Notes
2	116/3	P/S Conc Stringer	
2	121/3	Painted Steel Bottom Chord Thru T	
2	126/3	Painted Steel Thru Truss (excl. bot	
2	152/3	Painted Steel Floor Beam	
2	202/3	Painted Steel Column or Pile Exter	corrosion in lower part of the column.

BRIDGE NOTES

CULVERT

PAST INSPECTION

Inspection Date: 03/14/2003 Type: 1 Regular NBI
 Inspector: Pontis Pontis User Key: Pontis - Pontis Poi
 Scope:
 NBI: Other: Element:
 Underwater: Fracture Critical:

INSPECTION NOTES

PONTIS inspection comments -
 Structure 013071 -
 Date 2003-03-14 -
 Previous comments > < none >

PAST INSPECTION

Inspection Date: 05/01/1998 Type: 1 Regular NBI
 Inspector: -1 Pontis User Key: Pontis - Pontis Poi
 Scope:
 NBI: Other: Element:
 Underwater: Fracture Critical:

INSPECTION NOTES

INSPECTOR WORK CANDIDATES
 INSP002_Inspect_Report_Metric

Agency ID: 013071

Thu 4/24/2003 08:46:18
 Page 2 of 3

E.5 November 17, 2000

ENGINEER: HERIBERTO GONZALEZ ASSISTANTS: RAUL VAZQUEZ : JULIO DE HOSTOS : LUIS CACHO : :		PUERTO RICO HIGHWAY AUTHORITY BRIDGE STUDIES AND EVALUATION OFFICE BRIDGE REINSPECTION & EVALUATION REPORT BRIDGE No.: 01307 FEDERAL SYSTEM 88888888888888888888 ROAD No.: PR 26 KM. No. : 0004.500		PAGE 1 OF 7 EQUIPMENT BUS X LADDERS BOAT CAMERA X UNDERWATER SNOOPER # SNOOPER OPERATOR PLAQUE ID YRS X NO No. FILM No. 135	
---	--	--	--	---	--

EVALUATION DATE: 17-NOV-2000

=====IDENTIFICATION=====

1- STATE :	Commonwealth of Puerto Rico	721
2- HIGHWAY AGENCY DISTRICT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-2-1-00002-0	221000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 26	PR 26
8- STRUCTURE NUMBER :	BRIDGE 1307 1 OF 1	013071
9- LOCATION :	BALDORIOTY DE CASTRO AV.	BALDORIOTY DE CASTRO AV.
10- INVENTORY ROUTE, MINIMUM VERTICAL CLEARANCE :	5.21 MT (17' - 1")	0521
11- KILOMETERPOINT :	4.50	0004500
12- BASE HIGHWAY NETWORK :	INVENTORY ROUTE IS ON THE BASH NETWORK	0
13- LRS INVENTORY ROUTE, SUBROUTE NUMBER :		
16- LATITUDE :	18 DEGREES 27.3 MINUTES	18271800
17- LONGITUDE :	66 DEGREES 03.1 MINUTES	066030600
19- BYPASS, DETOUR LENGTH (NEAREST KILOMETER) :	STRUCTURE OVER HIGHWAY	000

=====CLASSIFICATION=====

20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE	12

=====AGE AND SERVICE=====

27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / 8 LANES UNDER	0008
29- A.D.T. OF INVENTORY ROUTE :	67,300	067300
30- YEAR OF A.D.T. :	1999	1999

=====STRUCTURE DATA=====

31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	29.29 MT (96.07 FT)	0293
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NONE	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	NO	0000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO	00000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000

IDBR No.: 01307

59- SUPERSTRUCTURE: FAIR RATING: 5 SUPERSTRUCTURE COMMENTS:
 59.1 BEARING DEVICES: TYPE: FIXED MOVABLE OUT OF PLUMB MODERATE TO SEVERE CORROSION ON SEVERA
 PAINT: GOOD FAIR POOR NONE USELESS TO PAINT L BRIDGE COMPONENTS.
 CONDITION: FUNCTIONING: YES NO
 59.2 BRIDGE SEATS, PEDESTAL, GROUT PADS, ABUTMENTS OR PIER SEATS WHERE BEAMS BEAR
 DIRECTLY ON CONCRETE
 CONDITION: 5 CRACKING: F M O SPALLING: S L
 SCALING: L M H S CRUSHING:
 59.3 STEEL-BEAMS: TYPE: DIMENSIONS (height; width; thickness):
 CONDITION: CORROSION: L M S PAINT: G F P N U
 59.4 CONCRETE-BOX BEAMS, T-BEAMS, SLAB, T-BEAM, TYPE:
 CONDITION: CRACKING: F M O SPALLING: S L
 SCALING: L M H S
 59.5 TRUSSES: TYPE: BAILEY PED CONDITION: 5 CORROSION: L X M X S
 DAMAGE: DEFORMATION: PAINT: G X F X P N U
 A=TRUSS B=UPPER & LOWER LATERAL BRACING C=PORTALS D=FLOOR BEAMS E=STRINGERS
 59.6 DRAINAGE: TYPE: CONDITION: FUNCTIONING: YES NO
 59.7 HINGES: CONDITION: FUNCTIONING: YES NO MOVEMENT: YES NO
 ALIGNMENT: GOOD BAD PAINT: G F P N U
 59.8 DEFLECTIONS: X NORMAL EXCESSIVE cms.
 59.9 VIBRATIONS: X MINIMAL MODERATE EXCESSIVE

60- SUBSTRUCTURE: FAIR RATING: 5

	ABUTMENTS				PIERS OR NON PILES BENTS					PILES BENTS				
	60.1.1	60.1.2	60.1.3	60.1.4	60.2.1	60.2.2	60.2.3	60.2.4	60.2.5	60.4.1	60.4.2	60.4.3		
	WINGWALL	BREAST	FOOTING	PILES	CAPS	BRACING	COLUMNS	FOOTINGS	PILES	CAPS	BRACING	PILES		
	BACKWALL													
MATERIAL					STEEL	STEEL	STEEL	CONCRETE						
CONDITION RATING					5	5	5	6						
CRACKING (F-M-O)								F						
SPALLING (S-L)								S						
SCALING (L-M-H-S)								L						
CORROSION (L-M-S)					M	M	M							
PAINT (G-F-P-N-U)					P	P	P							
MOVEMENT					NO	NO	NO	NO						
EROSION														
UNDERMINING														

60- SUBSTRUCTURE COMMENTS:

IDGR No.: 01307

67- STRUCTURAL EVALUATION : 3

MODERATE & SEVERE CORROSION AT STEEL PLATES, RAILINGS, UPPER CHORD & STRINGS.
 MODERATE CORROSION AT BRACINGS & BETWEEN LOWER CHORDS & FLOORBEAMS.
 LIGHT CORROSION AT STEEL COLUMNS & STAIRS. MODERATE CORROSION AT REST AREA.

68- DECK GEOMETRY : N

N/A.

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : 13 ft

(5.21m) (1.50m)
 EQUAL TO PRESENT MINIMUM CRITERIA.

70- BRIDGE POSTING : N/A. 5

71- WATERWAY ADEQUACY : N

N/A.

REMOTE - GREATER THAN 100 YEARS. SLIGHT - 11 TO 100 YEARS. OCCASIONAL - 3 TO 10 YEARS. FREQUENT - LESS THAN 3 YEARS.
 INSIGNIFICANT - MINOR INCONVENIENCE. HIGHWAY PASSABLE IN A MATTER OF HOURS. SIGNIFICANT - TRAFFIC DELAYS OF UP TO SEVERAL DAYS
 SEVERE - LONG TERM DELAYS TO TRAFFIC WITH RESULTING HARDSHIP

72- APPROACH ROADWAY ALIGNMENT : 6

N/A.

72.2 SLAB OR PAVEMENT	CONDITION :	MATERIAL :							
CRACKING:	F N O	SPALLING:	L S	SCALING:	L M H S	UNNEVEN:	YES NO	ROUGH:	YES NO
SETTLEMENT:	YES NO	MOVEMENT:	APPROACH SLAB-BACKWALL:	YES NO	BREAKING UP:	EXCESSIVE DEFORMATION:			
SAFETY:	HAZARDOUS YRS NO	DRAINAGE:	INADEQUATE:	YRS NO	FONDING:	YRS NO			
MOVEMENT:	PAVEMENT-APPROACH SLAB:	YES NO	EMBANKMENT:	CONDITION:					
	FUNCTIONING:	YES NO	EROSION:	YES NO	IF YES, EXPLAIN				
CONSTRUCTION:	FLIMSY:	YES NO							
	INTEGRITY IMPAIRED:	YES NO	IF YES, EXPLAIN						

72.3 UNDESIRABLE IMPACT: YES NO

72.4 JOINTS: TYPE: INADEQUATE: YES NO SATISFACTORY ALIGNMENT: YES NO

72.5 GUARDRAIL: TYPE: MATERIAL: CONDITION: FUNCTIONING: YES NO

ALIGNMENT: HORIZONTAL: GOOD BAD VERTICAL: GOOD BAD JOINTS: FUNCTIONING: YES NO

SAFETY: SECURELY ATTACHED: YES NO PEDESTRIAN HAZARDS: YES NO

72- APPROACH ROADWAY - COMMENTS:

BRIDGE NO. : 01307

=====PROPOSED IMPROVEMENT=====

75- TYPE OF WORK :	REHABILITATION	352
76- LENGTH OF STRUCTURE IMPROVEMENT :	55.63 MT. (182.47 FT)	000556

=====INSPECTIONS=====

89- RESERVED		--
90- INSPECTION DATE :	NOVEMBER-17-2000	1100
91- DESIGNATED INSPECTION FREQUENCY :	24 MONTHS	24
92- CRITICAL FEATURE INSPECTION :	N/A	N N N
93- CRITICAL FEAT. INSPECT. DATE :	N/A	
FRACTURE CRITICAL	UNDERWATER	OTHER

=====IMPROVEMENT COST=====

94- BRIDGE IMPROVEMENT COST :	N/A	000000
95- ROADWAY IMPROVEMENT COST :	N/A	000000
96- TOTAL PROJECT COST :	(55.63X2.84X\$1315X10%)=\$20,776	000021
97- YEAR OF IMPROVEMENT COST ESTIMATED :	2000	2000

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE :		--
99- BORDER BRIDGE STRUCTURE NUMBER :		--
100- STRAHNET HIGHWAY DESIGNATION :	DEFENSE HIGHWAY	2
101- PARALLEL STRUCTURE DESIGNATION :	NO PARALLEL STRUCTURE	N
102- DIRECTION OF TRAFFIC :	TRAFFIC NOT CARRIED	0
103- TEMPORARY STRUCTURE DESIGNATION :	N/A	
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE :	N.H.S.	0
105- FEDERAL LANDS HIGHWAYS :		0
106- YEAR RECONSTRUCTION :	NO RECONSTRUCTION	0000
107- DECK STRUCTURE TYPE :	STEEL PLATE	5
108- WEARING SURF. / PROTECT. SYSTEM :	OTHER - NONE - NONE	900
109- AVERAGE DAILY TRUCK TRAFFIC :	5 %	05
110- DESIGNATED NATIONAL NETWORK :	PART OF NETWORK	1
111- PIER OR ABUT. PROTECTION (NAVIGATION) :	N/A	
112- NBIS BRIDGE LENGTH :	YES	Y
113- SCOUR CRITICAL BRIDGE :	N/A	N
114- FUTURE AVERAGE DAILY TRAFFIC :	94,893	094893
115- YR. OF FUTURE A.D.T. :	2019	2019
116- MINIM. NAVIG. VERT. CLEARANCE :	N/A	
VERTICAL LIFT BRIDGE :		
117- SUFFICIENCY RATING :		
125- PRIORITY RATING :		
127- EVALUATION DATE :	17-NOV-2000	111700
130- CRITICAL FRACTURE INSP. DATE :		

ENGINEER : HERRIBERTO GONZALEZ

AL SYSTEM

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

PAGE 7 OF 7

BRIDGE NO. : 01307

AUXILIARY ITEMS

1-SIGNS

TYPE: ORIENTACION "SAN JUAN - BAYAMON - RIO PIEDRAS"

MATERIAL: ALUMINIO

CONDITION: 7

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE
ITEM #67.

E.6 May 8, 1998

ENGINEER: INEBELLE VELEZ
 ASSISTANTS: L. QUIÑONES
 : ERAZO
 :
 :
 :
 :
 EVALUATION DATE: 8-MAYO-98

PUERTO RICO HIGHWAY AUTHORITY
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PAGE 1 OF 7

EQUIPMENT

BUS X LADDERS
 BOAT CAMERA X
 UNDERWATER SNOOPER #
 SNOOPER OPERATOR
 PLAQUE ID
 YES X NO No.
 FILM No. 135

BRIDGE No.: 01307 FEDERAL SYSTEM
 ROAD No.: PR 26 XM. No.: 0004.500

-----IDENTIFICATION-----

1- STATE :	Commonwealth of Puerto Rico	721
2- HIGHWAY AGENCY DISTRICT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-1-1-00003-0	211000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 26	PR 26
8- STRUCTURE NUMBER :	BRIDGE 1307 1 OF 1	013071
9- LOCATION :	BALDORIOZY DE CASTRO AVE.	BALDORIOZY DE CASTRO AVE.
10- INVENTORY ROUTE, MINIMUM VERTICAL CLEARANCE :	5.21 MT (17' - 1")	0520
11- KILOMETERPOINT :	4.50	0004500
12- BASE HIGHWAY NETWORK :	INVENTORY ROUTE IS ON THE BASE NETWORK	1
13- LRS INVENTORY ROUTE, SUBROUTE NUMBER :		000000002600
16- LATITUDE :	18 DEGREES 27.3 MINUTES	18271800
17- LONGITUDE :	66 DEGREES 03.1 MINUTES	066030600
19- BYPASS, DETOUR LENGTH (NEAREST KILOMETER) :	STRUCTURE OVER HIGHWAY	000

-----CLASSIFICATION-----

20- TOLL :	ON FREE ROAD	3
21- MAINTENANCE RESPONSABILITY :	D.T.P.W.	01
22- OWNER :	D.T.P.W.	01
26- FUNCTIONAL CLASS. OF INVENTORY ROUTE :	URBAN INTERSTATE	11

-----AGE AND SERVICE-----

27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / 8 LANES UNDER	0008
29- A.D.T. OF INVENTORY ROUTE :	65,300	065300
30- YEAR OF A.D.T. :	1996	1996

-----STRUCTURE DATA-----

31- DESIGN LOAD :	PEDESTRIAN	7
32- APPROACH ROADWAY WIDTH :	29.29 MT (96.07 FT)	0293
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NONE	00
35- STRUCTURE FLARED :	NO	0
36- TRAFFIC SAFETY FEATURES :	N-N-N-N	NNNN
37- HISTORICAL SIGNIFICANCE :	5	5
38- NAVIGATION CONTROL :	N/A	N
39- NAVIGATION VERTICAL CLEARANCE :	NO	0000
40- NAVIGATION HORIZONTAL CLEARANCE :	NO	0000
41- STRUCT. OPEN, POSTED OR CLOSED TO TRAFF. :	OPEN	A
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000

IDGE No.: 01307

- 47- INV. ROUTE, TOTAL HORIZ. CLEARANCE : (14.40 mt) 10.82 MT. (35.49 FT)
- 48- LENGTH OF MAXIMUM SPAN : 21.65 MT. (71.01 FT)
- 49- STRUCTURE LENGTH : 55.63 MT. (182.47 FT)
- 50- CURB OR SIDEWALK WIDTHS : NONE

144
102
002
00055
00000

-----STRUCTURE DATA-----

- 51- BRIDGE ROADWAY WIDTH, CURB TO CURB : 1.64 MT. (5.38 FT)
- 52- DECK WIDTH, OUT TO OUT : 2.84 MT. (9.32 FT)
- 53- MI. VERT. CLEAR. OVER BRIDGE ROADWAY : UNLIMITED
- 54- MINIMUM VERTICAL UNDERCLEARANCE : 5.21 MT. (17' - 1")
- 55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT : 1.68 MT. (5.51 FT)
- 56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT : 1.50 MT. (4.92 FT)

0016
0028
9999
H052
H017
015

-----CONDITION-----

58- DECK: FAIR RATING: 5

DECK COMMENTS:

- 58.1 WEARING SURFACE: MATERIAL: STEEL CONDITION : 5
- THICKNESS: -
- DETERIORATION: X YES NO DRAINAGE: X ADEQUATE INADEQUATE
- PONDING: YES X NO SAFETY: X YES NO
- 58.2 SLAB OR PLATE: MATERIAL: STEEL CONDITION 5
- CRACKING: FINE MEDIUM OPEN

58.1 Y 58.2) TOLES DE ACERO CON LEVE CORROSION.
58.8) RAILING CON LEVE CORROSION. MODERADA CORROSION EN LAS SECCIONES #9, #10 Y #11 LADO OESTE.

	TOP	BOTTOM
SPALLING:	S < 1 in. DEEP x 6 in. & L > 1 in. x 6 in. &	
SCALING :	L to h in., M h in. - 8 in., H 8 in. - 1 in., S > 1	

EFFLORESCENCE EXUDATION RUST STAINS CORROSION: LIGHT MODERATE SEVERE

58.3 MOVEMENT: DECK TO BACKWALL - cm. DECK TO APPROACH SLAB cm.

	(58.4)	(58.5)	(58.6)	(58.7)	(58.8)
	CURBS	MEDIAN	SIDEWALKS	PARAPETS	RAILING
MATERIAL					STEEL
CONDITION RATING					5
HEIGHT LOSS (cm)					-
JOINTS					
DRAINAGE					G
ALIGNMENT					G
CORROSION (L-M-S)					M
PAINT: G-GOOD, F-FAIR, P-POOR, N-NONE					F
U-USELESS TO PAINT					
SAFETY					YES
CRACKING (F-M-O)					-
SPALLING (S-L)					-
SCALING (L-M-H-S)					-

- 58.9 LIGHTING STANDARDS: MATERIAL: - CONDITION: - FUNCTIONING: YES NO
- 58.10 UTILITIES: TYPE: - SIZE: SAFETY:
- 58.11 JOINTS: EXPANSION CONSTRUCTION CONDITION: FUNCTIONING: YES NO
- LEAKING: YES NO CRACKING: FINE MEDIUM OPEN SPALLING: SMALL LARGE
- 58.12 DRAINS AND SCUPPERS: MATERIAL: CONDITION: FUNCTIONING: YES NO

IDGE No.: 01307

59- SUPERSTRUCTURE: FAIR RATING: 5 SUPERSTRUCTURE COMMENTS:

59.1 BEARING DEVICES: TYPE: FIXED MOVABLE OUT OF PLUMB 59.5)UPPER CHORD TIENE LEVE Y MODERADA
PAINT: GOOD FAIR POOR NONE USELESS TO PAINT CORROSION.MODERADA CORROSION EN LO
CONDITION: FUNCTIONING: YES NO S PANELES #9,#10 Y #11 DE SUR A NORTE
59.2 BRIDGE SEATS, PEDESTAL, GROUT PADS, ABUTMENTS OR PIER SEATS WHERE BEAMS BEAR LADO OESTE.
DIRECTLY ON CONCRETE
CONDITION: 6 CRACKING: F M O SPALLING: S L MODERADA CORROSION EN EL LOWER
SCALING: L M H S CRUSHING: CHORD EN AREA DESCANSO DE FLOOR BEAM.F
59.3 STEEL-BEAMS: TYPE: DIMENSIONS (height;width;thickness): LOOR BEAM TIENE MODERADA CORROSION EN
CONDITION: CORROSION: L M S PAINT: G F P N U UNION AL LOWER CHORD.
59.4 CONCRETE-BOX BEAMS, I-BEAMS, SLAB, T-BEAM, TYPE: STRINGERS CON MODERADA CORROSION
CONDITION: CRACKING: F M O SPALLING: S L (EN EL 40% DE ELLOS).
SCALING: L M H S
59.5 TRUSSES: TYPE: BAILEY CONDITION: 5 CORROSION: L M S
DAMAGE: DEFORMATION: PAINT: G X F P N U
A=TRUSS B=UPPER & LOWER LATERAL BRACING C=PORTALS D=FLOOR BEAMS E*-STRINGERS
59.6 DRAINAGE: TYPE: CONDITION: FUNCTIONING: YES NO
59.7 HINGES: CONDITION: FUNCTIONING: YES NO MOVEMENT: YES NO
ALIGNMENT: GOOD BAD PAINT: G F P N U
59.8 DEFLECTIONS: X NORMAL EXCESSIVE cms.
59.9 VIBRATIONS: X MINIMAL MODERATE EXCESSIVE

60- SUBSTRUCTURE: FAIR RATING: 5

	ABUTMENTS				PIERS OR NON PILES BENTS				PILES BENTS			
	60.1.1	60.1.2	60.1.3	60.1.4	60.2.1	60.2.2	60.2.3	60.2.4	60.2.5	60.4.1	60.4.2	60.4.3
	WINGWALL	BREAST & BACKWALL	FOOTING	PILES	CAPS	BRACING	COLUMNS	FOOTINGS	PILES	CAPS	BRACING	PILES
MATERIAL					STEEL	STEEL	STEEL	CONCRETE				
CONDITION RATING	---	---	---	---	6	5	6	6	---	---	---	---
CRACKING (F-M-O)					-	-	-	-				
SPALLING (S-L)												
SCALING (L-M-H-S)					-	-	-	-				
CORROSION (L-M-S)					L	M	L					
PAINT (G-F-P-N-U)					F	F	-	-				
MOVEMENT					N	N	N	N				
EROSION					N	N	N	N				
UNDERMINING					N	N	N	N				

60- SUBSTRUCTURE COMMENTS:

60.2.2)MODERADA CORROSION CON PERDIDA DE SECCIONES EN LOS BRACING.
60.2.3)PILASTRA TIENE COLUMNAS CON LEVE CORROSION. ESCALERAS CON LEVE CORROSION. MODERADA CO
60.2.4)FOOTING EN BUENAS CONDICIONES. RROSION, EN AREA DE DESCANSO.

IDGE No.: 01307

61- CHANNEL & CHANNEL PROTECTION : N/A
 61.1 CHANNEL SCOUR (EXTENT) :
 61.2 EMBANKMENT EROSION (EXTENT) :

RATING : N

CHANNEL COMMENTS:

	TYPE	MATERIAL	CONDITION		FUNCTIONING		
			RATING			YES	NO
61.3 PROTECTIVE DEVICE							
61.4 FENDER SYSTEM	----		----	----	----	----	
61.5 RIP RAP							
61.6 SPUR DIKES, JETTIES							

61.7 OBSTRUCTION (DEBRIS, GROWTHS) :
 61.8 CHANNEL CHANGE: YES NO
 DETRIMENTAL: YES NO IF YES, EXPLAIN
 61.9 ADEQUATE WATERWAY: YES NO IF NO, EXPLAIN
 61.10 SURROUNDING AREA: FLOODING: YES NO IF YES, EXPLAIN
 EROSION: YES NO IF YES, EXPLAIN
 61.11 LOCATION OF PIERS AND/OR ABUTMENTS: DETRIMENTAL: YES NO
 IF YES, EXPLAIN
 61.12 OTHER FEATURES THAT MAY AFFECT STRUCTURE:

62- CULVERT AND RETAINING WALLS :N/A

RATING :N

CULVERT COMMENTS:

	62.1 BARREL	62.2 HEADWALL	62.3 CUT-OFF WALL	62.4 RETAINING WALL
MATERIAL				
CONDITION RATING	----	----	----	----
FUNCTIONING (Y-N)				
CRACKING (F-M-O)				
SPALLING (S-L)				
SCALING (L-M-H-S)				
CORROSION (L-M-S)				
SETTLEMENT (Y-N)				
ALIGNMENT (Y-N)				
PAINT (G-F-P-N-U)				

63- METHOD USED TO DETERMINE OPERATING RATING .. 2
 64- OPERATING RATING : PEDESTRIAN 800
 65- METHOD USED TO DETERMINE INVENTORY RATING .. 2
 66- INVENTORY RATING : PEDESTRIAN 800

IDGE No.: 01307

67- STRUCTURAL EVALUATION : *05/8*

LIGHT TO MODERATE CORROSION AT STEEL PLATES, RAILINGS, UPPER CHORD & STRINGS. MODERATE CORROSION AT BRACINGS & BETWEEN LOWER CHORDS & FLOORBEAMS.
LIGHT CORROSION AT STEEL COLUMNS & STAIRS. MODERATE CORROSION AT REST AREA.

68- DECK GEOMETRY : *ON/A*

N/A.

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : *06/8*

EQUAL TO PRESENT MINIMUM CRITERIA.

70- BRIDGE POSTING : N/A. 5

71- WATERWAY ADEQUACY : N

N/A.

REMOTE - GREATER THAN 100 YEARS. SLIGHT - 11 TO 100 YEARS. OCCASIONAL - 3 TO 10 YEARS. FREQUENT - LESS THAN 3 YEARS.
INSIGNIFICANT - MINOR INCONVENIENCE. HIGHWAY PASSABLE IN A MATTER OF HOURS. SIGNIFICANT - TRAFFIC DELAYS OF UP TO SEVERAL DAYS
SEVERE - LONG TERM DELAYS TO TRAFFIC WITH RESULTING HARDSHIP

72- APPROACH ROADWAY ALIGNMENT : *JEC*

N/A.

72.2 SLAB OR PAVEMENT	CONDITION :	MATERIAL :							
CRACKING:	F N O	SPALLING:	L S	SCALING:	I M H S	UNEVEN:	YES NO	ROUGH:	YES NO
SETTLEMENT:	YES NO	MOVEMENT:	APPROACH SLAB-BACKWALL:	YES NO	BREAKING UP:	EXCESSIVE DEFORMATION:			
SAFETY:	HAZARDOUS YES NO	DRAINAGE:	INADEQUATE:	YES NO	PONDING:	YES NO			
MOVEMENT:	PAVEMENT-APPROACH SLAB:	YES NO	EMBANKMENT:	CONDITION:					
	FUNCTIONING:	YES NO	EROSION:	YES NO	IF YES, EXPLAIN				
CONSTRUCTION:	FLIMSY:	YES NO							
	INTEGRITY IMPAIRED:	YES NO	IF YES, EXPLAIN						

72.3 UNDESIRABLE IMPACT: YES NO

72.4 JOINTS: TYPE: INADEQUATE: YES NO SATISFACTORY ALIGNMENT: YES NO

72.5 GUARDRAIL: TYPE: MATERIAL: CONDITION: FUNCTIONING: YES NO

ALIGNMENT: HORIZONTAL: GOOD BAD VERTICAL: GOOD BAD JOINTS: FUNCTIONING: YES NO

SAFETY: SECURELY ATTACHED: YES NO PEDESTRIAN HAZARDS: YES NO

72- APPROACH ROADWAY - COMMENTS:

AL SYSTEM
IDGE NO. :01307

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

-----PROPOSED IMPROVEMENT-----

75- TYPE OF WORK : REHABILITATION
76- LENGTH OF STRUCTURE IMPROVEMENT : 55.63 MT.(182.47 FT)

352
00055/10/ff

-----INSPECTIONS-----

89- RESERVED
90- INSPECTION DATE : MAY 8,1998.
91- DESIGNATED INSPECTION FREQUENCY : 24 MONTHS
92- CRITICAL FEATURE INSPECTION : N/A
93- CRITICAL FEAT. INSP. DATE: N/A
FRACTURE CRITICAL UNDERWATER OTHER

--
0598
24
N N N

-----IMPROVEMENT COST-----

94- BRIDGE IMPROVEMENT COST : N/A
95- ROADWAY IMPROVEMENT COST : N/A
96- TOTAL PROJECT COST : (55.63X2.84X\$1315X10%)=\$20,776
97- YEAR OF IMPROVEMENT COST ESTIMATED : 1998

000000
000000
000021
1998

==CLASSIFICATION AND STRUCTURE DATA==

98- BORDER BRIDGE :
99- BORDER BRIDGE STRUCTURE NUMBER :
100- STRAIGHT HIGHWAY DESIGNATION : DEFENSE HIGHWAY
101- PARALLEL STRUCTURE DESIGNATION : NO PARALLEL STRUCTURE
102- DIRECTION OF TRAFFIC : TRAFFIC NOT CARRIED
103- TEMPORARY STRUCTURE DESIGNATION : N/A
104- HIGHWAY SYSTEM OF THE INVENTORY ROUTE : N.H.S.
105- FEDERAL LANDS HIGHWAYS :
106- YEAR RECONSTRUCTION : NO RECONSTRUCTION
107- DECK STRUCTURE TYPE : STEEL PLATE
108- WEARING SURF./ PROTECT. SYSTEM : OTHER - NONE - NONE
109- AVERAGE DAILY TRUCK TRAFFIC : 5 %
110- DESIGNATED NATIONAL NETWORK : PART OF NETWORK
111- PIER OR ABUT. PROTECTION (NAVIGATION) : N/A
112- NBIS BRIDGE LENGTH : YES
113- SCOUR CRITICAL BRIDGE : N/A
114- FUTURE AVERAGE DAILY TRAFFIC : 92,703
115- YR. OF FUTURE A.D.T. : 2016
116- MINIM. NAVIG. VERT. CLEARANCE : N/A
VERTICAL LIFT BRIDGE :
117- SUFFICIENCY RATING :
125- PRIORITY RATING:
127- EVALUATION DATE : 8-MAYO-98
130- CRITICAL FRACTURE INSP. DATE :

--
0
0
0000
5
900
05
1
Y
N
092703
2016

ENGINEER :INEABELLE VELES

RAL SYSTEM

PUERTO RICO HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE
BRIDGE REINSPECTION & EVALUATION REPORT

PAGE 7 OF 7

IDGE NO. :01307

AUXILIARY ITEMS

1-SIGNS

TYPE: ORIENTACION "SAN JUAN - BAYAMON - RIO PIEDRAS"

MATERIAL: ALUMINIO

CONDITION: 7

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE
ITEM #67.

E.7 September 24, 1993

PUERTO RICO HIGHWAY AUTHORITY HIGHWAY SYSTEM ADMINISTRATION OFFICE BRIDGE REPORT		PAGE 1 OF 3 DATE : 09/30/93 TIME : 10:49:44
FEDERAL SYSTEM	BRIDGE NO. : 01307 - ROAD NO. : PR 26	- KN. NO. : 004500
=====IDENTIFICATION=====		
1- STATE :	Commonwealth of Puerto Rico	721
2- STATE HIGHWAY DEPARTMENT :	SAN JUAN	01
3- COUNTY (PARISH) CODE :	SAN JUAN	127
4- PLACE CODE :	SAN JUAN URBAN ZONE	76770
5- INVENTORY ROUTE :	2-1-1-00003-0	211000030
6- FEATURES INTERSECTED :	PEDESTRIAN WALKWAY	PEDESTRIAN WALKWAY
7- FACILITY CARRIED BY STRUCTURE :	P.R. - 26	PR 26
8- STRUCTURE NUMBER :	BRIDGE 1307 1 OF 1	013071
9- LOCATION :	BALDORTOTY DE CASTRO AVE.	BALDORTOTY DE CASTRO AVE.
11- MILEPOINT :	4.50	004500
16- LATITUDE :	18 DEGREES 27.3 MINUTES	18273
17- LONGITUDE :	66 DEGREES 03.1 MINUTES	066031
98- BORDER BRIDGE :		--
99- BORDER BRIDGE STRUCTURE NUMBER :		--
=====STRUCTURE TYPE AND MATERIAL=====		
43- STRUCTURE TYPE, MAIN :	STEEL TRUSS DECK	309
44- STRUCTURE TYPE APPR. :	NONE	000
45- NUMBER OF SPAN IN MAIN UNIT :	002	002
46- NUMBER OF APPROACH SPAN :	NONE	0000
107- DECK STRUCTURE TYPE :	STEEL PLATE	5
108- WEARING SURF./ PROTECT. SYSTEM :	OTHER - NONE - NONE	900
=====AGE AND SERVICE=====		
19- BYPASS, DETOUR LENGTH (NEAREST MILE) :	STRUCTURE OVER HIGHWAY	00
27- YEAR BUILT :	1968	1968
28- LANES ON AND UNDER STRUCTURE :	0 LANES ON / 8 LANES UNDER	0008
29- A.D.T. OF INVENTORY ROUTE :	95,400	095400
30- YEAR OF A.D.T. :	1970	90
42- TYPE OF SERVICE :	PEDESTRIAN OVER HIGHWAY	31
106- YEAR RECONSTRUCTION :	NO RECONSTRUCTION	0000
109- AVERAGE DAILY TRUCK TRAFFIC :	4 %	04
=====GEOMETRIC DATA=====		
10- INV. ROUTE, MIN. CLEAR. (0.01m) :	5.21 MT (17' - 1")	1701
32- APPROACH ROADWAY WIDTH :	29.29 MT (96.07 FT)	096
33- BRIDGE MEDIAN :	NONE	0
34- SKEW ANGLE :	NONE	00
35- STRUCTURE FLARED :	NO	0
47- INV. ROUTE, TOTAL HORIZ. CLEARANCE :	10.82 MT (35.49 FT)	354
48- LENGTH OF MAXIMUM SPAN :	21.65 MT (71.01 FT)	0071
49- STRUCTURE LENGTH :	55.63 MT (182.47 FT)	000183
50- CURB OR SIDEWALK WIDTHS :	NONE	000000
51- BRIDGE ROADWAY WIDTH, CURB TO CURB :	1.64 MT (5.38 FT)	0054
52- DECK WIDTH, OUT TO OUT :	2.84 MT (9.32 FT)	0093
53- MIN. VERT. CLEAR. OVER BRIDGE ROADWAY :	UNLIMITED	9999
54- MINIMUM VERTICAL UNDERCLEARANCE :	5.21 MT (17' - 1")	01701
55- MINIMUM LATERAL UNDERCLEARANCE ON RIGHT :	1.68 MT (5.51 FT)	0055
56- MINIMUM LATERAL UNDERCLEARANCE ON LEFT :	1.50 MT (4.92 FT)	049

PUERTO RICO HIGHWAY AUTHORITY
HIGHWAY SYSTEM ADMINISTRATION OFFICE

PAGE 3 OF 3

FEDERAL SYSTEM BRIDGE NO. : 01307 - ROAD NO. : FR 26 - KH. NO. : 004500

=====APPRAISALS=====

67- STRUCTURAL EVALUATION : 7
 STEEL PLATES, RAILINGS AND FLOOR BEAMS HAVE LIGHT CORROSION, MODERATE
 CORROSION IN THE STAIRS.

68- DECK GEOMETRY : N
 N/A.

69- UNDERCLEARANCE, VERTICAL & HORIZONTAL : 6
 EQUAL TO PRESENT MINIMUM CRITERIA.

71- WATERWAY ADEQUACY : N
 N/A.

72- APPROACH ROADWAY ALIGNMENT : N
 N/A.

36- TRAFFIC SAFETY FEATURES : N-N-N-N NNNN
 113- SCOUR CRITICAL BRIDGES : N/A N

=====ADDITIONAL COMMENTS=====

89- RESERVED --
 90- INSPECTION DATE : SEPTEMBER 24, 1993. 0993
 91- DESIGNATED INSPECTION FREQUENCY : 24 MONTHS 24
 92- CRITICAL FEATURE INSPECTION : N/A N N N
 95- CRITICAL FEAT. INSP. DATE : N/A
 105- RESERVED --
 117- SUFFICIENCY RATING :
 126- FIELD INSPECTION DATE : 092493 092493
 127- INSPECTION EVAL. DATE : 092493 092493
 130- CRITICAL FRACTURE INSP. DATE :
 131- EVALUATOR ENGINEER :

Javier Cruz Rojas

REMARKS:

THE REHABILITATION CONSIST IN REPAIR ALL DEFICIENCIES DESCRIBED ON THE
 ITEM #67.

BRIDGE STUDIES AND EVALUATION OFFICE
 PUERTO RICO HIGHWAY AUTHORITY

1307 0026 00430
 Bridge No. Road No. Km.No.

BRIDGE INVENTORY

IDENTIFICATION

Code Position

25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
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1. State Code _____ Commonwealth of Puerto Rico 7 2 1
2. State Highway Department District San Juan 0 1
3. County (Parish) Code San Juan 1 2 7
4. Place Code Urban 2 6 7 7 0
5. Inventory Route 2-1-1-00003-0 2 1 1 0 0 0 0 3 0
6. Features Intersected Pedestrian Walkway
P E D E S T R I A N W A L K W A Y
7. Facility Carried by Structure P.R. 26
P R 2 6
8. Structure Number 1307 10A1 0 0 0 0 0 0 0 0 0 0 0 1 3 0 7 1
9. Location Baldorioty De Castro Avenue
B A L D O R I O T Y D E C A S T R O A V E
10. Inventory Route, Minimum Vertical Clearance (0.01 m) 4.80 (15.74) 1 5 0 9
11. Milepoint 4.30 0 0 4 3 0 0
16. Latitude _____ 18 Degrees 27.3 Minutes 1 8 2 7 3
17. Longitude _____ 66 Degrees 03.1 Minutes 0 6 6 0 3 1
19. By Pass, Detour Length (Nearest mile) Structure over Highway 0 0

CLASSIFICATION

20. Toll Toll Free 3
21. Maintenance Responsibility D.T.P.W. 0 1
22. Owner State Highway Department 0 1
26. Functional Classification of Inventory Route Urban Interstate 1 1

6 5 4 3 2 1

AGE AND SERVICE

27. Year Built 1968 1 9 6 8

28. Lanes on the Structure 0 and Under the Structure 7 0 0 0 7

29. Average Daily Traffic 100,000 1 0 0 0 0 0

30. Year of Average Daily Traffic 1987 8 7

STRUCTURE DATA

31. Design Load Pedestrian 7

32. Approach Roadway Width 24.80m (81.34) 0 8 1

33. Bridge Median None Open Closed Closed NM 0

34. Skew None 0 0

35. Structure Flared Yes No 0

36. Traffic Safety Features N-N-N-N N N N N

37. Historical Significance 5

38. Navigation Control Yes No NA N

39. Navigation Vertical Clearance Yes No 0 0 0

40. Navigation Horizontal Clearance Yes No 0 0 0 0

41. Structure, Open, Posted, or Closed to Traffic Open A

42. Type of Service Pedestrian over Highway 3 1

43. Structure Type, Main Steel truss Deck 3 0 9

44. Structure Type, Approach Spans None 0 0 0

45. Number of Spans in Main Unit 2 0 0 2

46. Number of Approach Spans None 0 0 0 0

47. Inventory Route, Total Horizontal Clearance L=10.4m (34.1) R=14.4m (47.2) 3 8 1

48. Length of Maximum Span 20.46m (67.11) 0 0 6 7

49. Structure Length 430.2m (1411.1) 0 0 0 1 4 1

50. Curb or Sidewalk Widths None R N 0 0 0 0 0 0

51. Bridge Roadway Width, Curb-to-Curb 1.62m (5.31) 0 0 5 3

52. Deck Width, Out-to-Out 2.82m (9.25) 0 0 9 3

53. Minimum Vertical Clearance Over Bridge Roadway Unlimited 9 9 9 9

54. Minimum Vertical Underclearance H 4.8m (15.74) R N H 1 5 0 9

55. Minimum Lateral Underclearance on Right H 6.36m (44.6) R N H 0 4 5

56. Minimum Lateral Underclearance on Left 0.86m (2.82) 0 2 8

3	2	1
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CONDITIONS RATINGS

	Material	Condition	
58. Deck	Steel	Poor	4
59. Superstructure	Steel	Poor	4
60. Substructure	Steel	Fair	5
61. Channel and Channel Protection	N/A	—	N
62. Culverts	N/A	—	N
64. Operating Rating	Pedestrian		800
66. Inventory Rating	Pedestrian		800

APPRAISAL RATING

Deficiencies

67. Structural Evaluation	Deck is deteriorated, corrosion has perforated both ends of the structures. Deck does not meet safety criteria. Ponding is occurring at the full of the bridge when passing in capital has developed. Steel slab exhibits rust stain with severe corrosion. Spalling of concrete and severe cracking, although it is repaired. Spalling exhibited severe corrosion and wearing down of top and bottom. Upper and lower lateral (see notes)	4
68. Deck Geometry	N/A	N
69. Underclearances, Vertical and Horizontal	Condition better than present minimum criteria	7
70. Bridge Posting	No posting is required	5
71. Waterway Adequacy	N/A highway underpassing	N
72. Approach Roadway Alignment	N/A Pedestrian	N

15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
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PROPOSED IMPROVEMENTS

75. Type of Work Rehabilitation

351

76. Length of Structure Improvement 43.02 m = 141.11

000141

INSPECTIONS

89. (Reserved) n/a

1

90. Inspection Date 20/11/91

0291

91. Designated Inspection Frequency 24 Month

24

92. Critical Feature Inspection

A. Fracture Critical Details n/a

07

B. Underwater Inspection n/a

07

C. Other Special Inspection n/a

07

93. Critical Feature Inspection Date

A. Fracture Critical Details n/a

B. Underwater Inspection n/a

C. Other Special Inspection n/a

IMPROVEMENT COSTS

94. Bridge Improvement Cost \$5,000

000005

95. Roadway Improvement Cost

000000

96. Total Project Cost \$5,000

000000

97. Year of Improvement Cost Estimated 1991

91

CLASSIFICATION AND STRUCTURE DATA

98. Border Bridge n/a

99. Border Bridge Structure Number n/a

100. Defense Highway Designation no

1

101. Parallel Structure Designation n/a

0

102. Direction of Traffic Highway bridge not carry

0

103. Temporary Structure Designation n/a

0

104. Highway System of the Inventory Route Urban Intersect

1

105. (Reserved) n/a

1

6	5	4	3	2	1
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CLASSIFICATION AND STRUCTURE DATA (Cont.)

- 106. Year Reconstructed N/A
- 107. Deck Structure Type Steel Plate
- 108. Wearing Surface/Protective System N/A
- 109. Average Daily Truck Traffic 2100
- 110. Designated National Network Yes
- 111. Pier or Abutment Protection (for Navigation) N/A
- 112. NBIS Bridge Length N/A Pedestrian
- 113. Scour Critical Bridges N/A
- 114. Future Average Daily Traffic 130,000 est.
- 115. Year of Future Average Daily Traffic 2009
- 116. Minimum Navigation Vertical Clearance N/A
Vertical Lift Bridge N/A

0	0	0	0		
5					
N	N	0			
0	4				
1					
1					
2					
1	3	0	0	0	0
0	7				

Remarks:
 Rehabilitation consists in repairing all the deficiencies exposed on item #67.

BRIDGE STUDIES AND EVALUATION OFFICE
 PUERTO RICO HIGHWAY AUTHORITY

1307 0026 00450
 Bridge No. Road No. Km.No.

BRIDGE INVENTORY

IDENTIFICATION

Code Position

25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
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- 1. State Code Commonwealth of Puerto Rico 7 2 1
- 2. State Highway Department District San Juan 07
- 3. County (Parish) Code San Juan 127
- 4. Place Code San Juan Urban zone 76770
- 5. Inventory Route 2-1-1-09003-0 211000030
- 6. Features Intersected Pedestrian Walkway
P E D E S T R I A N W A L K W A Y
- 7. Facility Carried by Structure PR-26
P R - 2 6
- 8. Structure Number 1307 1 of 1 13071
- 9. Location Baldorioty de Castro Avenue
B A L D O R I O T Y D E C A S T R O A V E N U E
- 10. Inventory Route, Minimum Vertical Clearance (0.01 m) 5.21m (17.09') (17'-01") 1701
- 11. Milepoint 4.50 004500
- 16. Latitude 18 Degrees 27.3 Minutes 18273
- 17. Longitude 66 Degrees 03.1 Minutes 066031
- 19. By Pass, Detour Length (Nearest mile) Structure over highway 00

CLASSIFICATION

- 20. Toll On Free Road 0
- 21. Maintenance Responsibility H. J. P. W. 01
- 22. Owner H. J. P. W. 01
- 26. Functional Classification of Inventory Route Urban Interstate 11

6	5	4	3	2	1
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AGE AND SERVICE

27. Year Built 1968 1968

28. Lanes on the Structure 0 and Under the Structure 7 0007

29. Average Daily Traffic 95,400 095400

30. Year of Average Daily Traffic 1990 90

STRUCTURE DATA

31. Design Load Pedestrian 7

32. Approach Roadway Width 29.29m (96.07') 096

33. Bridge Median None Open Closed Closed NM 0

34. Skew None 00

35. Structure Flared Yes No 0

36. Traffic Safety Features None N N N N

37. Historical Significance None E

38. Navigation Control Yes No NA N

39. Navigation Vertical Clearance Yes No 000

40. Navigation Horizontal Clearance Yes No 0000

41. Structure, Open, Posted, or Closed to Traffic open A

42. Type of Service Pedestrian over highway 31

43. Structure Type, Main Steel Truss deck 309

44. Structure Type, Approach Spans None 000

45. Number of Spans in Main Unit 7 002

46. Number of Approach Spans None 0000

47. Inventory Route, Total Horizontal Clearance L=10.82m(35.49') R=10.80m(35.42') 354

48. Length of Maximum Span 21.65m (71.01') 0071

49. Structure Length 55.63m (182.47') 000183

50. Curb or Sidewalk Widths None L= R= 000000

51. Bridge Roadway Width, Curb-to-Curb 1.64m (5.38') 0054

52. Deck Width, Out-to-Out 2.84m (9.32') 0093

53. Minimum Vertical Clearance Over Bridge Roadway Unlimited 9999

54. Minimum Vertical Underclearance H=21m(1709) R N #1701

55. Minimum Lateral Underclearance on Right H=1.68m(5.51') R N #055

56. Minimum Lateral Underclearance on Left 1.50m (4.92') 049

CONDITIONS RATINGS

	Material	Condition	
58. Deck	<i>Steel</i>	<i>Good</i>	<input checked="" type="checkbox"/>
59. Superstructure	<i>Steel</i>	<i>Good</i>	<input checked="" type="checkbox"/>
60. Substructure	<i>Steel</i>	<i>Good</i>	<input checked="" type="checkbox"/>
61. Channel and Channel Protection	<i>OK</i>	<i>OK</i>	<input checked="" type="checkbox"/>
62. Culverts	<i>OK</i>	<i>OK</i>	<input checked="" type="checkbox"/>
64. Operating Rating	<i>OK</i>	<i>OK</i>	<input type="checkbox"/> 800
66. Inventory Rating	<i>OK</i>	<i>OK</i>	<input type="checkbox"/> 800

APPRAISAL RATING

	Deficiencies	
67. Structural Evaluation	<i>Joints are leaking. Some members of I beams show corrosion. Condition better than present minimum criteria.</i>	<input checked="" type="checkbox"/>
68. Deck Geometry	<i>OK</i>	<input checked="" type="checkbox"/>
69. Underclearances, Vertical and Horizontal	<i>17'-07" (V.U.); 5'-51" (L.U.R.). Basically intolerable, requiring high priority of corrective action.</i>	<input type="checkbox"/> 3
70. Bridge Posting	<i>OK</i>	<input checked="" type="checkbox"/>
71. Waterway Adequacy	<i>OK</i>	<input checked="" type="checkbox"/>
72. Approach Roadway Alignment	<i>OK</i>	<input checked="" type="checkbox"/>

15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
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PROPOSED IMPROVEMENTS

75. Type of Work Rehabilitation 362
 76. Length of Structure Improvement 55.63m (182.47') 000183

INSPECTIONS

89. (Reserved)
 90. Inspection Date December 13, 1991 1291
 91. Designated Inspection Frequency every 24 months 79
 92. Critical Feature Inspection Not Critical
 A. Fracture Critical Details N
 B. Underwater Inspection N
 C. Other Special Inspection N
 93. Critical Feature Inspection Date
 A. Fracture Critical Details
 B. Underwater Inspection
 C. Other Special Inspection

IMPROVEMENT COSTS

94. Bridge Improvement Cost — 000000
 95. Roadway Improvement Cost — 000000
 96. Total Project Cost 55.63 x 2.84 x 900 x .03 ≈ \$47,000 000004
 97. Year of Improvement Cost Estimated 1991 91

CLASSIFICATION AND STRUCTURE DATA

98. Border Bridge NO
 99. Border Bridge Structure Number 016
 100. Defense Highway Designation NO
 101. Parallel Structure Designation NO
 102. Direction of Traffic Highway Traffic not carried
 103. Temporary Structure Designation NO
 104. Highway System of the Inventory Route Urban Interstates (NAS)
 105. (Reserved) —

6	5	4	3	2	1
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CLASSIFICATION AND STRUCTURE DATA (Cont.)

106. Year Reconstructed	<u>not reconstructed</u>	<input type="text" value="0"/>	<input type="text" value="0"/>	<input type="text" value="0"/>	<input type="text" value="0"/>		
107. Deck Structure Type	<u>Steel Plate</u>	<input type="text" value="5"/>					
108. Wearing Surface/Protective System	<u>Other - none - none</u>	<input type="text" value="9"/>	<input type="text" value="0"/>	<input type="text" value="0"/>			
109. Average Daily Truck Traffic	<u>4% (not)</u>	<input type="text" value="6"/>	<input type="text" value="4"/>				
110. Designated National Network	<u>yes</u>	<input type="text" value="1"/>					
111. Pier or Abutment Protection (for Navigation)	<u>N/A</u>	<input type="text" value="-"/>					
112. NBIS Bridge Length	<u>40</u>	<input type="text" value="4"/>					
113. Scour Critical Bridges	<u>N/A</u>	<input type="text" value="N"/>					
114. Future Average Daily Traffic	<u>143,990</u>	<input type="text" value="1"/>	<input type="text" value="4"/>	<input type="text" value="3"/>	<input type="text" value="9"/>	<input type="text" value="9"/>	<input type="text" value="0"/>
115. Year of Future Average Daily Traffic	<u>2010</u>	<input type="text" value="1"/>	<input type="text" value="0"/>				
116. Minimum Navigation Vertical Clearance	<u>N/A</u>	<input type="text" value="0"/>	<input type="text" value="0"/>	<input type="text" value="0"/>			
Vertical Lift Bridge							

Remarks: Rehabilitation consists on repairing all deficiencies discussed on item 67. This pedestrian walkway bridge was relocated to Km. 4.50 and painted.

COMMONWEALTH OF PUERTO RICO
HIGHWAY AUTHORITY
BRIDGE STUDIES AND EVALUATION OFFICE

APPRAISAL OF ITEM 68

BRIDGE NO. 1307

ROAD NO. 26

KM. NO. 4.50

Year	ADT	Sec.	T W	T W Adequate or Inadequate	T W Inadequate By	TW + SH	TW + SH Adequate or Inadequate	TW + SH Inadequate By
1990		<input checked="" type="checkbox"/>	Actual =	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT =					
			<i>Not Applicable</i>					
2010			Reqd. For ADT =					
			Rec. PRHA =					

✓ Adequate
x Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE
 PUERTO RICO HIGHWAY AUTHORITY

GENERAL EVALUATION

This bridge is basically in 3 structural condition

excellent	4	fair	2	very poor	0
adequate	3	poor	1		

The load carrying capacity is 0

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
----------	---	--------------------	---	------------	---	----------------------------	---

The recommended modifications will make it 1

adequate	2	minimally adequate	1	no required (don't print)	0
				0 when previous is	2

The bridge should be replaced.

Yes (print)	1	not required (don't print)	0
-------------	---	----------------------------	---

It requires 2 @ 24

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months (A) (B)

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 3

routine maintenance	3
minor repairs and routine maintenance	2
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	1
emergency repairs to eliminate danger to the public and routine maintenance	0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (2010) ADT, the travelled way is 0

For PRHA (2010) recommendations, the travelled way is 0

For present ADT, the shoulders are 0

The future (2010) ADT, the shoulders are 0

For PRHA (2010) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>
Travelled way			Shoulders			

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print)					0

The (A) approach alignment is (B)

(A) = vertical 1 horizontal 0 horizontal and vertical 2 (A) (B)

(B) = satisfactory 3 minimally tolerable 2 poor and represents a danger to the public 1

not required (don't print) 0

Load Post for inventory rating shown in Item 64

Yes	1	No (don't print)	0
-----	---	------------------	---

Post for vertical underclearance shown in Items 54 and 69

Yes	1	No (don't print)	0
-----	---	------------------	---

FOR "REMARKS" SEE BRIDGE FILE.

PERSONNEL
 Structural Inspection John Perez By John Perez Date: 12/21/91
 Review of Field Data _____
 Transfers of Data _____
 Condition Analysis _____
 Appraisal John Perez 12/21/91

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM-370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

1307 0026
 No. Bridge No. Road No.

BRIDGE INVENTORY

Km. No. 4.30

IDENTIFICATION

Code Positions

25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
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1. State Commonwealth of Puerto Rico ¹⁰ 7 2 1

2. Highway District San Juan ¹³ 0 7

3. Municipality San Juan ¹⁵ 0 6 3

4. City/Town San Juan ¹⁸ 0 0 2 3

5. Inventory Route Est. 1 - 00003 - 0 ²² 2 1 1 0 0 0 0 3 0

6. Features Intersected Pedestrian Walkway ³¹ P E D E S T R I A N W A L K W A Y

7. Facility Carried by Structure ⁵⁸ PR-26 ⁷⁴ 0 2 0 2 7

8. Structure No. 1307 ⁷⁴ 0 2 0 2 7

9. Location ¹⁰ Baldoirio De Castro Avenue

10. Inventory Route, Minimum Vertical Clearance (0.01 m) 4.80 ¹⁰ 0 4 8 0

11. Kilometer Point (0.01 km) 4.30 ¹⁴ 0 0 4 3 0

12. Road Section Number (DOD) 25 ¹⁵ 0 0 2 5

13. Bridge Description DH NSTS PS ²⁴ 1 5

14. Defense Milepoint (0.01 ml) 6.37 ²⁶ 0 6 3 7

15. Defense Section Length (miles) (0.1 mile) 8.10 ³⁰ 0 8 1 0

16. Latitude: 18 Degrees 27.3 Minutes ³³ 1 8 2 7 3

17. Longitude: 66 Degrees 03.1 Minutes ³⁸ 0 6 6 0 3 1

18. Physical Vulnerability Steel Truss ⁴⁴ 7

19. By Pass, Detour Length (Nearest mile) Structure over highway ⁴⁵ 0 0

20. Toll Toll Free ⁴⁷ 3

21. Custodian D. T. P.W. ⁴⁸ 7

22. Owner State Highway Department ⁴⁹ 1

23. F.A.P. No. ⁶⁰ UNKNOWN

CLASSIFICATION

24. Fed. Aid System Unknown Interstate ¹⁰ 0 2

25. Administrative State ¹² 7

26. Functional Unknown Interstate ¹³ 4 7

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 18 6000

28. Lanes on Str. 0 Under 7 19 0007

29. ADT - Inventory Route 100,000 (estimated) 23 100000

30. Year 1985 29 85

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) 26.81 33 2681

33. Median None Open Closed 35 0

34. Skew none 36 00

35. Structure Flared Yes No 38 0

36. Traffic Safety Features 39 11111

37. Historical Significance Yes No 43 5

38. Navigation Control Yes No 45 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 000

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 0000

41. Structure, Open or Closed to Traffic Open 54 1

42. Type Service Pedestrian over highway 55 37

43. Structure Type-Main Steel truss-deck 57 307

44. Structure Type-Approach Spans None 61 000

45. No. of Spans-Main 2 63 002

46. No. of Spans-Approaches None 65 0000

47. Total Horizontal Clearance (0.01 m) L = 20.46m R = 19.16m 67 222

48. Max. Span Length (0.01 m) 20.46m center of piers to pier of abut 70 2205

49. Structure Length (0.01 m) 43.02m out to out of abut 73 22230

50. Sidewalk Widths (0.01 m) Left none Right none 76 222000

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.62m 78 0072

52. Deck Width (out-out) (0.01 m) 2.82m 80 0028

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) Unlimited 83 2222

54. Vertical Underclearance - Minimum (0.01 m) 4.80m 85 2280

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 5.21m R = 7.90m 88 252

56. Lateral Underclearance on Left (Inner) Sides - Minimum (0.01 m) L = 0.85m R = 0.80m 90 007

57. Wearing Surface Steel 92 7

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION

9 8 7 6 5 4 3 2 1

	Material	Condition Analysis	
58. Deck	Steel	Fair condition	99 <input checked="" type="checkbox"/>
59. Superstructure	Steel	Fair condition	60 <input checked="" type="checkbox"/>
60. Substructure	Steel	Generally good condition	61 <input checked="" type="checkbox"/>
61. Channel & Channel Protection	N/A	N/A	62 <input type="checkbox"/>
62. Culvert & Retaining Walls	N/A	N/A	63 <input checked="" type="checkbox"/>
63. Estimated Remaining Life		10 YEARS	64 <input checked="" type="checkbox"/>
64. Operating Rating		Pedestrian	65 <input checked="" type="checkbox"/>
65. Approach Alignment		N/A	66 <input checked="" type="checkbox"/>
66. Inventory Rating		Pedestrian	70 <input checked="" type="checkbox"/>

APPRAISAL

	Deficiencies	
67. Structural Condition	Deck with moderate corrosion generally at corners. Abutments and piles with fine cracks and small spallings	10 <input checked="" type="checkbox"/>
68. Deck Geometry	N/A	11 <input checked="" type="checkbox"/>
69. Underclearances-Vert. & Lateral (4.80m)	Condition better than present minimum criteria	12 <input checked="" type="checkbox"/>
70. Safe Load Capacity	N/A	13 <input checked="" type="checkbox"/>
71. Waterway Adequacy	N/A	14 <input checked="" type="checkbox"/>
72. Approach Alignment	N/A	15 <input checked="" type="checkbox"/>

PROPOSED IMPROVEMENTS

73. Year Needed	1988	16 <input checked="" type="checkbox"/>
Completed		
Described		
74. Type of Service	Pedestrian	18 <input checked="" type="checkbox"/>
75. Type of Work	Rehabilitation	19 <input checked="" type="checkbox"/>
76. Improvement Length (0.1 m)	N/A	22 <input checked="" type="checkbox"/>
77. Design Loading	N/A	28 <input checked="" type="checkbox"/>

BRIDGE STUDIES AND EVALUATION OFFICE

9 8 7 6 5 4 3 2 1

- 78. Roadway Width (ft)..... *N/A* "0000"
- 79. Number of Lanes..... *N/A* "00"
- 80. ADT..... *130,080* "130080"
- 81. Year of Estimated ADT..... *1995* "95"
- 82. Year of Proposed Adjacent Roadway Improvements..... *N/A* "00"
- 83. Prop. Adj. Rdwy Improvements-Type..... *N/A* "0"

COST OF IMPROVEMENTS

- 84 Total (dollars)\$..... *\$ 5,000* "00005"
- Estimated Design Time (months)..... *~*

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars)..... *N/A* "000"
- 86. Demolition Cost (Dollars)..... *N/A* "000"
- 87. Substructure (Dollars)..... *N/A* "00000"
- 88. Superstructure (Dollars)..... *N/A* "00000"
- 89. Blank..... *~* "0"
- 90. Date of Last Inspection..... *Dec. 2, 1988* "120888"
- 91. Rehabilitate Existing Structure (Dollars)..... *\$ 5,000* "005"
- 92. Detour and Traffic Maintenance (Dollars)..... *N/A* "000"
- 93. Approaches (Dollars)..... *~* "000"
- Approach Embankment (Dollars)..... *~*
- Approach Pavement (Dollars)..... *~*
- Approach Guardrail (Dollars)..... *~*
- *(Code to nearest thousand dollars)..... *~*

REMARKS

Rehabilitation consists in repair all deficiencies described on item # 67.

BRIDGE STUDIES AND EVALUATION OFFICE
GENERAL EVALUATION

This bridge is basically in 2 structural condition 16

excellent 4	fair 2	very poor 0
good 3	poor 1	

The load carrying capacity is 17

adequate 3	minimally adequate 2	inadequate 1	not required (don't print) 0
------------	----------------------	--------------	------------------------------

The recommended modifications will make it 18

adequate 2	minimally adequate 1	not required (don't print) 0
		0 when previous is 2

The bridge should be replaced. 19

Yes (print) 1	Not required (don't print) 0
---------------	------------------------------

It requires 2 2 2 4 20

routine inspection every 24 months (A) = 2 | (B) = 24

frequent inspection every (B) months to (A) (B)

monitor abnormal and/or suspected deficiencies (A) = 1 | (B) = months

frequent inspection every (B) months to determine (A) = 0 | (B) = months

the cause of and remedies for existing defects

It requires 2 21

routine maintenance 3

minor repairs and routine maintenance 2

urgent repairs to prevent further costly deterioration and/or the development 1

of a dangerous condition and routine maintenance 0

emergency repairs to eliminate danger to the public and routine maintenance

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1995) ADT, the travelled way is 0

For PRHA (1995) recommendations, the travelled way is 0 24

For present ADT, the shoulders are 0

For future (1995) ADT, the shoulders are 0

For PRHA (1995) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
0	0	0	0	0	0	0
Travelled way			Shoulders			

satisfactory 3	unsatisfactory 2	cannot be evaluated because of lack of information 1
not required (don't print) 0		

The (A) approach alignment is (B) 21

<u>(A)</u> vertical 1	horizontal 0	horizontal and vertical 2
<u>(B)</u> satisfactory 3	minimally tolerable 2	poor and represents a danger to the public 1
not required (don't print) 0		

Load Post for inventory rating shown in Item 64 23

Yes 1	No (don't print) 0
-------	--------------------

Post for vertical underclearance shown in Items 54 & 69 24

Yes 1	No (don't print) 0
-------	--------------------

For "Remarks" see bridge file.

PERSONNEL

Structural Inspection	By <u>J. McQuinn</u> Date <u>11-17-88</u>	Condition Analysis	By <u>J. Forbio</u> Date <u>12-08-88</u>
Review of Field Data	By <u>R. Meador</u> Date <u>11-18-88</u>	Appraisal	
Transfer of Data		IBM MT/SC file	
		IBM-370 file	

BRIDGE STUDIES AND EVALUATION OFFICE

Page 1 of 5

PUERTO RICO HIGHWAY AUTHORITY
 IBM-370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

1307 0020
 Bridge No. Road No.
 Km. No. 4.30

BRIDGE INVENTORY

IDENTIFICATION

		Code Positions	
		25	24
1. State	Commonwealth of Puerto Rico	10	721
2. Highway District	San Juan	13	01
3. Municipality	San Juan	15	013
4. City/Town	San Juan	18	0013
5. Inventory Route	2-1-1-0003-0	22	211000030
6. Features Intersected	Pedestrian Walkway	31	PEDESTRIAN WALKWAY
7. Facility Carried by Structure ³⁶	PR-26		
8. Structure No.	1307	74	013071
9. Location ¹⁰	Baldorioty de Castro Avenue		
10. Inventory Route, Minimum Vertical Clearance (0.01 m)	4.80 m	10	0480
11. Kilometer Point (0.01 km)	4.30	14	00430
12. Road Section Number (DOD)	25	19	0025-
13. Bridge Description	WH NSTS PS	24	-
14. Defense Milepoint (0.01 ml)	6.37	28	0637
15. Defense Section Length (miles) (0.1 mile)	8.10	30	081
16. Latitude: 18 Degrees 27.3 Minutes		33	18273
17. Longitude: 66 Degrees 03.1 Minutes		38	066031
18. Physical Vulnerability	Steel truss	44	4
19. By Pass, Detour Length (Nearest mile)	Structure over highway	45	00
20. Toll	Toll Free	47	3
21. Custodian	H.T. P.W.	48	1
22. Owner	State Highway Department	49	1
23. F.A.P. No. ⁵⁰	Unknown		

CLASSIFICATION

24. Fed. Aid System	Interstate, urban	10	02
25. Administrative	State	12	1
26. Functional	Interstate	13	41

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 15 6 8 0 0

28. Lanes on Str. 0 Under 7 16 0 0 0 7

29. ADT - Inventory Route 100,000 (Estimated) 23 1 0 0 0 0 0

30. Year 1985 28 0 5

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) 26.81 m 32 2 0 8

33. Median None Open Closed 35 0

34. Skew none 36 0 0

35. Structure Flared Yes No 38 0

36. Traffic Safety Features N M M M 39 N M M M

37. Historical Significance 43 5

38. Navigation Control Yes No 48 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 0 0 0

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 0 0 0 0

41. Structure, Open or Closed to Traffic open 54 A

42. Type Service Pedestrian over highway 56 3 1

43. Structure Type-Main Steel truss-deck 57 3 0 9

44. Structure Type-Approach Spans none 58 0 0 0

45. No. of Spans-Main 2 10 0 0 2

46. No. of Spans-Approaches none 13 0 0 0 0

47. Total Horizontal Clearance (0.01 m) L = 20.40 m R = 19.16 m 17 7 9 2

48. Max. Span Length (0.01 m) 20.40 m 20 0 2 0 5

49. Structure Length (0.01 m) 43.02 m 24 0 0 0 4 3 0

50. Sidewalk Widths (0.01 m) Left Right 30 0 0 0 0 0 0

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.62 m 38 0 0 1 6

52. Deck Width (out-out) (0.01 m) 2.82 m 40 0 0 2 8

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) unlimited 44 9 9 9 9

54. Vertical Underclearance - Minimum (0.01 m) 4.80 m 48 0 4 8 0

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 5.21 m R = 7.90 m 52 0 5 2

56. Lateral Underclearance on Left (Inner) Sides - Minimum (0.01 m) L = 0.85 m R = 0.86 m 55 0 0 9

57. Wearing Surface steel 58 7

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION

9 8 7 6 5 4 3 2 1

	Material	Condition Analysis	
58. Deck	steel	Generally Good Condition	59 <input type="checkbox"/>
59. Superstructure	steel	Generally Good Condition	60 <input type="checkbox"/>
60. Substructure	steel	Generally Good Condition	61 <input type="checkbox"/>
61. Channel & Channel Protection	N.A.	N.A.	62 <input type="checkbox"/>
62. Culvert & Retaining Walls	N.A.	N.A.	63 <input type="checkbox"/>
63. Estimated Remaining Life		15 years	64 <input type="checkbox"/>
64. Operating Rating		Pedestrian	65 <input type="checkbox"/>
65. Approach Alignment		N.A.	66 <input type="checkbox"/>
66. Inventory Rating		Pedestrian	70 <input type="checkbox"/>

APPRAISAL

	Deficiencies	
67. Structural Condition	all the structure has been repaired and painted actually	10 <input type="checkbox"/>
68. Deck Geometry	N.A.	11 <input type="checkbox"/>
69. Underclearances-Vert. & Lateral	(4.80m) Condition better than present -minimum criteria	12 <input type="checkbox"/>
70. Safe Load Capacity	N.A.	13 <input type="checkbox"/>
71. Waterway Adequacy	N.A.	14 <input type="checkbox"/>
72. Approach Alignment	N.A.	15 <input type="checkbox"/>

PROPOSED IMPROVEMENTS

73. Year Needed	Improvement not needed	16 <input type="checkbox"/>
Completed		
Described		
74. Type of Service	Pedestrian	18 <input type="checkbox"/>
75. Type of Work		19 <input type="checkbox"/>
76. Improvement Length (0.1 m)		22 <input type="checkbox"/>
77. Design Loading	N.A.	28 <input type="checkbox"/>

BRIDGE STUDIES AND EVALUATION OFFICE

[9][8] 7[6] 5[4] 3[7] 1[1]

- 78. Roadway Width (0.01m)..... *NA* "0[0]0[0]
- 79. Number of Lanes..... *NA* "6[0]
- 80. ADT..... *130,080* "7[3]0[0]8[0]
- 81. Year of Estimated ADT..... *1995* "9[5]
- 82. Year of Proposed Adjacent Roadway Improvements..... *NA* "0[0]
- 83. Prop. Adj. Rdwy Improvements-Type..... *NA* "0

COST OF IMPROVEMENTS

- 84. Total (dollars)\$ — "0[0]0[0]0[0]
- Estimated Design Time (months)

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars)..... — "0[0]0[0]
- 86. Demolition Cost (Dollars)..... — "0[0]0[0]
- 87. Substructure (Dollars)..... — "0[0]0[0]0[0]
- 88. Superstructure (Dollars)..... — "0[0]0[0]0[0]
- 89. Blank..... — "—
- 90. Date of Last Inspection..... *Nov. 12, 1996* "1[1]1[2]8[6]
- 91. Rehabilitate Existing Structure (Dollars)..... — "0[0]0[0]
- 92. Detour and Traffic Maintenance (Dollars)..... — "0[0]0[0]
- 93. Approaches (Dollars)..... — "0[0]0[0]
- Approach Embankment (Dollars).....
- Approach Pavement (Dollars).....
- Approach Guardrail (Dollars).....
- *(Code to nearest thousand dollars).....

REMARKS

This bridge is in good structural conditions.

BRIDGE STUDIES AND EVALUATION OFFICE
GENERAL EVALUATION

This bridge is basically in 3 structural condition 16

excellent	4	fair	2	very poor	0
good	3	poor	1		

The load carrying capacity is 0 17

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
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The recommended modifications will make it 0 18

adequate	2	minimally adequate	1	not required (don't print)	0
				0 when previous is	2

The bridge should be replaced. 19

Yes (print)	1	Not required (don't print)	0
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It requires 20

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 23

routine maintenance	3
minor repairs and routine maintenance	2
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	1
emergency repairs to eliminate danger to the public and routine maintenance	0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1995) ADT, the travelled way is 0

For PRHA (1995) recommendations, the travelled way is 0 24

Deck	Present	Future	PRHA	Present	Future	PRHA
Travelled way	0	0	0	0	0	0
Shoulders						

For present ADT, the shoulders are 0

For future (1995) ADT, the shoulders are 0

For PRHA (1995) recommendations, the shoulders are 0

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print)		0			

The (A) approach alignment is (B) 31

<u>(A)</u> vertical	1	horizontal	0	horizontal and vertical	2
<u>(B)</u> satisfactory	3	minimally tolerable	2	poor and represents a danger to the public	1
not required (don't print)		0			

Load Post for inventory rating shown in Item 64 33

Yes	1	No (don't print)	0
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Post for vertical underclearance shown in Items 54 & 69 34

Yes	1	No (don't print)	0
-----	---	------------------	---

For "Remarks" see bridge file.

PERSONNEL	<i>Reece</i> ^{BY} → ^{Date} <i>10/23/86</i>	Condition Analysis	<i>A. Carver</i> ^{Date} <i>11/12/86</i>
Structural Inspection	_____	Appraisal	_____
Review of Field Data	_____	IBM MT/SC file	_____
Transfer of Data	_____	IBM - 370 file	_____

BRIDGE NO. 1307

ROAD NO. 26 KM. 4.30

APPRAISAL OF ITEM 00

BRIDGE STUDIES AND
EVALUATION OFFICE

YEAR	ADT	Sec.	T W	TW	TW	TW + SH	TW + SH	TW + SH
				Adequate or Inadequate	Inadequate By		Adequate	Inadequate
1985	NA	X	Actual =	X	X		X	X
			For ADT =					
	NA							
1995	NA		Reqd. For ADT =					
			Rec. PRHA =					

✓ Adequate
X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM-370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

1307 0026
 Card No. Bridge No. Road No.

BRIDGE INVENTORY

Km. No. 4.27

IDENTIFICATION

Code Positions

25 24 23 22 21 20 19 18 17 16 15 14 13 12 11 10 9 8 7 6 5 4 3 2 1

- 1. State Commonwealth of Puerto Rico ¹⁰ 721
- 2. Highway District *San Juan* ¹⁵ 01
- 3. Municipality *San Juan* ¹⁵ 063
- 4. City/Town *San Juan* ¹⁸ 0063
- 5. Inventory Route *Z-1-1-00003-0* ²² 2110000510
- 6. Features Intersected *Pedestrian Walkway* ³ P E D E S T R I A N W A L K W A Y
- 7. Facility Carried by Structure ⁵⁶ *P.R. 26*
- 8. Structure No. *1307* *141* ⁷⁴ 073071
- 9. Location ¹⁰ *Baldorioty De Castro Avenue*
- 10. Inventory Route, Minimum Vertical Clearance (0.01 m) *4.80 m* ¹⁰ 0480
- 11. Kilometer Point (0.01 km) *4.27* ¹⁴ 00427
- 12. Road Section Number (DOD) *25* ¹⁵ 0025-
- 13. Bridge Description *DH* *NST'S* ²⁴ -1
- 14. Defense Milepoint (0.01 ml) *6.37* ²⁶ 0637
- 15. Defense Section Length (miles) (0.1 mile) *8.1* ³⁰ 081
- 16. Latitude: *18* Degrees *27.3* Minutes ³³ 18273
- 17. Longitude: *66* Degrees *03.1* Minutes ³⁶ 066031
- 18. Physical Vulnerability *Steel truss* ⁴⁴ 4
- 19. By Pass, Detour Length (Nearest mile) *Structure over highway* ⁴⁵ 00
- 20. Toll *Toll Free* ⁴⁷ 3
- 21. Custodian *D.T.P.W.* ⁴⁸ 1
- 22. Owner *State Highway Department* ⁴⁹ 1
- 23. F.A.P. No. ⁵⁰ *Unknown*

CLASSIFICATION

- 24. Fed. Aid System *Interstate, urban* ¹⁰ 02
- 25. Administrative *State* ¹² 7
- 26. Functional *Interstate* ¹³ 47

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 18 6 8 0 0

28. Lanes on Str. 0 Under 7 19 0 0 0 7

29. ADT - Inventory Route 67,600 23 0 6 7 6 0 0

30. Year 1979 28 7 9

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) 26.81m 32 2 6 8 1

33. Median None Open Closed 35 0

34. Skew None 36 0 0

35. Structure Flared Yes No 38 0

36. Traffic Safety Features 39

37. Historical Significance 43 5

38. Navigation Control Yes No 48 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 0 0 0

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 0 0 0 0

41. Structure, Open or Closed to Traffic Open 64 1

42. Type Service Pedestrian over highway 68 3 1

43. Structure Type-Main Steel truss - deck 87 3 1 0 9

44. Structure Type-Approach Spans None 90 0 0 0 0

45. No. of Spans-Main 2 10 0 0 0 2

46. No. of Spans-Approaches None 13 0 0 0 0

47. Total Horizontal Clearance (0.01 m) L = 20.46m R = 19.16m 17 7 9 7 2

48. Max. Span Length (0.01 m) 20.46m 20 0 2 0 5

49. Structure Length (0.01 m) 43.02m 24 0 0 0 2 3 0

50. Sidewalk Widths (0.01 m) Left Right 80 0 0 0 0 0 0

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.42m 38 0 0 1 6

52. Deck Width (out-out) (0.01 m) 2.82m 40 0 0 2 8

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) Unlimited 44 9 9 9 9

54. Vertical Underclearance - Minimum (0.01 m) 4.80m 48 0 4 8 0

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 5.2m R = 7.90m 58 0 5 2

56. Lateral Underclearance on Left (Inner) Sides - Minimum (0.01 m) L = 0.55m R = 0.86m 68 0 0 9

57. Wearing Surface Steel 88 7

BRIDGE STUDIES AND EVALUATION OFFICE

CONDITION

9	8	7	6	5	4	3	2	1
---	---	---	---	---	---	---	---	---

CONDITION	Material	Condition Analysis	
58. Deck	Steel	Generally Good Condition	59 <input checked="" type="checkbox"/>
59. Superstructure	Steel	Generally Good Condition	60 <input checked="" type="checkbox"/>
60. Substructure	Steel	Generally Good Condition	61 <input checked="" type="checkbox"/>
61. Channel & Channel Protection	NA		62 <input checked="" type="checkbox"/>
62. Culvert & Retaining Walls	NA		63 <input checked="" type="checkbox"/>
63. Estimated Remaining Life		15	64 <input checked="" type="checkbox"/> 15
64. Operating Rating		Pedestrian	66 <input type="checkbox"/> 8 <input type="checkbox"/> 0 <input type="checkbox"/> 0
65. Approach Alignment		NA	69 <input checked="" type="checkbox"/>
66. Inventory Rating		Pedestrian	70 <input type="checkbox"/> 8 <input type="checkbox"/> 0 <input type="checkbox"/> 0

APPRAISAL

	Deficiencies	
67. Structural Condition	Sandblasting and paint have been applied recently.	10 <input checked="" type="checkbox"/>
68. Deck Geometry	NA	11 <input checked="" type="checkbox"/>
69. Underclearances-Vert. & Lateral	Condition equal to present minimum criteria.	12 <input checked="" type="checkbox"/>
70. Safe Load Capacity		13 <input checked="" type="checkbox"/>
71. Waterway Adequacy		14 <input checked="" type="checkbox"/>
72. Approach Alignment		15 <input checked="" type="checkbox"/>

PROPOSED IMPROVEMENTS

73. Year Needed		16 <input type="checkbox"/> 0 <input type="checkbox"/> 0
Completed		
Describe		
74. Type of Service	Pedestrian	18 <input type="checkbox"/>
75. Type of Work	NA	19 <input type="checkbox"/> 0 <input type="checkbox"/> 0 <input type="checkbox"/> 0
76. Improvement Length (0.1 m)	NA	22 <input type="checkbox"/> 0 <input type="checkbox"/> 0 <input type="checkbox"/> 0 <input type="checkbox"/> 0 <input type="checkbox"/> 0
77. Design Loading		28 <input type="checkbox"/>

BRIDGE STUDIES AND EVALUATION OFFICE

9 8 7 6 5 4 3 2 1

- 78. Roadway Width (0.01m)..... *NA*²⁸ 0 0 0
- 79. Number of Lanes..... *NA*³³ 0 0
- 80. ADT..... *130,080*³⁵ 1 3 0 0 8 0
- 81. Year of Estimated ADT..... *1995*⁴¹ 9 5
- 82. Year of Proposed Adjacent Roadway Improvements..... *NA*⁴³ 0 0
- 83. Prop. Adj. Rdwy Improvements-Type..... *NA*⁴⁵ 0

COST OF IMPROVEMENTS

- 84 Total (dollars)\$⁴⁸ 0 0 0 0 0 *
- Estimated Design Time (months)

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars).....⁵¹ 0 0 0 *
- 86. Demolition Cost (Dollars)⁵⁴ 0 0 0 *
- 87. Substructure (Dollars).....⁵⁷ 0 0 0 0 0 *
- 88. Superstructure (Dollars).....⁶² 0 0 0 0 0 *
- 89. Blank.....⁶⁷ - *
- 90. Date of Last Inspection..... *June 13, 1983*⁶⁸ 0 6 / 3 8 3 *
- 91. Rehabilitate Existing Structure (Dollars).....⁷⁴ 0 0 0 *
- 92. Detour and Traffic Maintenance (Dollars).....⁷⁷ 0 0 0 *
- 93. Approaches (Dollars).....⁸⁰ 0 0 0 *
- Approach Embankment (Dollars).....
- Approach Pavement (Dollars).....
- Approach Guardrail (Dollars).....
- *(Code to nearest thousand dollars).....

REMARKS

This bridge is in good structural condition.

.....

.....

.....

.....

.....

**BRIDGE STUDIES AND EVALUATION OFFICE
GENERAL EVALUATION**

This bridge is basically in 3 structural condition.

16

excellent	4	fair	2	very poor	0
good	3	poor	1		

The load carrying capacity is 0

17

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
----------	---	--------------------	---	------------	---	----------------------------	---

The recommended modifications will make it 0

18

adequate	2	minimally adequate	1	not required (don't print)	0
				0 when previous is	2

The bridge should be replaced.

19

Yes (print)	1	Not required (don't print)	0
-------------	---	----------------------------	---

It requires 2 or 24

20

routine inspection every 24 months (A) = 2 (B) = 24

frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies (A) = 1 (B) = months

frequent inspection every (B) months to determine the cause of and remedies for existing defects (A) = 0 (B) = months

It requires 3

23

routine maintenance	3
minor repairs and routine maintenance	2
urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance	1
emergency repairs to eliminate danger to the public and routine maintenance	0

The deck geometry is 0

For present ADT, the travelled way is 0

For future (1995) ADT, the travelled way is 0

For PRHA (1995) recommendations, the travelled way is 0

For present ADT, the shoulders are 0

For future (1995) ADT, the shoulders are 0

For PRHA (1995) recommendations, the shoulders are 0

Deck	Present	Future	PRHA	Present	Future	PRHA
0	0	0	0	0	0	0
Travelled way			Shoulders			

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
not required (don't print)					0

The (A) approach alignment is (B)

31

(A) vertical	1	horizontal	0	horizontal and vertical	2
(B) satisfactory	3	minimally tolerable	2	poor and represents a danger to the public	1
not required (don't print)					0

Load Post for inventory rating shown in Item 64

33

Yes	1	No (don't print)	0
-----	---	------------------	---

Post for vertical underclearance shown in Items 54 & 69

34

Yes	1	No (don't print)	0
-----	---	------------------	---

For "Remarks" see bridge file.

PERSONNEL
Structural Inspection _____
Review of Field Data _____
Transfer of Data _____

By _____ Date _____
Condition Analysis
Appraisal _____
IBM MT/SC file _____
IBM-370 file _____

BRIDGE NO. 1307

APPRAISAL OF ITEM 68

ROAD NO. 26 KM. 4.27

BRIDGE STUDIES AND
EVALUATION OFFICE

YEAR	ADT	Sec.	T W	T W	T W	T W + SH	T W + SH	T W + SH
				Adequate By	Inadequate By		Adequate By	Inadequate By
1982		<input checked="" type="checkbox"/>	Actual =	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT =					
	NA							
1995			Reqd. For ADT =					
			Rec. PRHA =					

✓ Adequate
X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM-370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

1307 0000
 Card No. Bridge No. Road No.

BRIDGE INVENTORY

Km. No. *N.A.*

IDENTIFICATION

Code Positions

	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1									
1. State	Commonwealth of Puerto Rico																							10	7	2	1							
2. Highway District	<i>San Juan</i>																							13	0	7								
3. Municipality	<i>San Juan</i>																							15	0	6	3							
4. City/Town	<i>San Juan</i>																							16	0	0	6	3						
5. Inventory Route	<i>1830500000</i>																							22	1	0	0	0	0	0	0	0	0	0
6. Features Intersected	<i>P.R. 26</i>																							31	P	R	2	6						
7. Facility Carried by Structure ⁵⁶	<i>Pedestrian walkway</i>																							74	0	1	3	0	7	1				
8. Structure No.	<i>1307 one of only</i>																							74	0	1	3	0	7	1				
9. Location ¹⁰	<i>East of Cem. Shopping Center Balleriños de Castro Avenue</i>																																	
10. Inventory Route, Minimum Vertical Clearance (0.01 m)	<i>Unlimited</i>																							10	9	9	9							
11. Kilometer Point (0.01 km)	<i>N.A.</i>																							14	0	0	0	0						
12. Road Section Number (DOD)	<i>NPH</i>																							19	0	0	0	0						
13. Bridge Description	<i>NPH 204 VSTSP</i>																							24	0	0								
14. Defense Milepoint (0.01 ml)	<i>NPH</i>																							26	0	0	0							
15. Defense Section Length (miles) (0.1 mile)	<i>NPH</i>																							30	0	0	0							
16. Latitude:	<i>18</i>	Degrees	<i>29.3</i>	Minutes																					33	1	8	0	0	0	3			
17. Longitude:	<i>66</i>	Degrees	<i>03.1</i>	Minutes																					39	0	6	0	3	1				
18. Physical Vulnerability	<i>Start Truss</i>																							44	<input checked="" type="checkbox"/>									
19. By Pass, Detour Length (Nearest mile)	<i>Walk across highway</i>																							45	0	0								
20. Toll	<i>No toll for road on bridge</i>																							47	<input checked="" type="checkbox"/>									
21. Custodian	<i>D.T.P.M.</i>																							48	<input checked="" type="checkbox"/>									
22. Owner	State Highway Department																							49	1									
23. F.A.P. No. ⁵⁰	<i>Unlimited</i>																																	

CLASSIFICATION

24. Fed. Aid System	<i>N.A.</i>																							10	0	0
25. Administrative	<i>STATE</i>																							12	<input checked="" type="checkbox"/>	
26. Functional	<i>N.A.</i>																							13	0	0

BRIDGE STUDIES AND EVALUATION OFFICE

STRUCTURE DATA

9 8 7 6 5 4 3 2 1

27. Year Built 1968 15 6 6 0 0

28. Lanes on Str. 0 Under 6 19 0 0 0 0

29. ADT - Inventory Route NA 23 0 0 0 0 0

30. Year NA 29 0 0

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) 1.62 32 0 0 0

33. Median None Open Closed 35 7

34. Skew None 36 0 0

35. Structure Flared Yes No 38 0

36. Traffic Safety Features 1-1-0-9 39 7 7 0 0

37. Blank 2 lanes 43 0 0 0

38. Navigation Control Yes No 46 0

39. Navigation Vertical Clearance (0.1 m) Yes No 47 0 0 0

40. Navigation Horizontal Clearance (0.1 m) Yes No 50 0 0 0 0

41. Structure, Open or Closed to Traffic Open 54 7

42. Type Service Pedestrian over Highway 55 3 1

43. Structure Type-Main Steel deck spans 57 3 0 0

44. Structure Type-Approach Spans None 60 0 0 0

45. No. of Spans-Main 1 used 10 0 0 0

46. No. of Spans-Approaches None 13 0 0 0 0

47. Total Horizontal Clearance (0.01 m) L = R = 1.02 17 0 0 0

48. Max. Span Length (0.01 m) 2.0142 20 0 2 0 0

49. Structure Length (0.01 m) 43.00 24 0 0 0 0 0

50. Sidewalk Widths (0.01 m) Left Right 30 0 0 0 0 0

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.02 36 0 0 0 0

52. Deck Width (out-out) (0.01 m) 1.62 40 0 0 0 0

53. Vertical Clearance over Bridge Roadway-Minimum (0.01 m) Un. limited 44 0 0 0 0

54. Vertical Underclearance - Minimum (0.01 m) 4.05 48 0 0 0 0

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.3 R = 3.28 52 0 1 0

56. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 0.45 R = 0.5 56 0 0 0

57. Wearing Surface Steel plate 58 7

9 8 7 6 5 4 3 2 1

- 78. Roadway Width (0.01 m) 29 0 0 0 0
- 79. Number of Lanes 33 0 0
- 80. ADT 35 0 0 0 0 0 0
- 81. Year of Estimated ADT 41 0 0
- 82. Year of Proposed Adjacent Roadway Improvements 43 0 0
- 83. Prop. Adj. Rdwy Improvements-Type 45 0

NA

COST OF IMPROVEMENTS

- 84. Total (dollars) \$ 7,000 (based on 1996 unit costs) 46 0 0 0 0 0 *
- Estimated Design Time (months)

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars) 51 0 0 0 *
- 86. Demolition Cost (Dollars) 54 0 0 0 *
- 87. Substructure (Dollars) 57 0 0 0 0 *
- 88. Superstructure (Dollars) 62 0 0 0 0 *
- 89. Blank 67 0 *
- 90. Rehabilitate Existing Structure (Dollars) 8,000 68 0 0 0 *
- 91. Detour and Traffic Maintenance (Dollars) 71 0 0 0 *
- 92. Approaches (Dollars) 74 0 0 0 *
- Approach Embankment (Dollars)
- Approach Pavement (Dollars)
- Approach Guardrail (Dollars)
- (* Code to nearest thousand dollars)
- 93. Date of Last Inspection July 18/75 10 0 0 1 8 7 5

REMARKS

a) Rehabilitation (Item 90) consist of

- a) Repair of joints against leakage
- b) Improvement of drainage system
- c) Removal of rust that appear in the bridge by sand blasting system and the protection of it against future corrosion with paint

BRIDGE STUDIES AND EVALUATION OF BRIDGE
GENERAL EVALUATION

This bridge is basically in 3 structural condition

excellent	4	fair	2	very poor	0
good	3	poor	1		

16

The load carrying capacity is 0

adequate	3	minimally adequate	2	inadequate	1	not required (don't print)	0
----------	---	--------------------	---	------------	---	----------------------------	---

17

The recommended modifications will make it 0

adequate	2	minimally adequate	1	not required (don't print)	0
				0 when previous is	2

18

The bridge should be replaced.

Yes (print)	1	Not required (don't print)	0
-------------	---	----------------------------	---

19

It requires 1; 12

- routine inspection every 24 months
- frequent inspection every (B) months to monitor abnormal and/or suspected deficiencies
- frequent inspection every (B) months to determine the cause of and remedies for existing defects

(A) = 2 (B) = 24

(A) = 1 (B) = months

(A) = 0 (B) = months

20

(A) (B)

It requires 2

- routine maintenance
- minor repairs and routine maintenance
- urgent repairs to prevent further costly deterioration and/or the development of a dangerous condition and routine maintenance
- emergency repairs to eliminate danger to the public and routine maintenance

3

2

1

0

23

The deck geometry is N/A
 For present ADT, the travelled way is N/A
 For future (1990) ADT, the travelled way is N/A
 For PRHA (1990) recommendations, the travelled way is 24
 For present ADT, the shoulders are N/A
 For future (1990) ADT, the shoulders are N/A
 For PRHA (1990) recommendations, the shoulders are N/A

Deck	Present	Future	PRHA	Present	Future	PRHA
<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>	<input type="checkbox"/>
Travelled way			Shoulders			

satisfactory	3	unsatisfactory	2	cannot be evaluated because of lack of information	1
				not required (don't print)	0

The (A) approach alignment is (B)

(A) vertical	1	horizontal	0	horizontal and vertical	2
(B) satisfactory	3	minimally tolerable	2	poor and represents a danger to the public	1
				not required (don't print)	0

31

(A) (B)

Load Post for inventory rating shown in Item 66

Yes	1	No (don't print)	0
-----	---	------------------	---

33

Post for vertical underclearance shown in Items 54 & 69

Yes	1	No (don't print)	0
-----	---	------------------	---

34

For "Remarks" see bridge file.

PERSONNEL
 Structural Inspection
 Review of Field Data
 Transfer of Data

By L. Pines Date Jul 18/75
J. Capers Jul 22/75
E. Powell Mar 3/76

Condition Analysis
 Appraisal
 IBM MT/SC file
 IBM-370 file

By C. Velasco Date 9/5/76
A. Perkins Apr 1976

BRIDGE NO. _____

APPRAISAL OF ITEM 68

ROAD NO. _____ KM. _____

**BRIDGE STUDIES AND
EVALUATION OFFICE**

YEAR	ADT	Sec.	T W	T W Adequate or Inadequate	T W Inadequate By	T W + SH	TW+SH Adequate or Inadequate	TW + SH Inadequate By
1970		<input checked="" type="checkbox"/>	Actual =	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>		<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
			For ADT =					
1990			Reqd. For ADT =					
			Rec. PRHA =					

✓ Adequate

X Inadequate

BRIDGE STUDIES AND EVALUATION OFFICE

PUERTO RICO HIGHWAY AUTHORITY
 IBM - 370 AND IBM MT/SC
 ELECTRONIC DATA STORAGE

Card No.

1307 Bridge No.

0026 Road No.

ROUTE UNDER STRUCTURE

Km. No. 4.28

Code Positions

IDENTIFICATION

25 24 23 22 21 20 19 18 17 16 15 14 13 12 11 10 9 8 7 6 5 4 3 2 1

- 5. Inventory Route 263.515.000.2150 ¹⁰ 2 3 1 0 0 2 6 0
- 6. Features Intersected Pedestrian Walkway ¹⁹ P E D E S T R I A N W A L K W A Y
- 8. Structure No. 1307 ⁴⁴ 0 1 3 0 7 1
- 10. Inventory Route, Minimum Vertical Clearance (0.01 m) 4.85 ⁵⁰ 0 4 8 5
- 11. Kilometer Point (0.01 km) 4.28 ⁵⁴ 0 0 4 2 8
- 12. Road Section Number (DOD) 25 ⁵⁹ 2 0 2 5 6
- 13. Bridge Description NDD. 100. 1875 P ⁶⁴ 0 6
- 14. Defense Milepoint (0.01 m) 6.31 ⁶⁶ 0 6 3 1
- 15. Defense Section Length (miles) (0.1 mile) 0.1 ⁷⁰ 0 1 1
- 19. By Pass, Detour Length (Nearest mile) N. Bridge Site ⁷³ 0 0

CLASSIFICATION

- 24. Fed. Aid System OTHER F.M. Primary Urban ⁷⁵ 0 4
- 25. Administrative S.T.A.T.F. ⁷⁷ 1
- 26. Functional Urban Principal Arterial ⁷⁶ 4 3

STRUCTURE DATA

- 29. ADT - Inventory Route 50560 ¹⁰ 0 5 0 5 6 0
- 30. Year 1970 ¹⁶ 7 0
- 47. Total Horizontal Clearance (0.01 m) L = 14.38 R = 10.35 ¹⁸ 7 0 8

E.14 November 13, 1972



COMMONWEALTH OF PUERTO RICO
HIGHWAY AUTHORITY
IBM-370 AND IBM MT/SC
ELECTRONIC DATA STORAGE

Page 1 of 4

C. Providencia

Card No. Bridge No. 7307 Road No. 6666

BRIDGE INVENTORY

Km. No. _____

Code Positions

IDENTIFICATION	25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1							
1. State	Commonwealth of Puerto Rico																							10	7	2	1					
2. Highway District	San Juan																							13	0	1						
3. Municipality	San Juan																							15	0	6	3					
4. City/Town	San Juan																							18	0	0	6	3				
5. Inventory Route	C. Providencia, Pedestrian walkway																							22	1	8	0	6	6	6	6	0
6. Features Intersected	P.R. 26																							31	P	R	2	6				
7. Facility Carried by Structure	Pedestrian walkway, C. Providencia																							52								
8. Structure No.	1307, 1071																							74	0	1	3	0	7	1		
9. Location	East of Gem. shopping center, Are. Baldorioty de Castro																							10								
10. Inventory Route, Minimum Vertical Clearance (0.01 m)	Unlimited																							10	9	7	9	9				
11. Kilometer Point (0.01 km)	N.A.																							14	6	6	6	6				
12. Road Section Number (DOD)	Nat. defense highway																							19	0	0	0	0				
13. Bridge Description	N.A. H.D.H. N.S.T.S. P.																							24	6	7						
14. Defense Milepoint (0.01 m)	Nat. defense highway																							26	0	0	0	0				
15. Defense Section Length (miles) (0.1 mile)	Nat. defense highway																							30	0	0	0					
16. Latitude: Degrees Minutes	1.8 27.0																							33	1	8	2	7	0			
17. Longitude: Degrees Minutes	6.6 03.0																							38	0	6	6	0	3	0		
18. Physical Vulnerability	Steel Truss																							44	7							
19. By Pass, Detour Length (Nearest mile)	walk across highway																							45	0	0						
20. Toll	No toll for road or bridge																							47	3							
21. Custodian	State																							48	7							
22. Owner	State Highway Department																							49	1							
23. F.A.P. No.	50																															
CLASSIFICATION																																
24. Fed. Aid System	N.A.																							10	6	6						
25. Administrative	state																							12	7							
26. Functional	Pedestrian walkway, N.A.																							13	6	6						

B2 130

9 8 7 6 5 4 3 2 1

STRUCTURE DATA

27. Year Built 1968 15 8800

28. Lanes on Str. Under 6 19 0006

29. ADT - Inventory Route: N.A. 23 000000

30. Year N.A. 29 00

31. Design Load Pedestrian 31 7

32. Approach Roadway width including shoulders (0.01 m) N.A. 32 666

33. Median None Open Closed 35 0

34. Skew No 36 00

35. Structure Flared Yes No 38 0

36. Hydraulic Structure Yes No 39 0

37. Report Available Yes No 40 0

38. Navigation Control N.A. Yes No 41 0

39. Navigation Vertical Clearance (0.1 m) N.A. Yes No 42 000

40. Navigation Horizontal Clearance (0.1 m) N.A. Yes No 45 0000

41. Relief Structures None 49 00

42. Type Service Pedestrian, over highway 51 37

43. Structure Type-Main Steel deck, Truss, Bailey Bridge 53 309

44. Structure Type-Approach Spans None 56 000

45. No. of Spans-Main 2 10 002

46. No. of Spans-Approaches None 13 0000

47. Total Horizontal Clearance (0.01 m) L = 1.62 R = 17 016

48. Max. Span Length (0.01 m) 20.87 20 0203

49. Structure Length (0.01 m) 29.62 24 000396

50. Sidewalk Widths (0.01 m) Left 0.00 Right 0.00 30 000000

51. Bridge Roadway Width (curb-curb) (0.01 m) 1.62 walkway 36 0076

52. Deck Width (out-out) (0.01 m) 1.62 40 0076

53. Vertical Clearance over Bridge Roadway--Minimum (0.01 m) Unlimited 44 9999

54. Vertical Underclearance - Minimum (0.01 m) 4.85 48 0485

55. Lateral Underclearance on Right (Outer) Sides - Minimum (0.01 m) L = 1.30 R = 3.28 52 013

56. Lateral Underclearance on Left (Median) Sides - Minimum (0.01 m) L = 0.95 R = 0.50 55 005

57. Wearing Surface Steel plate 58 7



CR. 1307

CONDITION		9 8 7 6 5 4 3 2 1									
	Material	Condition Analysis									
58.	Deck . steel	Minor. 1 r.m.f.									59 [7]
59.	Superstructure . steel	Minor. 1 r.m.f.									60 [7]
60.	Substructure . steel	Minor. 1 r.m.f.									61 [7]
61.	Channel & Channel Protection . N.A										62 [N]
62.	Culvert & Retaining Walls . N.A										63 [N]
63.	Estimated Remaining Life . 20 years										64 [2][0]
64.	Operating Rating	PEDESTRIAN LOADING									66 [8][0][6]
65.	Approach Alignment	N.A.									69 [N]
66.	Inventory Rating	PEDESTRIAN LOADING									70 [8][0][0]

APPRAISAL

		Deficiencies									
67.	Structural Condition	Needs painting									10 [7]
68.	Deck Geometry	N.A.									11 [N]
69.	Underclearances—Vert. & Lateral X=4.85 m; Lateral underclearance width equals recommended (P.R.H.A.) (1990) section width										12 [6]
70.	Safe Load Capacity	N.A.									13 [6]
71.	Waterway Adequacy	N.A.									14 [N]
72.	Approach Alignment	N.A.									15 [N]

PROPOSED IMPROVEMENTS

73.	Year Needed	1973										16 [2][3]
	Completed											
	Describe											
74.	Type of Service	Pedestrian									18 [3]	
75.	Type of Work	Rehabilitation									19 [3][7][1]	
76.	Improvement Length (0.1 m)	43.0									22 [0][0][4][3][0]	
77.	Design Loading	N.A.									28 [0]	



Bn 13070

- 78. Roadway Width (0.01 m) 1.62 29 0 0 1 6
- 79. Number of Lanes N.A. 33 0 0
- 80. ADT N.A. 35 0 0 0 0 0 0
- 81. Year of Estimated ADT N.A. 41 0 0
- 82. Year of Proposed Adjacent Roadway Improvements N.A. 43 0 0
- 83. Prop. Adj. Rdwy Improvements-Type N.A. 45 0

COST OF IMPROVEMENTS

- 84. Total (dollars) \$ 3,000 46 0 0 0 0 3 *

SUMMARY OF IMPROVEMENT COSTS

- 85. Preliminary Engineering (Dollars) 51 [] [] [] *
- 86. Demolition Cost (Dollars) 54 [] [] [] *
- 87. Substructure (Dollars) 57 [] [] [] [] *
- 88. Superstructure (Dollars) 62 [] [] [] [] *
- 89. Priority Letter 67 [] *
- 90. Rehabilitate Existing Structure (Dollars) 68 [] [] [] *
- 91. Detour and Traffic Maintenance (Dollars) 71 [] [] [] *
- 92. Approaches (Dollars) 74 [] [] [] *

Approach Embankment (Dollars) _____

Approach Pavement (Dollars) _____

Approach Guardrail (Dollars) _____

(*Code to nearest thousand dollars)

Remarks
 Rehabilitation ^{required} ~~should~~ include s. painting

PERSONNEL	By	Date		By	Date
Structural Inspection	S.M. Rodgers	4/13/72	Condition Analysis		
Topographic Survey			Appraisal		
Review of Field Data			IBM MT/SC file		
Transfer of Data			IBM-370 file		

B. 9307

REMARKS

Multiple horizontal lines for handwritten remarks.

Travelled way narrow for present (1970) ADT by . Narrow for future (1990) ADT by , and for future PRHA recommended section by .
Roadway (including shoulders) narrow for present (1970) ADT by and for future PRHA recommended section by .
Narrow for future (1990) ADT by

GENERAL EVALUATION

This bridge is basically in excellent structural condition.
~~fair~~
good
~~poor~~
~~very poor~~

The load-carrying capacity is ~~minimally~~ adequate ~~inadequate~~.

The recommended modifications will make it ~~minimally~~ adequate.

The bridge should be replaced.

It requires routine inspection (every 2 years).
~~frequent inspection (every months) to determine the cause of~~
~~and remedies for existing defects.~~
frequent inspection (every months) to monitor abnormal
and/or suspected deficiencies.

It requires routine maintenance,
minor repairs and routine maintenance,
emergency repairs to eliminate danger to the public,
urgent repairs to prevent further costly deterioration and/or
the development of a dangerous condition.
Non-urgent programmed repairs.

The deck geometry is ~~unsatisfactory~~. The travelled way is ~~unsatisfactory~~ for present requirements. The shoulders are ~~unsatisfactory~~ for present requirements.

The travelled way is ~~unsatisfactory~~ for future (1990) () requirements.

The shoulders are ~~unsatisfactory~~ for future (1990) () requirements.

The approach alignment is satisfactory
~~minimally adequate.~~
~~poor and represents a danger to the public.~~



Card No.

Bridge No.

Road No.

ROUTE UNDER STRUCTURE

Km. No. 4.29

Code Positions

IDENTIFICATION

25	24	23	22	21	20	19	18	17	16	15	14	13	12	11	10	9	8	7	6	5	4	3	2	1
----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	---	---	---	---	---	---	---	---	---

- 5. Inventory Route . . . PR 26 . . . 263616.0002650 . . . 10
- 6. Features Intersected Pedestrian walkway, C. Providencia
 19
- 8. Structure No. . . 1307 1.0P1 44
- 10. Inventory Route, Minimum Vertical Clearance (0.01 m) . . . 4.85 50
- 11. Kilometer Point (0.01 km) . . . 4.29 54
- 12. Road Section Number (DOD) . . . 62 59
- 13. Bridge Description . . . NDH.D.H. N.T.S. P. 64
- 14. Defense Milepoint (0.01 m) . . . 1.23 66
- 15. Defense Section Length (miles) (0.1 mile) . . . 1.7 70
- 19. By Pass, Detour Length (Nearest mile) . . . 1 mile 73

CLASSIFICATION

- 24. Fed. Aid System . . . Other Federal Aid, Primary urban 75
- 25. Administrative . . . State 77
- 26. Functional . . . Urban principal arterial 78

STRUCTURE DATA

- 29. ADT - Inventory Route . . . 36,550 10
- 30. Year 1970 16
- 47. Total Horizontal Clearance (0.01 m) . . . L 14.38 . . . R 10.35 18

Card 2

Bridge Inspection Report

Bridge Key: 023361 Agency ID: 023361 Sufficiency Rating: -2.0

IDENTIFICATION

State 1: 72 Puerto Rico Struc Num 8: 023361
 Facility Carried 7: PR 17 (PINERO AV.) Location 9: 4.48 KM EAST OF PR 18
 Rte. (On/Under)5A: One Route Under Rte. Signing Prefix 5B: 3 State Hwy
 Level of Service 5C: 1 Mainline Rte. Number 5D: 00017
 Directional Suffix 5E: 0 N/A (NBI) % Responsibility: NA
 SHD District 2: -1 County Code 3: SAN JUAN
 Place Code 4: 74017 Kilometer Post 11: 08.2 km
 Feature Intersected 8: PEDESTRIAN WALKWAY
 Latitude 16: 18d 24' 36" Longitude 17: 066d 01' 30"
 Border Bridge Code 98: Unknown (P)
 Border Bridge Number 99: Unknown

INSPECTION

Frequency 91: 48 months Inspection Date 90: 4/16/2014 Next Inspection: 04/16/2018
 FC Frequency 92A: NA FC Inspection Date 93A: NA Next FC Inspection: NA
 UW Frequency 92B: NA UW Inspection Date 93B: NA Next UW Inspection: NA
 SI Frequency 92C: NA SI Date 93C: NA Next SI: NA
 Element Frequency: 48 months Element Inspection Date: 01/01/1901 Next Elem. Insp. Due: 01/01/1901

CLASSIFICATION

Defense Highway 100: 0 Not a STRAHNET hwy Parallel Structure 101: Unknown (NBI)
 Direction of Traffic 102: 0 Not hwy traffic Temporary Structure 103: Not Applicable (P)
 Highway System 104: 0 Not on NHS NBIS Length 112: Unknown (NBI)
 Toll Facility 20: 3 On free road Functional Class 26: 14 Urban Other Princ
 Historical Significance 37: Not Applicable (P)
 Owner 22: -1 Unknown (P)
 Custodian 21: -1 Unknown (P)

STRUCTURE TYPE AND MATERIALS

Number of Approach Spans 46: -1 Number of Spans Main Unit 45: -1
 Main Span Material/Design 43A/B: 5 Prestressed Concrete 06 Single/Spread Box
 Deck Type 107: Unknown (NBI)
 Wearing Surface 108A: Unknown (NBI)
 Membrane 108B: Unknown (NBI)
 Deck Protection 108C: Unknown (NBI)

CONDITION

Deck 58: 6 Satisfactory Super 59: 7 Good Sub 60: 7 Good
 Culvert 62: N N/A (NBI) Channel/Channel Protection 61: N N/A (NBI)

LOAD RATING AND POSTING

Inventory Rating Method 65: Not Applicable (P) Operating Rating Method 63: Not Applicable (P)
 Inventory Rating 66: MS.0 Operating Rating 64: MS0.0
 Design Load 31: 7 Pedestrian Posting 70: Unknown (NBI)
 Posting status 41: Not Applicable (P)

AGE AND SERVICE

Year Built 27: 1994 Year Reconstructed 106: Unknown
 Type of Service on 42A: 3 Pedestrian-bicycle
 Type of Service under 42B: 1 Highway
 Lanes on 28A: Unknown Lanes Under 28B: 8 Detour Length 19: 0.0 km
 ADT 29: 89,200 Truck ADT 109: 5 % Year of ADT 30: 2005

APPRAISAL

Bridge Rail 36A: N N/A or not required Approach Rail 36C: N N/A or not required
 Transition 36B: N N/A or not required Approach Rail Ends 36D: N N/A or not required
 Str. Evaluation 67: N Deck Geometry 68: N Not applicable (NBI)
 Underclearance, Vertical and Horizontal 69: 3 Intolerable - Correct
 Waterway Adequacy 71: N Not applicable Approach Alignment 72: Not Applicable
 Scour Critical 113: Not Applicable (P)

GEOMETRIC DATA

Length Max Span 48: 25.00 m Structure Length 49: 49.90 m
 Curb/Sdwik With L 50A: Curb/Sidewalk Width R 50B:
 Width Curb to Curb 51: 2.10 m Width Out to Out 52:
 Approach Roadway Width 32: 0.00 m Median 33: Unknown (NBI)
 Deck Area:
 Skew 34: -1.00 * Structure Flared 35: Unknown (NBI)
 Minimum Vertical Clearance Over Bridge 53:
 Minimum Vertical Underclearance Reference 54A: H Hwy beneath struct
 Minimum Vertical Underclearance 54B: 05.03 m
 Minimum Lateral Underclearance Reference R 55A: H Hwy beneath struct
 Minimum Lateral Underclearance R 55: 00.80 m
 Minimum Lateral Underclearance L 56: 00.40 m

PROPOSED IMPROVEMENTS

Bridge Cost 94: \$ 0 Type of Work 75: Not Applicable (P)
 Roadway Cost 95: \$ 0 Length of Improvement 76: 00.00 m
 Total Cost 96: \$ 0 Future ADT 114: 104,125
 Year of Cost Estimate 97: 2014 Year of Future ADT 115: 2020

NAVIGATION DATA

Navigation Control 38: - Unknown (NBI)
 Vertical Clearance 39: Horizontal Clearance 40:
 Pier Protection 111: Not Applicable (P) Lift Bridge Vertical Clearance 116:

ELEMENT CONDITION STATE DATA

BRIDGE NOTES

****note**:** AT THE PRESENT TIME (04-16-2014) THIS STRUCTURE IS BEING RECONSTRUCTED. CONSTRUCTION OF ADDITIONAL SPAN AT SOUTH SIDE BY CONSTRUCTION OF RAMP WITH ACCESS TO PR-17.

Bridge Inspection Report

PAST INSPECTION

Inspection Date: 04/16/2014

Type: 1 Regular NBI

Inspector: -1

Pontis User Key: Pontis - Pontis Poi

Scope:

NBI:

Other:

Element:

Underwater:

Fracture Critical:

INSPECTION NOTES

FINE TRANSVERSE AND LONGITUDINAL CRACKS, EXPOSED AGGREGATES AND SMALL SPALLINGS AT WEARING SURFACE. SOME HOLES AT CYCLONE FENCE OF RAILINGS.

INSPECTOR WORK CANDIDATES

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

Team Leader: Arturo Cáceres

Bridge Inspector: Micky Santiago

Bridge Evaluator: Arturo Cáceres

Assistants: John Dávila _ _ _ _

Driver: Juan C. Otero

Inspection date: Abril-16-2014

Weather Conditions: Sunny

Amount of Time on Inspection: 1 Hours

Equipment: Bus or Van Underwater Ladders Snooper Camera Boat
 Other:

Bridge Number: **2336**

Road on Structure: N/A **Number or Name:** _ Km. _

Road Under Structure: State Highway **Number or Name:** PR-17 Km. 8.2

Ident. Plaque: No **Num.** _

36. Traffic Safety Features:

Bridge railings: not applicable or safety not required

Transitions: not applicable or safety not required

Approach Guardrail: not applicable or safety not required

Approach Guardrail Ends: not applicable or safety not required


41. Posting: **Condition:** A - Open **Sign Type:** _ **Posting Load:** _

COMMENTS AND/OR RECOMMENDATIONS:

Actualmente se está trabajando en la estructura, construcción de un "span" adicional en lado sur, por construcción de rampa de acceso a la PR-17.

Inspection by:  **Micky Santiago**

Bridge Inspector

Revised and Approved by:  **Arturo Cáceres**

Bridge Evaluator

BR-2336

1

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

58. DECK

58.1 Wearing Surface: Material: Concrete **Condition:** 6 Thickness cm.

Deterioration: 0 to 10 % Drainage: Adequate Ponding: Yes Safety: Yes

58.2 Slab or Plate: Material: Concrete **Condition:** 7

Cracking: None

Spalling: Top: Small Bottom: No

Scaling: Top: Light Bottom: No

Efflorescence: No Exudation: No Rust Stains: No Corrosion: No

58.3 Movement: Deck to backwall: cm. Deck to approach slab: cm.

	Material	Cond. Rating	Height Loss (cm)	Joints	Drainage	Alignment	Corrosion	Paint	Safety	Cracking	Spalling	Scaling
58.4 Curb	N/A	--		--	--	--	--	--	--	--	--	--
58.5 Median	N/A	--		--	--	--	--	--	--	--	--	--
58.6 Sidewalks	N/A	--		--	--	--	--	--	--	--	--	--
58.7 Parapets	Concrete	7		Good	Good	Good	--	Good	Yes	Fine	--	--
58.8 Railing	Galvanized	7		Good	Good	Good	--	Good	Yes	--	--	--

58.9 Lighting Standards: Material: Aluminium Condition: 7 Functioning: Yes

58.10 Utilities:

Type: Size: Safety: _

Type: Size: Safety: _

Type: Size: Safety: _

58.11 Joints: Condition: 6

Type: Expansion Functioning: Yes Leaking: Yes Cracking: N Spalling: N Armor: No

Type: N/A Functioning: N/A Leaking: N/A Cracking: N Spalling: N Armor: No

58.12 Drains and Scuppers:

Material: Other Condition: N Functioning: N/A

Comments:

58.1- Superficie con algunas grietas finas transversales y longitudinales, agregados expuestos y pequeños "spallings".

58.8- Algunos huecos en las mallas de protección del "railing".

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

59. SUPERSTRUCTURE

59.1 Bearing Devices: Type: Elastometric Out of Plumb: No Paint: None

Condition: 7

59.2 Bridge seats, pedestal, grout pads, abutments or pier seat where beams bear directly on conc.

Condition: 7 Cracking: No Spalling: None Scaling: None

Crushing: No

59.3 Steel beams: N/A Type: N/A

Condition: N Corrosion: None Paint: None

59.4 Concrete: Slab Type: Prest.

Condition: 7 Cracking: No Spalling: None Scaling: None

59.5 Truss: N/A Paint: None Corrosion: None Members: N/A

59.6 Drainage: Type: **Condition:** N Functioning: No

59.7 Hinges: **Condition:** N Functioning: N/A Movement: N/A

59.8 Deflection: Normal

59.9 Vibrations: Minimal

60.0 SUBSTRUCTURE:

			Material	Cond. Rating	Cracking	Spalling	Scaling	Corrosion	Paint	Movement	Erosion	Undermining
Abutments	60.1.1	Wingwalls	N/A	N	--	--	--	--	--	--	--	--
	60.1.2	Breast-Backwall	N/A	N	--	--	--	--	--	--	--	--
	60.1.3	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.1.4	Piles	N/A	N	--	--	--	--	--	--	--	--
Piers or Non Pile Bents	60.2.1	Caps	Concrete	7	F	--	--	--	Good	No	--	--
	60.2.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.2.3	Columns	Concrete	7	F	--	--	--	Good	No	No	No
	60.2.4	Footing	N/A	N	--	--	--	--	--	--	--	--
	60.2.5	Piles	N/A	N	--	--	--	--	--	--	--	--
Rocker Bents	60.3.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.3.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.3.3	Piles	N/A	N	--	--	--	--	--	--	--	--
Pile Bents	60.4.1	Caps	N/A	N	--	--	--	--	--	--	--	--
	60.4.2	Bracing	N/A	N	--	--	--	--	--	--	--	--
	60.4.3	Piles	N/A	N	--	--	--	--	--	--	--	--

Comments:

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

71. WATER ADEQUACY: N/A N/A

72. APPROACH ROADWAY

72.1 Alignment: 6

72.2 Slab or Pavement Condition: N Material: Other

Cracking: No Spalling: None Scaling: None Uneven: No
Rough: No Settlement: No Movement: Approach slab-backwall: No Breaking up: No
Excessive deformation: No
Safety: Hazardous: No
Drainage: Inadequate: No
Movement: Pavement-approach slab: No
Embankment: Condition: N Functioning: Yes Erosion: No
Construction: Flimsy: No Integrity impaired: No

72.3 Undesirable Impact: No

72.4 Joints: No Type: None Inadequate: N/A Satisfactory Alignment: N/A

72.5 Guardrail Type: None Material: N/A Functioning: N/A

Condition: N Alignment Horizontal: N/A Vertical: N/A

Joints Functioning: N/A Safety Securely Attached: N/A Pedestrian Hazards: N/A

Comments:

90. INSPECTION DATE: Abril-16-2014

93. CRITICAL FEATURE INSPECTION DATE

Fracture Critical:

Underwater:

Other Special Inspection:

102. DIRECTION OF TRAFFIC: Highway traffic not carried

106. YEAR RECONSTRUCTED:

107. DECK STRUCTURE TYPE: Concrete Cast-in-Place

108. WEARING SURFACE/PROTECTIVE SYSTEM

Type wearing surface: Concrete

Type Membrane: None

Deck protection: None

111. PIER OR ABUTMENT PROTECTION (FOR NAVIGATION): N/A

113. SCOUR CRITICAL BRIDGES: N

116. MINIMUM NAVIGATION VERTICAL CLEARANCE:

AUXILIARY ITEMS

Signs: Type: Route Orientation Material: Alum. Condition: 7

Type: N/A

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

RATING ITEM 58 TO 60:

- N:** Not applicable
- 9:** Excellent Condition
- 8:** Very Good Condition: no problem noted.
- 7:** Good Condition: some minor problems.
- 6:** Satisfactory Condition: structural element show some minor deterioration.
- 5:** Fair Condition: all primary structural elements are sound but may have minor section loss, cracking, spalling, or scour.
- 4:** Poor condition: advanced section loss, deterioration, spalling, or scour.
- 3:** Serious condition: loss of section, deterioration, spalling, or scour have seriously affected primary structural components. Local failures are possible. Fatigue cracks in steel or shear cracks in concrete may be present.
- 2:** Critical condition: advanced deterioration of primary structural elements. Fatigue cracks in steel or shear cracks in concrete may be present or scour may have removed substructure support. Unless closely monitored it may be necessary to close the bridge until corrective action is taken.
- 1:** "Imminent" failure condition: major deterioration or section loss present in critical structural components or obvious vertical or horizontal movement affecting structure stability. Bridge is closed
- 0:** Failed condition: out of service: beyond corrective action.

RATING ITEM 61:

- N:** Not applicable. Use when bridge is not over a waterway (channel)
- 9:** There are no noticeable noteworthy deficiencies which affect the condition of the channel
- 8:** Banks are protected or well vegetated. River control devices such as spur dikes and embankment protection are not required or are in a stable condition.
- 7:** Bank protection is in need of minor repairs. River control devices and embankment protection have a little minor damage. Banks and/or channel have minor amounts of drift.
- 6:** Bank is beginning to slump. River control devices and embankment protection have widespread minor damage. There is minor stream bed movement evident. Debris is restricting the channel slightly.
- 5:** Bank protection is being eroded. River control devices and/or embankment have major damage. Trees and brush restrict the channel.
- 4:** Bank and embankment protection is severely undermined. River control devices have severe damage. Large deposits of debris are in the channel.
- 3:** Bank protection has failed. River control devices have been destroyed. Stream bed aggradations, degradation, or lateral movement has changed the channel to now threaten the bridge and/or approach roadway.
- 2:** The channel has changed to the extent the bridge is near a state of collapse.
- 1:** Bridge closed because of channel failure. Corrective action may put back in light service.
- 0:** Bridge closed because of channel failure. Replacement necessary.

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

RATING ITEM 62:

N: Not applicable. Use if structure is not a culvert.

9: No deficiencies.

8: No noticeable or noteworthy deficiencies which affect the condition of the culvert. Insignificant scrape marks caused by drift.

7: Shrinkage cracks, light scaling, and insignificant spalling which does not expose reinforcing steel. Insignificant damage caused by drift with no misalignment and not requiring corrective action. Some minor scouring has occurred near curtain walls, wingwalls, or pipes. Metal culverts have a smooth symmetrical curvature with superficial corrosion and no pitting.

6: Deterioration or initial disintegration, minor chloride contamination, cracking with some leaching, or spalls on concrete or masonry walls and slabs. Local minor scouring at curtain walls, wingwalls, or pipes. Metal culverts have a smooth curvature, non-symmetrical shape, significant corrosion, or moderate pitting.

5: Moderate to major deterioration or disintegration, extensive cracking and leaching, or spalls on concrete or masonry walls and slabs. Minor settlement or misalignment. Noticeable scouring or erosion at curtain walls, wingwalls, or pipes. Metal culverts have significant distortion and deflection in one section, significant corrosion, or deep pitting.

4: Large spalls, heavy scaling, wide cracks, considerable efflorescence, or opened construction joint permitting loss of backfill. Considerable settlement or misalignment. Considerable scouring or erosion at curtain walls, wingwalls, or pipes. Metal culverts have significant distortion and deflection throughout, extensive corrosion or deep pitting.

3: Any condition described in Code 4 but which is excessive in scope. Severe movement or differential settlement of the segments, or loss of fill. Holes may exist in walls or slabs. Integral wingwalls nearly severed from culvert. Severe scour or erosion at curtain walls, wingwalls, or pipes. Metal culverts have extreme distortion and deflection in one section, extensive corrosion, or deep pitting with scattered perforations.

2: Integral wingwalls collapsed severe settlement of roadway due to loss of fill. Section of culvert may have failed and can no longer support embankment. Complete undermining at curtain walls and pipes. Corrective action required to maintain traffic. Metal culverts have extreme distortion and deflection throughout with extensive perforations due to corrosion.

1: Bridge closed. Corrective action may put back in light service.

0: Bridge closed. Replacement necessary.

**PUERTO RICO HIGHWAY AND TRANSPORTATION AUTHORITY
BRIDGE ENGINEERING OFFICE**

RATING ITEM 113:

N: Bridge not over waterway.

U: Bridge with "unknown" foundation that has not been evaluated for scour. Until risk can be determined, a plan of action should be developed and implemented to reduce the risk to users from a bridge failure during and immediately after a flood event (see HEC 23).

T: Bridge over "tidal" waters that has not been evaluated for scour, but considered low risk. Bridge will be monitored with regular inspection cycle and with appropriate underwater inspections until an evaluation is performed ("Unknown" foundations in "tidal" waters should be coded U.)

9: Bridge foundations (including piles) on dry land well above flood water elevations.

8: Bridge foundations determined to be stable for assessed or calculated scour conditions. Scour is determined to be above top of footing. (Example A) by assessment i.e., bridge foundation are on rock formations that have been determined to resist scour within the service life of the bridge), by calculation or by installation of properly designed countermeasures (see HEC 23).

7: Countermeasures have been installed to mitigate an existing problem with scour and to reduce the risk of bridge failure during a flood event. Instructions contained in a plan of action have been implemented to reduce the risk to users from a bridge failure during or immediately after a food event.

6: Scour calculation/evaluation has not been made. (Use only to describe case where bridge has not yet been evaluated for scour potential.)

5: Bridge foundations determined to be stable for assessed or calculated scour conditions. Scour is determined to be within the limits of footing or piles (Exam. B) By assessment i.e., bridge foundations are on rock formations that have been determined to resist scour within the service life of the bridge), by calculations or by installation of properly designed countermeasures (see HEC 23).

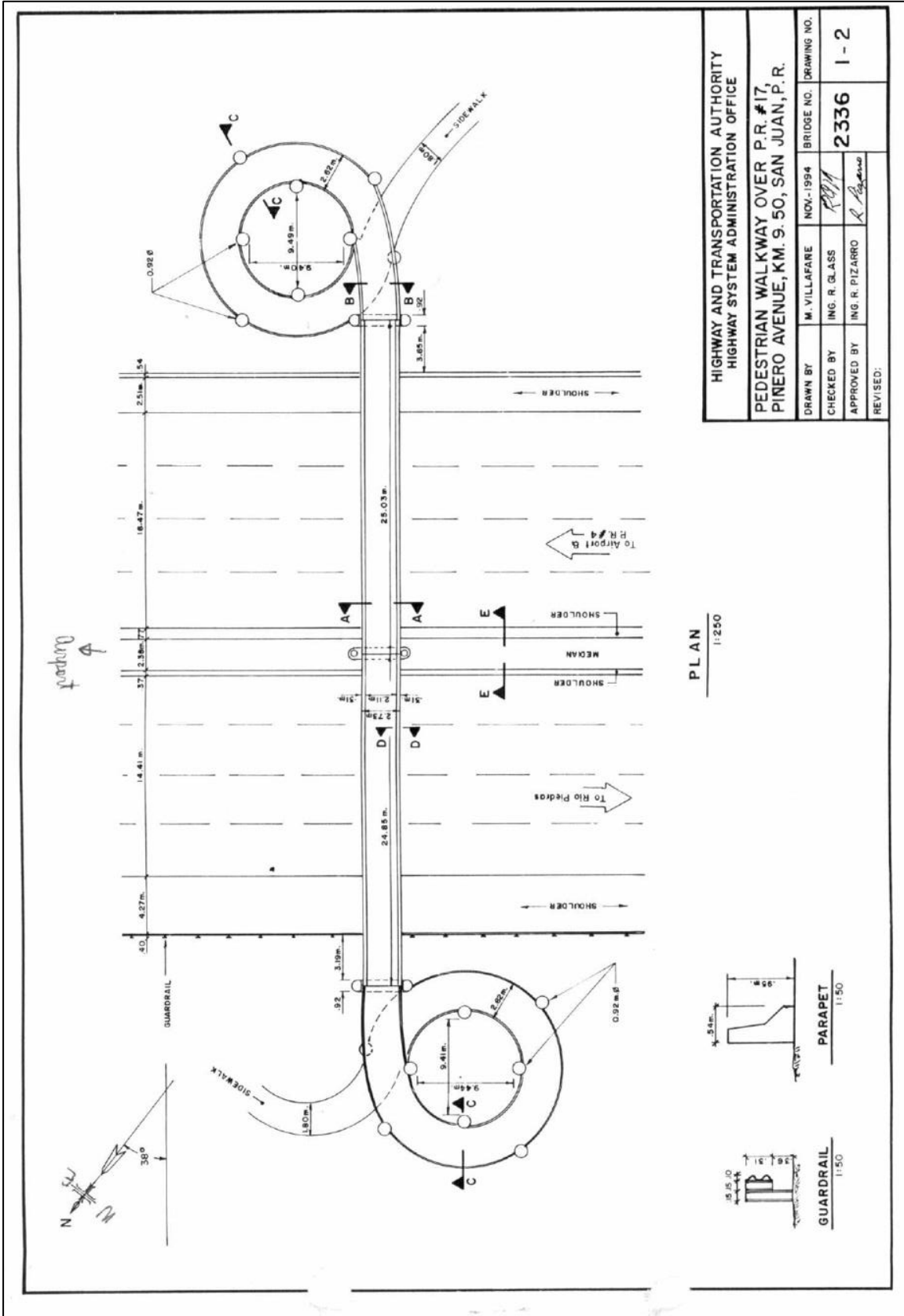
4: Bridge foundations determined to be stable for assessed or calculated scour conditions; field review indicates action is required to protect exposed foundation (see HEC 23).

3: Bridge is scour critical; bridge foundations determined to be unstable for assessed or calculated scour conditions: Scour within limits of footing or piles. (Example B) or Scour below spread-footing base or piles tips. (Example C)

2: Bridge is scour critical; field review indicates that extensive scour has occurred at bridge foundations, which are determined to be unstable by: (a comparison of calculated scour and observed scour during the bridge inspection) or (an engineering evaluation of the observed scour condition reported by the bridge inspector in Item 60)

1: Bridge is scour critical; field review indicates that failure of piers/abutments is imminent. Bridge is closed to traffic. Failure is imminent based on: (a comparison of calculated scour and observed scour during the bridge inspection) or (an engineering evaluation of the observed scour condition reported by the bridge inspector in Item 60).

0: Bridge is scour critical. Bridge has failed and is closed to traffic.

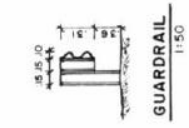
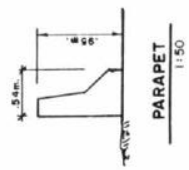


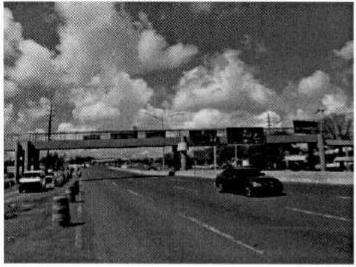
HIGHWAY AND TRANSPORTATION AUTHORITY
HIGHWAY SYSTEM ADMINISTRATION OFFICE

PEDESTRIAN WALKWAY OVER P.R. #17,
PINERO AVENUE, KM. 9.50, SAN JUAN, P.R.

DRAWN BY	M. VILLAFANE	NOV-1994	BRIDGE NO.	2336	DRAWING NO.	1-2
CHECKED BY	ING. R. GLASS					
APPROVED BY	ING. R. PIZARRO					
REVISED:						

PLAN
1:250

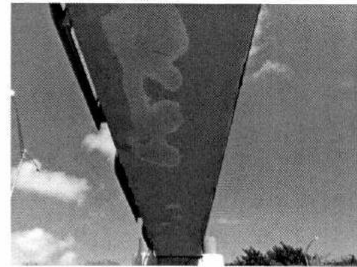




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2336-Apr.-16-2014-004.jpg



2336-Apr.-16-2014-006.jpg



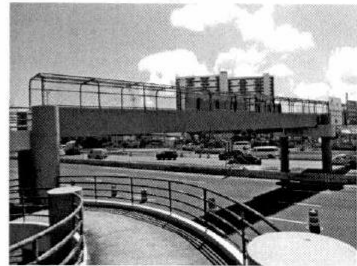
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2336-Apr.-16-2014-012.jpg

Improving Transportation Infrastructure Resilience Against Hurricanes, Other Natural Disasters, and Weathering: Part II - Analysis of Pedestrian Bridges Failures Due to Hurricane Maria

Volume 2

FINAL REPORT
August 2021

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And
Puerto Rico
Department of Transportation and Public Works
And
U.S. Department of Transportation
Federal Highway Administration

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1 Introduction

On September 20, 2017, Puerto Rico was impacted by Hurricane Maria, causing devastation across the island. After Hurricane Maria, Puerto Rico was left with its communication, electrical power, potable water, and transportation infrastructure severely affected. While for some sectors the restoration of services took days, in other areas it took several months. The damages caused by Hurricane Maria in Puerto Rico and the Virgin Islands was about \$90 billion, making it the third most expensive hurricane in the history of the United States (Pasch, Penny, & Berg, 2019). Utilities services were not the only ones affected. Among the irreparable damaged were structures such as houses, buildings, roads, and bridges.

This research project focuses on a pedestrian bridge that used to be located at kilometer 4.1 of the Román Baldorioty de Castro Expressway in San Juan, Puerto Rico. After Hurricane Maria, it was observed that the bridge experienced permanent lateral deflections. Based on preliminary observations, it was theorized that the plastic lateral deflection was the result of the pressure generated by the strong winds on guide signs that were installed on the bridge at some point after the bridge was constructed.

The objective of this project is to assess if the presence of the signs played a key role in the resulting inelastic lateral deflections. To that end, the bridge's general information (geometry and material properties) was obtained in order to develop a structural model of the bridge. This model was subjected to lateral loads consistent with those of the winds cause by Hurricane Maria. Two cases were considered in the analysis of the model: with and without the presence of the guide signs that were present on the bridge during the hurricane. When the results of both cases were compared, it was determined that the signs

were an important factor in the lateral deflection of the bridge. There were also other factors observed that may have contributed to the large lateral deflections. Conclusions and recommendations on the impacts of placing signs on pedestrian bridges are presented, together with suggestions that can be considered as preventive measures.

2 Literature Review

To develop the case study, relevant background information was collected, focusing on bridge types and characteristics, bridge modeling, wind loads modelling, steel material behavior (plastic deformations, corrosion), bridge inspection and condition assessment procedures, and bridge design. In this chapter, the topics mentioned were synthesized into two sections. The first section covers the issues of failures and how to identify them, while the second section focuses on pedestrian bridge design.

2.1 Structural Failures on Steel Members

A structure can react in different ways, depending on its current conditions. Therefore, this section covers topics relevant to the structural failure of steel bridges (plastic deformations and corrosion in structural members) and bridge inspection procedures to help identify these and other failures.

2.1.1 Plastic Deformation

When materials are exposed to loads, they suffer deformations. Most of these deformations are in the elastic range and the material returns to its original form when unloaded, but this is not always the case. Materials can also deform in the plastic range, in which the material does not return to its original form when unloaded. In this section, basic concepts of mechanics of materials are reviewed to further explained the subject of plastic deformations.

Stress is a measure of how a force applied to a body is distributed over a particular area. In the case of an axially loaded bar, normal stress is computed with the following equation:

$$\sigma = \frac{P}{A} \quad (1)$$

where:

σ = normal stress

P = axial load

A = cross sectional area of the bar

Typically, a positive sign is used for tensile loads, while a negative sign is used for compressive loads. When using the United States Customary System (USCS), stress is most typically measured in pounds per square inch (psi).

When a body experiences stress, it will generally cause deformations. For an axially loaded bar, a positive normal stress will cause the member to stretch in its longitudinal direction. If the normal stress is negative, the bar will contract in its longitudinal direction. This deformation in the material is known as normal strain and it can be computed using the following formula:

$$\varepsilon = \frac{\delta}{L} \quad (2)$$

where:

ε = normal strain

δ = change in length of the bar

L = length of the bar before it is deformed by being loaded

The interaction between stress and strain in an axially loaded bar can be analyzed to understand the behavior of the material. This interaction is best represented with the stress vs. strain diagram. In the diagram, the vertical axis represents the normal stress of the bar, while the horizontal axis represents the normal strain of the bar.

Figure 2.1 is an illustration of a typical stress vs. strain diagram for steel. The first section of the diagram is known as the linear elastic region. In the linear region, the material will suffer a deformation as the stress is applied, but once the stress is removed, the material will return to its original shape with no permanent deformations. In this section, the stress is proportional to the strain. The slope of the line in this linear elastic region is known as the modulus of elasticity of the material, and has the same stress units, since the strain is dimensionless.

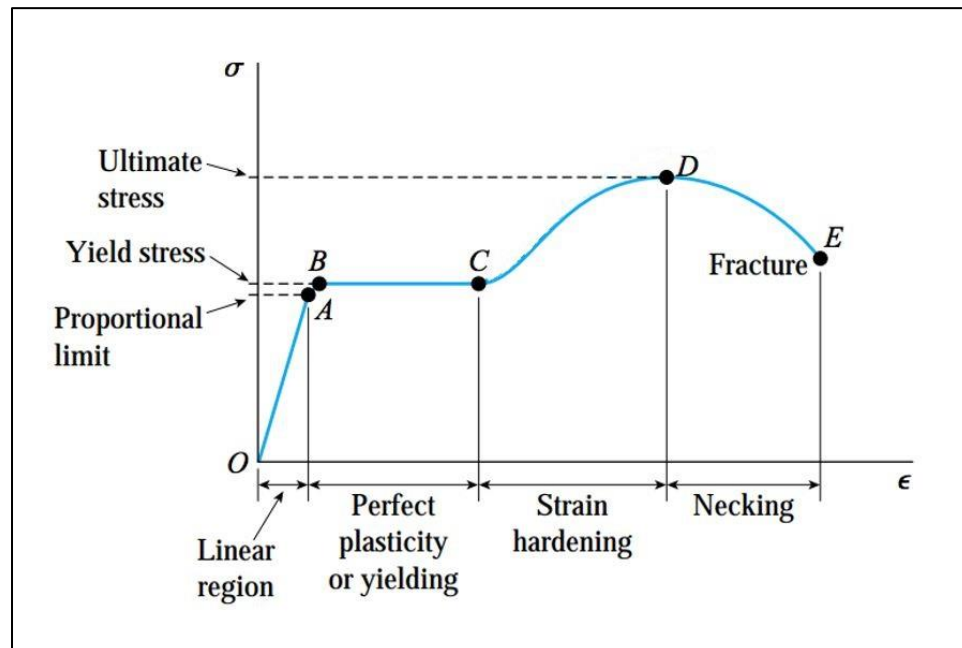


Figure 2.1: Stress vs. Strain Diagram (Gere & Goodno, 2008)

If the stress is continued to be increased, it will reach the proportional limit, and then the material enters the next region which is known as the perfect plasticity or yielding region. In this region, the material's deformation stops being proportional to the stress. The deformation rate increases until reaching the yield point, where the slope becomes practically zero and the material continues its deformation without any increase in stress.

Once the material reaches this region, if the stress is removed, the material does not return to its original shape, therefore a permanent deformation is created.

The following region in the stress vs. strain diagram is known as the strain hardening zone, where the material undergoes changes in its structure, resulting in increased strength resistance. This increase in resistance continues until the ultimate stress point is reached, where the material loses the ability to resist further load. During this process, a necking in the material is observed while it continues to deform until finally reaching the point of fracture.

In terms of the response of a bar element to lateral loads, the idealized elastic-perfectly plastic model for the material stress-strain behavior can be used, in conjunction with the Bernoulli-Navier hypothesis of beam behavior, to obtain the member plastic moment M_p . As presented in the Figure 2.2, M_p is the moment required to produce full plasticization of the cross section, that is to say, all the cross-section points are working at yielding stress.

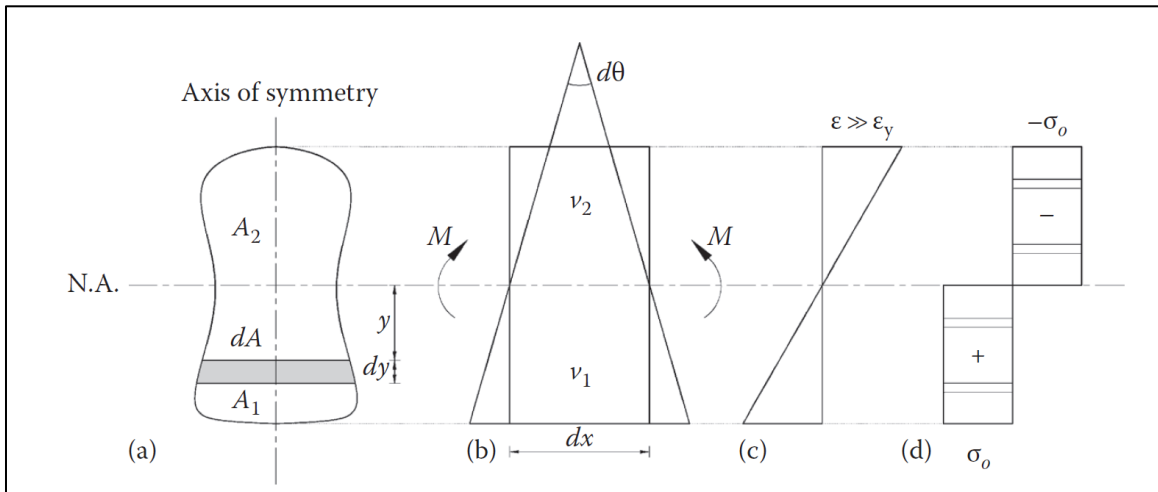


Figure 2.2: Section Plastic Moment: (a) Cross Section; (b) Segment; (c) Strain Distribution; and (d) Stress Distribution (Chen & El-Metwally, 2011)

In this model of concentrated plasticity, it is considered that, once this condition is reached, a plastic hinge is developed at the location of the cross section where M_p is acting, and that the element reached its maximum moment capacity, and the structure could start experiencing large plastic displacements and/or exhibiting failure mechanisms behavior due the plastic hinge formation.

This plastic moment M_p is the maximum moment capacity considered in steel design (that may be reduced due to lateral torsional buckling). It is included as the nominal moment capacity M_n in the interaction equations used for combined actions in beam-column members analysis. Thus, the use of this design equations is an indirect way of observing if such full plasticization condition, due to high strength demands imposed by the loading, is close to be reached in a particular member.

2.1.2 Corrosion in Structural Elements

Corrosion is the destructive attack of metal caused by it reacting to its environment (Roberge, 1999). Corrosion may cause failures to any steel structure to which an adequate maintenance has not been given. These failures may turn into costly damages and may threaten the safety of the public. Corrosion can be present in different environments. For example, steel can experience corrosion while being in direct contact with the atmosphere, with fresh water, with seawater, with soils, with reinforced concrete or with biofilms (Roberge, 1999). This section describes atmospheric corrosion, how it affects the structures, how to identify it, and how to control it.

Atmospheric corrosion occurs when a corrosive material is exposed to air and its contaminants (Roberge, 1999). For this electrochemical corrosion to take place in the atmosphere, the presence of an electrolyte is required, which, having free ions, is a good

electrical conductor and allows balancing between the anode and cathode reactions. An anode is an electrode that is positively charged, and a cathode is an electrode that is negatively charged. During the balancing of the reactions the metal loses electrons in the balancing of the reactions, starting the corrosion process.

According to the damage it produces, corrosion can be classified into three groups (Roberge, 1999):

- Group 1 – Identifiable by visual inspection
- Group 2 – Identifiable with special inspection tools
- Group 3 – Identifiable by microscopic examination

Figure 2.3 presents these three groups, with examples of different types of damages that may occur in each category.

Group 1 consists of uniform corrosion, pitting, crevice corrosion, and galvanic corrosion. Uniform corrosion is characterized by being uniformly distributed on the surface of the material and the material starts thinning until it reaches failure. Pitting is characterized by being in a specific area where cavities form on the surface. This type of corrosion is more difficult to identify; therefore, it is more dangerous than uniform corrosion. Crevice corrosion, in the same way, is a corrosion characterized by being in a specific area. This type of corrosion is formed when there is a stationary solution creating a microenvironment favorable to corrosion. Examples of areas where crevice corrosion can occur are in gaskets, washers, insulating material, surface deposits, loose coatings, threads, and clamps. Finally, the galvanic corrosion occurs when two metals are put in contact with an electrolyte in between. The metal with the noblest corrosion potential then becomes the cathode, while the less noble material is consumed in the anodic solution (Roberge, 1999).

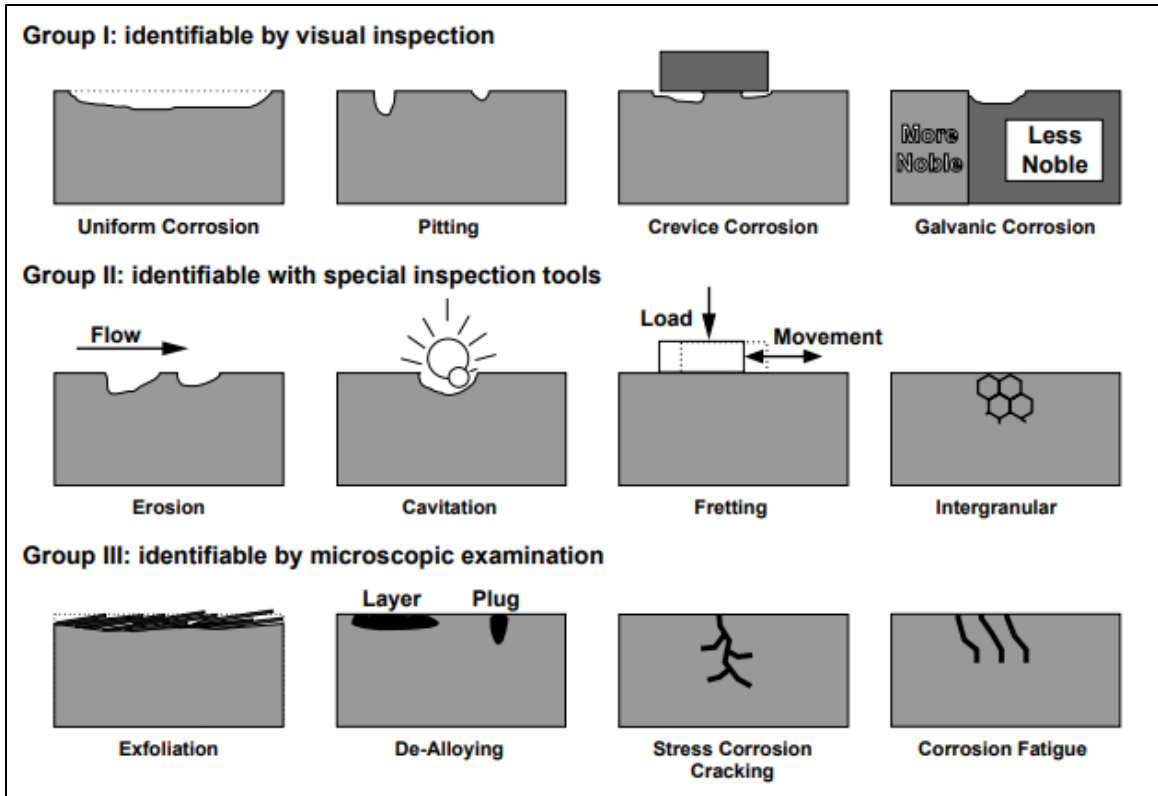


Figure 2.3: Principal Forms of Corrosion (Roberge, 1999)

The modes of failure of Groups 2 and 3 are not discussed here since they are out of the scope of this research because they cannot be identified visually and require additional methods of examination.

To avoid corrosion, it is essential that good prevention methods are carried out. For the prevention of corrosion there are five methods (Roberge, 1999):

- Switch to a more suitable material.
- Modify the material's environment.
- Use protective coatings.
- Apply cathodic or anodic protection.
- Design modifications to the system or component.

However, it is important to remember that the best protection against corrosion is to take it into consideration during the design process. Designs should take into consideration low points where the water accumulates, provide good ventilation and drainage, avoid absorbent materials, provide a good preparation of surface before installing any protective material, and implement a plan of inspection and maintenance.

2.1.3 Bridge Inspection

To facilitate bridge inspection, the *Manual for Bridge Element Inspection* (AASHTO, 2013) can be used as a reference. This manual includes a list of the elements that can be found in bridges with the different types of defects that they may experience. These elements should be evaluated during inspections to determine the condition of the bridge. This manual was used in this research as a reference to make a list of the items to be inspected in order to identify the areas where the bridge defects were located.

The manual divides the elements found in a bridge into two categories. The first is the National Bridges Elements (NBE). This category includes all the structural elements that make up a bridge and are the main elements that support the loads, e.g., columns, beams, trusses, among others. The second is the category Bridge Management Elements (BME). This category focuses on joints, surfaces with wear, and protective layers of materials.

To determine the condition of a bridge, AASHTO (2013) identifies different defects that the components of the NBE and MBE categories may have. For example, a beam can have signs of cracking, corrosion, and loose connections, among others. To evaluate a component being inspected, all the conditions found are rated using a scale of 1 to 4, where 1 represents good and 4 represents severe condition. All these defects are to be evaluated

during the field inspection. AASHTO (2013) defines the defects and indicates how to rate their severity. Examples of these are shown in Table 2-1 and Table 2-2.

Table 2-1: Example of Defect Definitions (AASHTO, 2013)

Defect Name (Number)	Used to Report	Materials
Crack (Wearing Surface) (3220)	Cracking in wearing surface elements.	Wearing Surfaces
Effectiveness (Wearing Surface) (3230)	Loss of effectiveness in the protection provided to the deck by the wearing surface elements.	Wearing Surfaces
Chalking (Steel Protective Coatings) (3410)	Chalking in metal protective coatings.	Steel/Metal Protective Coatings
Peeling/Bubbling/Cracking (Steel Protective Coatings) (3420)	Peeling, bubbling, or cracking in metal protective coatings.	Steel/Metal Protective Coatings

Table 2-2: Example of Defect Rating (AASHTO, 2013)

Defects	Condition States			
	1	2	3	4
	GOOD	FAIR	POOR	SEVERE
Delamination/Spall/Patched Area/Pothole (Wearing Surfaces) (3210)	None.	Delaminated. Spall less than 1 in. deep or less than 6 in. diameter. Patched area that is sound. Partial depth pothole.	Spall 1 in. deep or greater or 6 in. diameter or greater. Patched area that is unsound or showing distress. Full depth pothole.	The wearing surface is no longer effective.
Crack (Wearing Surface) (3220)	Width less than 0.012 in. or spacing greater than 3.0 ft.	Width 0.012–0.05 in. or spacing of 1.0–3.0 ft.	Width of more than 0.05 in. or spacing of less than 1.0 ft.	
Effectiveness (Wearing Surface) (3230)	Fully effective. No evidence of leakage or further deterioration of the protected element.	Substantially effective. Deterioration of the protected element has slowed.	Limited effectiveness. Deterioration of the protected element has progressed.	

To carry out the inspection, it is necessary to define all the components of the bridge and their quantities. For this is important to have available the drawings of the bridge, or an initial field visit may be required to quantify the different elements. Once all the elements and their quantities are defined, it can be proceeded to identify all the faults found in the bridge components by a site inspection.

2.2 Design and Modeling of Pedestrian Bridges

The topic of pedestrian bridge design was researched in order to have the necessary knowledge to perform adequate modeling of this type of structure. This section discusses modular bridges, the design loads of pedestrian bridges, the installation of signs on these structures, and modeling a bridge using a finite element program.

2.2.1 Modular Bridges

Modular bridges are bridges that are designed to be prefabricated and easy to install on site. These bridges are designed with prefabricated concrete or steel, in controlled environments. These bridges can be installed temporarily or permanently. One of the most famous bridges of this type is the Bailey Modular Bridge.

The Bailey Bridge was designed by the British engineer Donald Coleman Bailey during the Second World War. The safe, quick, and efficient way to assemble the bridge gave it the recognition of being the preferred bridge for military use. Once the Second World War ended, other adaptations for urban and rural use for the bridge were found. Currently, there are improved versions of the bridge like the Acrow Bridge in USA and the Mabey Bridge in Great Britain.

The Bailey bridge consists of two principal beams composed of trusses. Perpendicularly to the two main beams there are also transverse beams which reinforce the bridge. The deck rests on the upper part of the transverse beams. For additional horizontal reinforcement, diagonal bars are installed between the two main beams. In the same way, other reinforcing bars are installed between the transverse beams and the trusses to keep the trusses in vertical position. A main beam could consist of one, two or three trusses that

are mounted side by side. One can also add additional truss floors to increase the reach or load capacity. Figure 2.4 shows an example of the Bailey bridge.

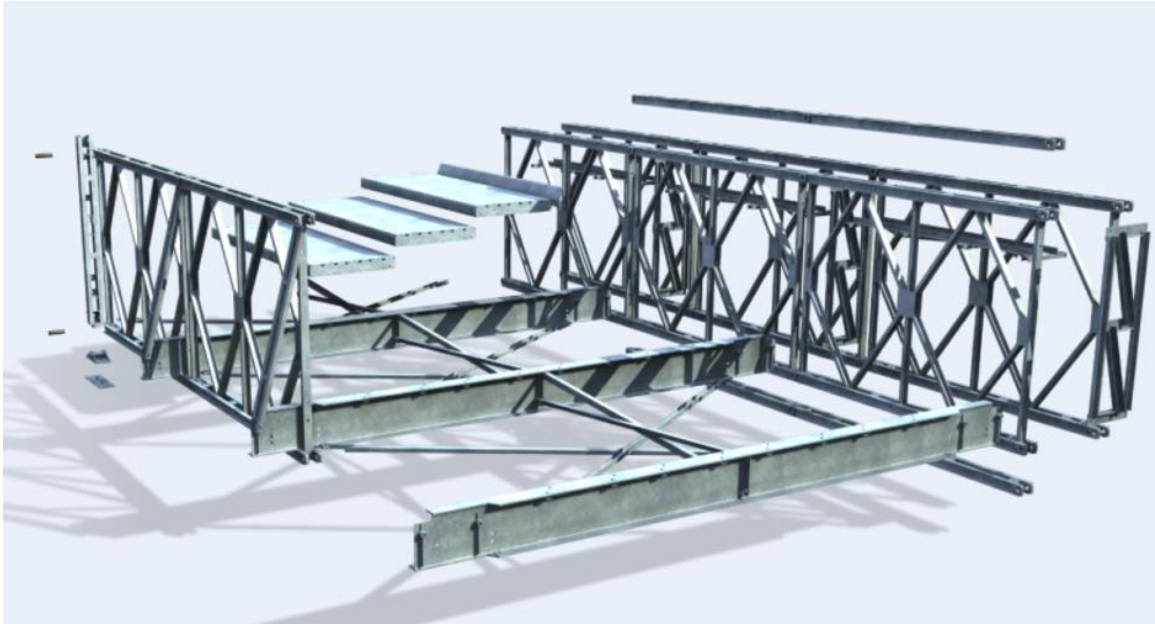


Figure 2.4: Bailey Bridge Model by Mabey (Mabey Bridge, 2019)

2.2.2 Pedestrian Bridge Design

For the design of pedestrian bridges, AASHTO developed the *LRFD Guide Specifications for the Design of Pedestrian Bridges* (AASHTO, 2009). The guide summarizes the design process for pedestrian bridges by citing other design books from AASHTO, the American Welding Society, the American Steel Construction Institute, and others.

Chapter 3 of this guide indicates the loads on the deck of the pedestrian bridge. The first load that is identified is the pedestrian live load of 90 psf. This load is distributed evenly over the length of the pedestrian bridge, and the contemplated load must be for a bridge width equal to or greater than 2 ft.

The guide continues with the vehicular load that must be contemplated in the design of a pedestrian bridges when access to vehicles is not blocked with a permanent structure,

such as concrete pedestals, or that the width of the bridge does not allow access of a vehicle. For this project's case study, the vehicular live load is not contemplated since the bridge does not have access for vehicles, not is it wide enough.

The next live load that is identified in the guide is the patch load for equestrian traffic. This patch load has a magnitude of 1,000 pounds distributed over a 4-in by 4-in square.

The next loads discussed in the guide are the ones related to wind. For wind loads, the guide refers directly to the *Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals* (AASHTO, 2015), in particular to articles 3.8 and 3.9, which covers wind loads and their effects on structures. These two sections will be discussed in the next section.

Likewise, for wind loads, the guide refers to section 3.8.2 of the *LRFD Bridge Design Specifications* (AASHTO, 2020) which indicates that the design of a pedestrian bridge must contemplate the vertical wind load on a quarter of the bridge on the windward side. The vertical load to be contemplated is 200 psf multiplied by the width of the bridge.

Table 2-3 presents the combination of loads specified in the *LRFD Bridge Design Specifications* (AASHTO, 2020), where:

DD = negative surface friction

DC = dead load of structural components and non-structural accessories

DW = dead load of wear surfaces and utilities

EH = horizontal earth pressure load

EL = cumulative effects of blocked force resulting from the construction process, including secondary forces of post-tensioning

ES = additional ground load

EV = vertical pressure of dead load of landfill

BR = vehicle braking force

CE = vehicular centrifugal force

CR = creep

FR = friction

IM = vehicle dynamic load tolerance

LL = vehicular live load

LS = surcharge for live load

PL = pedestrian live load

SH = contraction

TU = uniform temperature

WA = water load and current pressure

WL = wind live load

WS = wind load in the structure

Regarding Table 2-3, the combinations Strength II, Strength IV and Strength V do not apply according to the *Manual for Bridge Element Inspection* (AASHTO, 2013).

Table 2-3: Load Combinations and Load Factors (AASHTO, 2020)

Load Combination Limit State	<i>DC</i> <i>DD</i> <i>DW</i> <i>EH</i> <i>EV</i> <i>ES</i> <i>EL</i>	<i>LL</i> <i>IM</i> <i>CE</i> <i>BR</i> <i>PL</i> <i>LS</i>	<i>WA</i>	<i>WS</i>	<i>WL</i>	<i>FR</i>	<i>TU</i> <i>CR</i> <i>SH</i>
STRENGTH I (unless noted)	γ_p	1.75	1.00	—	—	1.00	0.50/1.20
STRENGTH II	γ_p	1.35	1.00	—	—	1.00	0.50/1.20
STRENGTH III	γ_p	—	1.00	1.40	—	1.00	0.50/1.20
STRENGTH IV	γ_p	—	1.00	—	—	1.00	0.50/1.20
STRENGTH V	γ_p	1.35	1.00	0.40	1.0	1.00	0.50/1.20
EXTREME EVENT I	γ_p	γ_{EQ}	1.00	—	—	1.00	—
EXTREME EVENT II	γ_p	0.50	1.00	—	—	1.00	—
SERVICE I	1.00	1.00	1.00	0.30	1.0	1.00	1.00/1.20
SERVICE II	1.00	1.30	1.00	—	—	1.00	1.00/1.20
SERVICE III	1.00	0.80	1.00	—	—	1.00	1.00/1.20
SERVICE IV	1.00	—	1.00	0.70	—	1.00	1.00/1.20

2.2.3 Installation of Road Signs

For the installation of road signage in existing structures, a specific design must be developed for the structure that will hold the sign in place. For this purpose, AASHTO developed specifications for structures supporting signs, traffic signals and luminaires (AASHTO, 2015). As mentioned in the previous section, these specifications must be used for the calculation of wind loads on pedestrian bridges, even if the bridge is not supporting signs, traffic signals or luminaires. These specifications consider dead loads (*DL*), ice loads, wind loads (*W*) and fatigue loads. The combinations for these loads are presented in Table 2-4.

Table 2-4: Load Combinations for Structures Supporting Signs (AASHTO, 2015)

Group Load	Load Combination ^a	Percentage of Allowable Stress ^b
I	<i>DL</i>	100
II	<i>DL + W</i>	133
III	<i>DL + Ice + 1/2(W)</i> ^c	133
IV	Fatigue	^d

Regarding wind loads, the design wind pressure is calculated as follows:

$$P_z = 0.00256K_zK_dGV^2C_d \quad (3)$$

where:

P_z = design wind pressure in pounds per square feet (psf)

K_z = height and exposure factor

K_d = directionality factor

G = gust effect factor

V = basic wind speed, expressed as a 3-s gust wind speed, at 33 ft above the ground in open terrain, in miles per hours (mph)

C_d = drag coefficient

Equation (3) is based on the formulation developed for and presented in the ASCE/SEI 7 Standard. The AASHTO specifications indicate how to calculate or determine the different parameters of Equation (3).

2.2.4 Bridge Modeling

There are several finite element programs that may be used to model bridges. Among the programs with commercial licenses are Abaqus, ANSYS, LUSAS, Midas, CSiBridge, and SAP2000. These programs include tutorials that help the user to understand how to perform the modeling of bridges.

However, at the time of modeling, a limiting factor is the availability and accessibility of the program. For this reason, open-source programs are also an option. There are different open-source programs to perform non-linear analyzes such as Elmer and OpenSEES.

For the current project, due to its robustness and availability, SAP2000 v21 was selected to perform the comparative analysis of the bridge response to wind loads.

3 Methodology

A brief description of the steps followed to develop this research project is presented in this chapter. Also, some additional considerations to the methodology are mentioned.

3.1 General Procedure

The following list briefly describes the process followed in this project to achieve the research objective:

1. Perform a literature review on the type of bridges to be analyzed.
2. Collect background information and available data on the specific bridge to be studied.
3. Develop an inspection list using the *Manual for Bridge Element Inspection* (AASHTO, 2013).
4. Perform a field visual inspection of the bridge in the field to determine the condition of the elements of the bridge and confirm that there are no existing failures that influenced the bridge deformation. This inspection was carried out using the list that was made in step 3.
5. Obtain the geometry of the bridge performing a detail survey. A total station and a drone survey (that obtains high-quality, orthorectified images) were used.
6. Select a finite element program for the structural analysis. The program selected was SAP2000 v21.
7. With the field data that was obtained, develop a finite element model using the program that was chosen in step 6.

8. Model the bridge without the signage and observe the behavior of the pedestrian bridge to see if it fails when the wind pressures, equivalent to those produced by Hurricane Maria, are applied.
9. Model the bridge with the signage and observe the behavior of the pedestrian bridge to see if it fails when the wind pressures, equivalent to those produced by Hurricane Maria, are applied.
10. Compare the behaviors of the bridge obtained in steps 8 and 9. With the comparison of these results, assess if the signs presence was the main reason for the inelastic deformation that had occurred.
11. Offer recommendations regarding the installation of signs on modular pedestrian bridges.

3.2 Additional Considerations

The condition observed in the pedestrian bridge during the inspection (such as the large amount of corrosion in several elements) and other findings (such as bridge support details, and signs support details) that may have played a role in the lateral deflections are also described in the results analysis and conclusions.

This research will help future assessments to be carried out on different bridges around Puerto Rico to which road signs were added without the proper reinforcement of the original structure.

4 Bridge Information and Previous Studies

During this research phase, the Puerto Rico Highway and Transportation Authority (PRHTA) of the Puerto Rico Department of Transportation and Public Works (PRDOT) was visited to obtain all the available information on the bridge being studied. It was found that the bridge is identified as Pedestrian Bridge (PB) 1137 and that it was constructed in 1968. The following sections include information about its location, and the recovered drawings and inspection reports.

4.1 Location

PB 1137 was located at kilometer 4.1 of the Roman Baldorioty de Castro Expressway, San Juan, Puerto Rico. The bridge connected the Norte Shopping Center and the Luis Lloréns Torres public housing complex located north of the expressway with the Villa Palmeras sector south of the expressway.

Figure 4.1 presents an aerial view of the area. In the figure, the area north of the expressway, that includes the Norte Shopping Center and the Luis Lloréns Torres complex, are shaded blue. The area south of the expressway that includes the sector Villa Palmeras is shaded yellow. The bridge has been shaded red and finally the expressway is marked with a green line.

Figure 4.2 presents a view of the bridge from March 2016, while Figure 4.3 presents a view of the bridge after the damages produced by Hurricane Maria. Notice that at some point between March 2016 and the passage of Hurricane Maria in September 2017, a sign on the bridge was replaced with multiple signs. At the time of the field visit, all the signs had been removed.



Figure 4.1: Aerial View of the Bridge Location (Source: Google Maps)



Figure 4.2: Image of the Bridge from March 2016 (Source: Google Maps)



Figure 4.3: Image of the Bridge from September 2018 (Provided by PRHTA)

4.2 Drawings Recovered from Puerto Rico Department of Transportation

During the visits that were performed to the Department of Transportation, copies of two different set of drawings were obtained. The first set of drawings obtained was from 1976, and the second set of drawings acquired was from 2001. The 2001 drawings are presented in Appendix A. Since the bridge was built in 1968, based on the inspection reports, both sets of drawings are as-builts.

The set of drawings from 2001 includes one drawing for the existing condition, one drawing for the proposed layout and one drawing for the sections and details. Since one of the drawings is a proposed layout, it is understood that this set of drawings was for a remodeling of the bridge. Even though this set of drawings is for a remodeling of the bridge, the existing condition drawing is still different from the existing condition drawing of 1976. Meaning that between 1976 and 2001 there might have been another bridge remodeling for which information was not obtained.

Comparing the existing layout of the 1976 drawings with the one from the 2001 drawings, the bridge was extended from 52.140 meters to 59.585 meters. Additionally, the proposed layout modifications of the 2001 drawing were confirmed to be completed during the site inspections.

4.3 Previous Inspections from Puerto Rico Department of Transportation

During the visits that were done to the Puerto Rico Department of Transportation, copies of different inspection reports of the bridge were obtained. The inspection reports date from 1972 to the most recent of May 2017, just a few months before Hurricane Maria made landfall in Puerto Rico. A total of 13 reports were obtained. There are two major gaps of inspection reports, either because the reports were not saved or because the report was never prepared. These gaps are from 1975 to 1986 and from 2001 to 2014. Outside of these major gaps, the time between reports ranged from one to three years.

The inspection reports have the structural information of the bridge and the evaluation of the condition of three mayor areas: the deck, the superstructure, and the substructure. The evaluation is based on a 0 to 9 scale system, where 9 represents the best evaluation and 0 represents the poorest evaluation. In addition to the condition evaluation, the inspection also consisted in appraising the structural condition of the structure with the same point system. The inspection reports end with recommendations based on the findings. Table 4-1 summarizes the findings of reports that where recovered. In this table, Item 58 represents the deck, Item 59 represents the superstructure, Item 60 represents the substructure and Item 67 represents the structural condition.

Table 4-1: Summary of Previous Inspection Reports

Year	58	59	60	67	Remarks (as included in the reports)
1972	7	7	8	7	Rehabilitation should include replacing missing bolts and safety pins and also painting
1975	8	7	7	7	Rehabilitation should include: (a) Removal of rust from structural steel by means of wire brush or any other acceptable method and the protection of it with paint; (b) The replacement of any structural element if necessary
1986	7	7	7	7	Rehabilitation consists of Maintenance, sand blasting and painting
1987	6	6	7	7	Rehabilitation consists of repairing light corrosion at steel plate. The structure was painted recently. Fine cracking, small spalling and light corrosion at footing and columns respectfully.
1989	6	6	6	6	Rehabilitation consists of repairing ponding, holes, and rust stains at deck steel plates. Severe corrosion at angles. Moderate corrosion at some structural members. Moderate Corrosion at columns. Fine crack, small spalling, and light scaling at footings. Stair railings are broken due to moderate corrosion.
1991	5	5	5	5	Severe Corrosion
1993	6	6	6	6	This bridge was rehabilitated last year (1992), and no improvement is needed
1994	5	5	6	5	Rehabilitation consists of repairing severe corrosion and light section loss at wearing surface due to ponding. Fine cracks, small spallings and light scaling at columns footings. Ponding at stairs and rest area.
1996	5	5	6	5	Rehabilitation consists of repairing light corrosion and light section loss. Bridge was painted
1999	5	5	6	5	Rehabilitation consists of repairing severe corrosion and light section loss at wearing surface due to ponding. Fine cracks, small spallings and light scaling at columns footings. Ponding at stairs and rest area.
2001	5	5	6	5	Rehabilitation consists of repairing severe corrosion and light section loss at wearing surface due to ponding. Fine cracks, small spallings and light scaling at columns footings. Ponding at stairs and rest area.
2014	5	5	6	/	Light to moderate corrosion and some holes by corrosion at steel plates. Lateral components of bracings show light to moderate corrosion and section loss. Moderate section loss by corrosion at upper chords.
2017	4	5	5	/	There is moderate and severe corrosion on the deck surface, including several large perforations. The deck surface of the structure is in poor condition. Several of the lateral components of the "bracing" have light, moderate and severe corrosion, and loss of section. Some sections in the upper chord have moderate loss due to corrosion. Steel columns in good condition, light corrosion

From Table 4-1 it may be appreciated that the substructure was, in the most cases, considered in a better condition than the deck and the superstructure. It is also appreciated

that, as the years passed, the condition of the structure for the three major areas kept on deteriorating. In the recommendations, in 1987 and 1993, the inspection reports mentioned that the bridge was recently rehabilitated. Nonetheless, with the exception of the report of 1993, all the other reports mentioned that corrosion was present, and that it needed to be repaired.

The corrosion emphasis grew per report and, starting with the report of 1994, the section loss due to corrosion was evident. The final report had the deck condition set as 4 and the superstructure and substructure set as 5, which meant that the bridge was in poor condition.

5 Visual Inspection and Condition Evaluation

In this chapter, the performed visual inspections are described and the corresponding findings are discussed. For the condition evaluation, the lateral deformation of the bridge was not considered.

5.1 Site Visits

As part of the inspection of the bridge, different site visits were conducted. On the first visit it was observed that the bridge was closed to pedestrians because of its poor structural condition. An expanded metal mesh was welded against the steel frames of the bridge and a banner indicating the bridge was closed was installed, as shown in Figure 5.1 and Figure 5.2. During this visit, different photos of the bridge were taken and the dimensions of the elements that were accessible were measured. In addition, a Total Station (shown in Figure 5.3 and Figure 5.4) was used to take points along the bridge to delineate the bridge's lateral displacement. When the points taken were traced in AutoCAD, no significant deflection was observed, thus not enough points during the measuring process were collected to capture the deformation. Since the deflection was not captured with the Total Station, it was decided that another method had to be used.

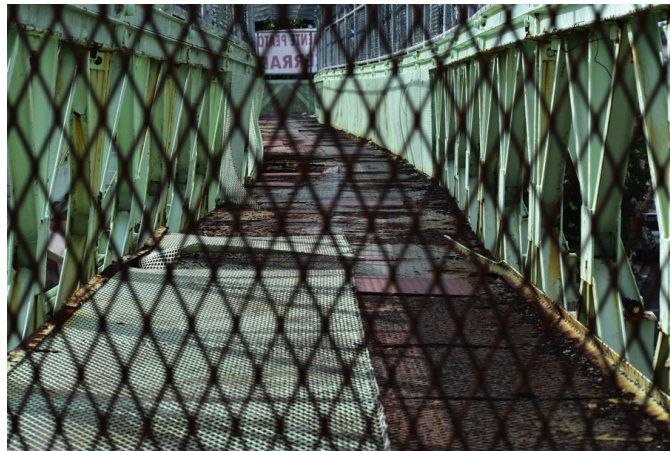


Figure 5.1: Metal Mesh Used to Close Pedestrian Bridge



Figure 5.2: Sign Indicating Pedestrian Bridge is Closed



Figure 5.3: Total Station



Figure 5.4: Total Station Positioned to Measure Bridge

During the second inspection, a DJI PHANTOM drone (as shown in Figure 5.5) was used to take aerial photos. The photos taken during this visit (an example is shown in Figure 5.6) were of low quality probably because the drone flight was done rapidly because of security concerns. It was decided that another flight was needed.



Figure 5.5: Drone Operation at Beginning of Flight



Figure 5.6: Aerial Photograph of the Pedestrian Bridge

For the third site visit, the Administration of the Public Housing Residency was visited, and permission was asked to enter the area and to stay several hours. Security was offered during the site inspection. During this final site visit, another drone flight was made, and a series of high quality and high-definition photographs were taken. Also, closer images were taken so that the structural elements could be analyzed for their condition assessment directly from the photos. Every picture taken up-close included two to three truss panels. An example of this is presented in Figure 5.7. More photos taken of the bridge are presented in Appendix B.

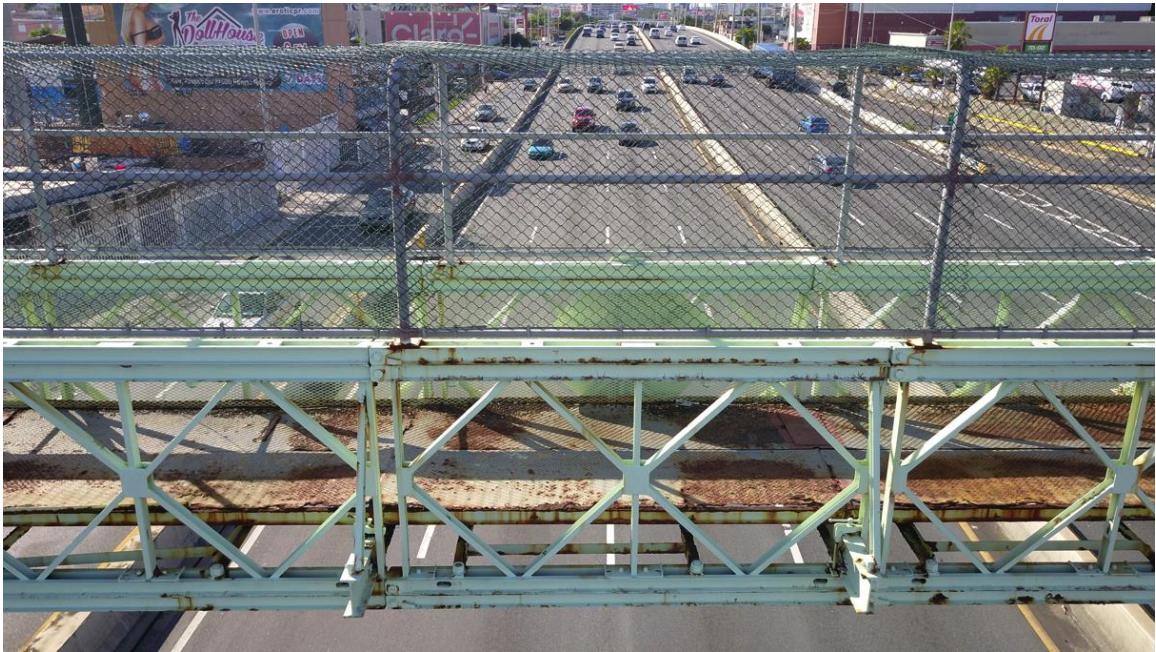


Figure 5.7: Up-close Photo of the Bridge

The series of aerial photos that were taken were orthorectified, meaning that the photos were geometrically corrected so that the scale of the photo is uniform. After this procedure, the photos that were taken were assembled to form a map and real measurements were obtained from the aerial photos.

5.2 Form creation for bridge condition evaluation

Using as a basis the *Manual for Bridge Element Inspection* (AASHTO, 2013), a checklist form was created with the elements that could be found in the pedestrian bridge that was being evaluated. The inspection manual also includes different failures that may occur per structural elements, and they were also taken into consideration.

The elements that were to be inspected were determined from Chapter 2 of the manual. The first element that was determined to be inspected was obtained from Table 2.1.1 of the manual, reprinted in this report as Table 5-1. From this table, the steel deck was the only element that was determined to be inspected. In the manual, this element has the identification number 30 and is measured in square feet.

Table 5-1: Decks and Slabs (AASHTO, 2013)

Element	Units	Decks	Slab	Other
Reinforced Concrete Deck/Slab	area, ft ²	12	38	
Prestressed Concrete Deck	area, ft ²	13		
Prestressed Concrete Top Flange	area, ft ²	15		
Reinforced Concrete Top Flange	area, ft ²	16		
Steel Deck—Open Grid	area, ft ²	28		
Steel Deck—Concrete Filled Grid	area, ft ²	29		
Steel Deck—Corrugated/ Orthotropic/Etc.	area, ft ²	30		
Timber Deck/Slab	area, ft ²	31	54	
Other Material Deck/Slab	area, ft ²	60	65	

The second, third and fourth elements that were determined were obtained from Table 2.1.3 of the manual, reprinted in this report as Table 5-2. From this table, the trusses, the floor beams, and the pins were determined. In the manual the truss and the floor beams are measured by length and their identifications numbers are 120 and 152, respectively. The

pins identification number is 161 and they are quantified by the total number of units the bridge contains.

Table 5-2: Superstructure (AASHTO, 2013)

Element	Units	Steel	Prestressed Concrete	Reinforced Concrete	Timber	Masonry	Other
Girder/Beam	length, ft	107	109	110	111		112
Closed Web/Box Girder	length, ft	102	104	105			106
Stringer	length, ft	113	115	116	117		118
Truss	length, ft	120			135		136
Arch	length, ft	141	143	144	146	145	142
Floor Beam	length, ft	152	154	155	156		157
Cable—Primary	length, ft	147					
Cable—Secondary	each	148					149
Gusset Plate	each	162					
Pin, Pin and Hanger Assembly, or Both	each	161					

The fifth and sixth elements that were determined to be inspected were obtained from Table 2.1.5 of the manual, reprinted in this report as Table 5-3. From this table the columns and the column tower were determined to be inspected. The columns identification number is 202 and the number of units quantify it. The column tower identification number is 207 and they are measured by height.

Table 5-3: Substructure (AASHTO, 2013)

Element	Units	Steel	Prestressed Concrete	Reinforced Concrete	Timber	Masonry	Other
Columns	each	202	204	205	206		203
Column Tower (Trestle)	length, ft	207			208		
Pier Wall	length, ft			210	212	213	211
Abutment	length, ft	219		215	216	217	218
Pile	each	225	226	227	228		229
Pier Cap	length, ft	231	233	234	235		236
Pile Cap/Footing	length, ft			220			

Finally, the seventh and eighth elements that were determined were obtained from Table 2.2.3 of the manual, reprinted here as Table 5-4. From this table, the wearing surfaces and steel protective coating were determined to be inspected. In the manual these elements have the identification numbers 510 and 515, respectively, and are measured in square feet.

Table 5-4: Wearing Surfaces and Protective Coatings (AASHTO, 2013)

Element	Units	Element Number
Wearing Surfaces	area, ft ²	510
Steel Protective Coating	area, ft ²	515
Concrete Reinforcing Steel Protective System	area, ft ²	520
Concrete Protective Coating	area, ft ²	521

After determining the elements that were to be inspected, Chapter 3 of the manual was used to choose the conditions for which each element would be evaluated. The conditions chosen, with their corresponding identification numbers, were the following: corrosion (1000), connection (1020), distortion (1900), damage (7000), chalking (3410) and effectiveness (3440). These conditions are evaluated in a point system going from 1 to 4, where 1 is good and 4 is severe. An example of this evaluation may be seen in Table 5-5, which illustrates the different defects and how they are categorized. Note that this scale is different from the one used in previous inspections reports presented in the section 4.3, where the evaluation is done from 0 to 9 where, 0 is bad and 9 is very good.

With all the elements and conditions established, a table based on the Table B3.3 of the manual was developed.

Table 5-6 includes all the elements that were decided to be evaluated.

Table 5-5: Condition State Definitions for Steel Deck (AASHTO, 2013)

Defects	Condition States			
	1	2	3	4
	GOOD	FAIR	POOR	SEVERE
Corrosion (1000)	None.	Freckled rust. Corrosion of the steel has initiated.	Section loss is evident or pack rust is present but does not warrant structural review.	The condition warrants a structural review to determine the effect on strength or serviceability of the element or bridge; OR a structural review has been completed and the defects impact strength or serviceability of the element or bridge.
Cracking (1010)	None.	Crack that has self-arrested or has been arrested with effective arrest holes, doubling plates, or similar.	Identified crack that is not arrested but does not warrant structural review.	
Connection (1020)	Connection is in place and functioning as intended.	Loose fasteners or pack rust without distortion is present but the connection is in place and functioning as intended.	Missing bolts, rivets, or fasteners; broken welds; or pack rust with distortion but does not warrant a structural review.	
Damage (7000)	Not applicable.	The element has impact damage. The specific damage caused by the impact has been captured in Condition State 2 under the appropriate material defect entry.	The element has impact damage. The specific damage caused by the impact has been captured in Condition State 3 under the appropriate material defect entry.	The element has impact damage. The specific damage caused by the impact has been captured in Condition State 4 under the appropriate material defect entry.

Table 5-6: Element Quantity and Condition State Summary

Element Type	Area	Element Number	Element Description	Unit of Measure	Total Quantity	Condition State 1	Condition State 2	Condition State 3	Condition State 4	Defect #	
National Bridge Elements (NBS)	Decks and Slabs	30	Steel Deck	area, ft ²							
		1000	Corrosion	area, ft ²							
		1020	Connection	area, ft ²							
	Superstructure		120	Truss	length, ft						
			1000	Corrosion	length, ft						
			1020	Connection	length, ft						
			1900	Distortion	length, ft						
			7000	Damage	length, ft						
			152	Floor Beam	length, ft						
			1000	Corrosion	length, ft						
			1020	Connection	length, ft						
			161	Pin	each						
			1000	Corrosion	each						
	1020	Connection	each								
	Substructure		202	Columns	each						
1020			Connection	each							
207			Column Tower	length, ft							
1000			Corrosion	length, ft							
Bridge Management Elements (BMS)	Wearing Surfaces and Protective Coatings	1020	Connection	length, ft							
		510	Wearing Surfaces	area, ft ²							
		515	Steel Protective Coating	area, ft ²							
		3410	Chalking	area, ft ²							
		3440	Effectiveness	area, ft ²							

5.3 Assessment

To complete the form presented on

Table 5-6, all the elements had to be quantified and visually given a value condition from 1 to 4. To determine the quantity of the elements, the following computations were made:

- Deck
 - The deck is composed of steel plates.
 - Steel Deck (Element 30): Quantity 204 ft (in length) x 5.5 ft (in width)
= 1,122 ft²
- Superstructure
 - The superstructure is composed of trusses, floor beams and pins.
 - Truss (Element 120) Quantity: 9 ft (in length) x 2 truss (per span) x 20 (spans) = 360 ft
 - Floor Beam (Element 152) Quantity: 9 ft (in length) x 1 beam (per span) x 20 (spans) = 180 ft
 - Pins (Element 161) Quantity: 8 (pins per span) x 20 (spans) = 160 pins
- Substructure
 - The substructure is composed of columns and columns towers.
 - Columns (Element 202) Quantity: 4 in total
 - Column Towers (Element 207) Quantity: 2 in total
- Protective Coating
 - The surface of the structure is completely painted.

- Steel Protective Coating (Element 515) Quantity: (Calculated from "as-built" drawings presented in Appendix A) = 8,490 ft²
- To determine the area of the structure, Table 5-7 was created.

Table 5-7: Surface Area

Group	Element	Qty	Sections	Total Qty	From Drawing		Surface [in]	Total Surface [in]	Total Area [sq. in.]	Total Area [sq. ft.]	Total [sq. ft.]
					Nominal Size	Length [in]					
Truss Panel	Longitudinal Channel Top 1	2	40	80	4" x 2" x 1/4"	116	8	16	1856	12.89	1031.11
	Longitudinal Channel Top 2	2	40	80	4" x 2" x 1/4"	116	8	16	1856	12.89	1031.11
	Longitudinal Channel Bottom 1	2	40	80	4" x 2" x 1/4"	116	8	16	1856	12.89	1031.11
	Longitudinal Channel Bottom 2	2	40	80	4" x 2" x 1/4"	116	8	16	1856	12.89	1031.11
	Vertical Channel	3	40	120	3" x 1" x 1/8"	57	5	10	570	3.96	475.00
	Diagonal Channel	2	40	80	3" x 1-1/2" x 1/8"	41	6	12	492	3.42	273.33
	Diagonal Channel	2	40	80	3" x 1-1/2" x 1/8"	41	6	12	492	3.42	273.33
	Diagonal Channel	2	40	80	3" x 1-1/2" x 1/8"	41	6	12	492	3.42	273.33
	Diagonal Channel	2	40	80	3" x 1-1/2" x 1/8"	41	6	12	492	3.42	273.33
	Reinforcing Plate	1	40	40	9" x 9"	9	9	18	162	1.13	45.00
Reinforcing Plate	2	40	80	9" x 4-1/2"	4.5	9	18	81	0.56	45.00	
Floor	Transverse Beam	1	20	20	10" x 5" x 5/16"	108	20	40	4320	30.00	600.00
	Diagonal Bracing Channel	2	20	40	3" x 1-1/2" x 1/8"	51.5	6	12	618	4.29	171.67
	Transverse Bracing Channel	4	20	80	4" x 1-1/2" x 1/8"	75	7	14	1050	7.29	583.33
	Longitudinal Channel	4	20	80	4" x 1-1/2" x 1/4"	116	7	14	1624	11.28	902.22
	Transverse Angle Bracing	2	20	40	3" x 3" x 1/8"	75	6	12	900	6.25	250.00
	Longitudinal Angle Bracing	2	20	40	3" x 3" x 1/8"	60	6	12	720	5.00	200.00
Total										8490.00	

With the quantities established, the bridge was evaluated. The results from the evaluation are presented in Table 5-8. Following is a discussion of how these results were obtained.

Table 5-8: Element Quantity and Condition State Summary Evaluate

Element Type	Area	Element Number	Element Description	Unit of Measure	Total Quantity	Condition State 1	Condition State 2	Condition State 3	Condition State 4	Defect #	
National Bridge Elements (NBS)	Decks and Slabs	30	Steel Deck	area, ft ²	1,122					4	
		1000	Corrosion	area, ft ²	1,122				1,122	4	
		1020	Connection	area, ft ²	1,122				1,122	4	
	Superstructure	120	Truss	length, ft	360						4
		1000	Corrosion	length, ft	360			198	162		3,4
		1020	Connection	length, ft	360			198	162		2,3
		1900	Distortion	length, ft	360	148				212	1,4
		7000	Damage	length, ft	360	360					1
		152	Floor Beam	length, ft	180						2
		1000	Corrosion	length, ft	180	117	63				1,2
		1020	Connection	length, ft	180	144	36				1,2
		161	Pin	each	160						1
		1000	Corrosion	each	160	160					1
		1020	Connection	each	160	160					1
		Substructure	202	Columns	each	4					
	1020		Connection	each	4	4					1
	207		Column Tower	length, ft	27						4
	1000		Corrosion	length, ft	27		27				2
	1020		Connection	length, ft	27					27	4
	Bridge Management Elements (BMS)	Wearing Surfaces and Protective Coatings	510	Wearing Surfaces	area, ft ²	8,490					N/A
515			Steel Protective Coating	area, ft ²	8,490					3	
3410			Chalking	area, ft ²	8,490		8,490			2	
3440			Effectiveness	area, ft ²	8,490			8,490		3	

To evaluate the pedestrian bridge, it is important to understand the points system.

This is summarized as follows (AASHTO, 2013):

- 1 - Good - No observation is made.
- 2 - Fair - Corrosion has initiated, loose fasteners, distortion not requiring mitigation, the surface is substantially effective.
- 3 - Poor - Section loss is evident, missing elements, distortion that requires mitigation, the surface has limited effectiveness.
- 4 - Severe - The condition warrants a structural review to determine the effect on strength or serviceability of the element or bridge.

Based on this points system, the following overall condition was given to the structure:

- Deck - Condition State 4 - It was evaluated in this condition due to the corrosion along the complete deck, causing some holes. Also due to corrosion, some bracing elements had fallen off. Falling elements are a major threat to safety since the bridge is located above a principal roadway of Puerto Rico. Figure 5.8 shows that corrosion had already eaten up part of the steel deck surface (as appreciated by the sunlight passing through the deck) and that diagonal bracings have fallen off. The type of diagonal bracing that is missing in Figure 5.8, can be somewhat appreciated in Figure 5.9, which itself shows an incomplete set of diagonals.



Figure 5.8: Loss of Decking due to Corrosion and Missing Bracing Elements



Figure 5.9: Missing Bracing Elements

- Superstructure - Condition State 4 - Even though the superstructure consists of three different items (the truss, the floor beam, and the pins), the overall condition given to this item was 4 due to the evident corrosion along the truss elements, including section loss. Since the truss was determined in condition

state 4, the whole superstructure was determined as this state. This determination was based on if a truss element fails, it will compromise the integrity of the whole structure. In Figure 5.10, Figure 5.11 and Figure 5.12 the uniform corrosion along the truss may be seen. Going even further, in Figure 5.11 and Figure 5.12 some areas have holes in the channel of the truss due to the corrosion. Because of this level of corrosion, the channels were given a condition state 4. In Figure 5.13 and Figure 5.14 a uniform corrosion may be seen on top of the beam and crevice corrosion may be seen where the beam is in contact with the channel. It is evident that corrosion is present, but since the bridge was closed at the time of the inspection, a closer observation was not possible. From afar, section loss of the beams was not evident, so they were evaluated as condition state 2.



Figure 5.10: Corrosion in Truss Element (Example 1)



Figure 5.11: Corrosion in Truss Element (Example 2)



Figure 5.12: Corrosion in Truss Element (Example 3)



Figure 5.13: Corrosion in Floor Beams (Example 1)



Figure 5.14: Corrosion in Floor Beams (Example 2)

- Substructure - Condition State 4 - The substructure consists of the columns and the column tower. Even though the columns were found in good condition, the column towers were missing connections. Due to the missing connections the overall condition given to the substructure was 4, as in the case of the superstructure. In Figure 5.15 and Figure 5.16 a loose connection and a section loss due to corrosion are presented, respectively.



Figure 5.15: Loose Connection at Column Tower



Figure 5.16: Section Loss due to Corrosion at Column Tower

- Wearing Surfaces - Condition State 3 - This item was evaluated in the condition state 3 because of the limited effectiveness the paint has, due to the evident corrosion in the structure, as it may be appreciated in Figure 5.17.



Figure 5.17: Painting Effectiveness in Corrosion

Appendix E of the element inspection manual indicates different actions that may be taken given a state condition, as follows:

- 1 - Good - Do nothing; Protect.
- 2 - Fair - Do nothing; Protect; Repair.
- 3 - Poor - Do nothing; Protect; Repair; Rehabilitate; Replace.
- 4 - Severe - Do nothing; Protect; Repair; Rehabilitate; Replace.

Based on these indications, it was determined that for the deck, replacement would be the best action due to the section loss of the steel deck, the pedestrian safety could not be assured. On the other hand, it is evident that the superstructure merits a structural review. Once a structural review has been carried out, if the pedestrian bridge may still be used, repair or replacement of the compromised sections would be required. For the substructure, repair and rehabilitation would be needed. Finally, for the wearing surfaces, it would be necessary to repair by removing the corrosion and rehabilitation by application of a new layer of protective coating.

5.4 Comparing Results

With the evaluation of the bridge condition completed, the values obtained were compared with those presented in the latest inspection report recovered from the Department of Transportation of Puerto Rico dated May 2017. To be able to carry the comparison, a relationship needed to be established between the 1-to-4-point system used in this report with the 0-to-9 scale used in PRDOT inspection reports. The relationship established between the two systems is presented in Table 5-9. For example, it was determined that a Good Condition corresponding to the number 1 on the 1-to-4 scale corresponds to a value on 7 to 9 in the 0-to-9 scale.

Table 5-9: Equivalency Between Current and PRDOT Condition State Criteria

Condition	Current Inspection	PRDOT Inspection
Good	1	9 - 7
Fair	2	6 - 5
Poor	3	4 - 3
Severe	4	2 - 0

Having established the relationship between the different rating systems, Table 5-10 presents a comparison between the assessment of the May 2017 inspection report by PRDOT and the evaluation carried out for this project in 2018. It can be seen that for the 2017 inspection the condition state given to the bridge was poor, while for this project the condition state was determined to be severe. It is apparent that the degradation of the bridge due to corrosion was accelerating, and although the current evaluation could have been too strict, it is evident that immediate action was required for the bridge.

Table 5-10: Condition State Comparison

Inspection	Deck	Superstructure	Substructure
2017 (Previous)	4 (Poor)	5 (Fair)	5 (Fair)
2018 (Current)	4 (Severe)	4 (Severe)	4 (Severe)

6 Assessment of Bridge Deflected Geometry

The surveying of the bridge was performed with a DJI Phantom Drone, as mentioned in Chapter 5. The following two sections will describe how the surveying of the bridge was done using the drone.

6.1 Procedure

The use of drones for surveying has become more practical due to the advancement in image processing and GIS/Mapping software. Aerial mapping is now done with the use of drones and the orthorectified images obtained gives accurate surveying dimensions. For the surveying of the pedestrian bridge, a DJI Phantom Drone was used (see Figure 6.1). Using the software PIX4D, first the area in which the mapping will be done must be selected. The selection is done directly from the smart device that is used to fly the drone. Once the area is selected, the photo mapping starts.



Figure 6.1: DJI Phantom Drone (Source: DJI)

During the mapping, the drone flew over the pedestrian bridge taking pictures of the bridge every few feet. Approximately 100 pictures were taken overall. These pictures were then processed with PIX4D software. The software overlaid the pictures taken, creating a map of the area. Then, the same software orthorectifies their dimension creating an

accurately dimensioned map. Figure 6.2 illustrates the result of the orthorectified-mapping image obtained from PIX4D.

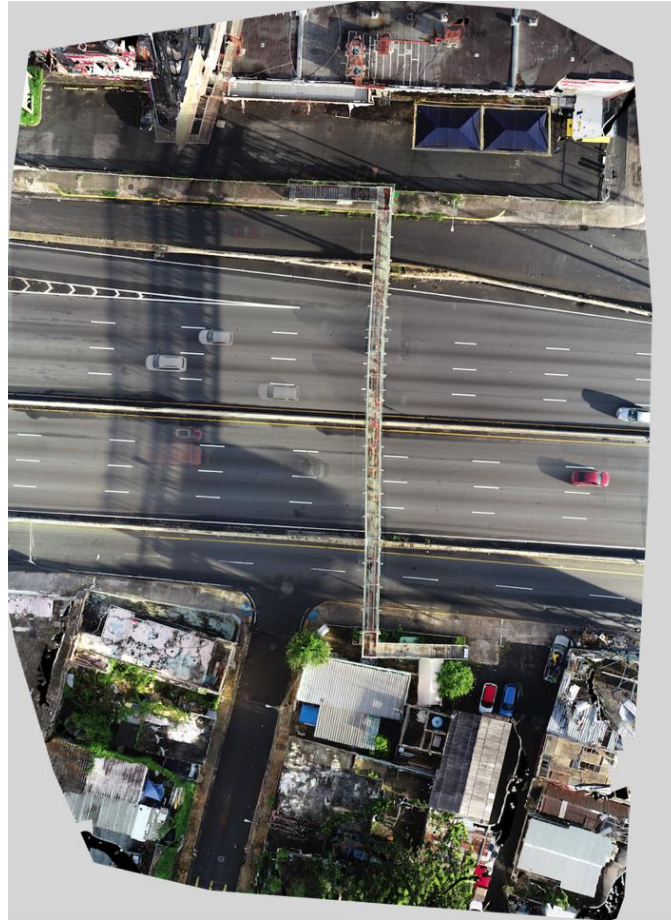


Figure 6.2: Areal Mapping of the Pedestrian Bridge

6.2 Outcomes

With the image obtained from the aerial mapping, measurements of the bridge may be obtained. First, the image was compared to the aerial image that was retrieved from Google Maps. In Figure 6.3, the image from Google Maps is compared to the one obtained from the survey. A red line was overlaid on both images to make the lateral deformation of the bridge after Hurricane Maria more noticeable. To accurately determine the lateral deformation of the bridge, the image obtained from the survey was imported to AutoCAD.



Figure 6.3: 2016 (Google Earth) and 2018 (Drone Survey) Bridge Geometry Comparison

Using Google Earth, the dimension of the bridge was verified, as illustrated in Figure 6.4. The length obtained from Google Earth was 202 ft. Considering that the bridge has a length equivalent to 20 truss panels, and every truss panel measures approximately 10 ft, the distance obtained from Google Earth is considered accurate. The distance of 10 feet of the truss panel was verified in the field, and it is also the same length indicated in an old manual for the Bailey Bridge (U.S. War Department, 1943). With the surveyed image imported to AutoCAD, it was scaled to measure the same distance measured using Google Earth. As illustrated in Figure 6.5, the length of the pedestrian bridge was scaled to measure 2,424 inches (202 feet).



Figure 6.4: Google Earth Dimension Verification



Figure 6.5: Bridge Dimension

In addition to the bridge length, the deflection was also measured. For this, two colored lines were drawn in AutoCAD and the distance between them was measured (as presented in Figure 6.5 and Figure 6.6). The first line drawn was the red line, which runs straight from north to south of the bridge. The second line drawn was the blue line, which follows the contour of the bridge. Then an offset was given to the red line so that it touched the tangent of the blue line. In the intersection of the tangent point, the distance between the original red line and the blue line measured 25 inches, as illustrated in Figure 6.6. With this dimension, it is understood that the bridge had an approximate lateral deflection of 25 inches in its most critical point. In addition, looking closely, it can be appreciated that this point corresponds to the location of the second to last structural beam that supported the signs.

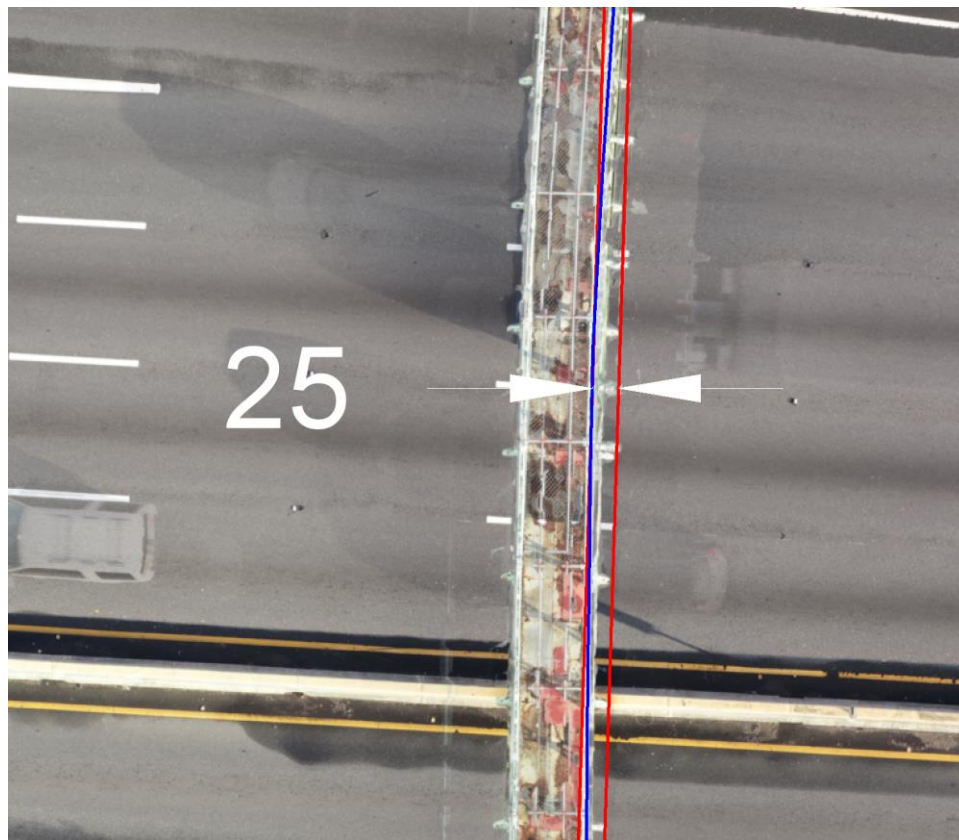


Figure 6.6: Bridge Maximum Plastic Deflection (in inches)

7 Laboratory Testing of Bridge Fragments

Due to its condition, the pedestrian bridge was demolished. This permitted the collection of fragments of the bridge to be tested for determining material strength. This chapter presents information on how the bridge was demolished, how the fragments were recovered and tested, and what were the results of the tests.

7.1 Bridge Demolition and Sample Procurement

The bridge was constructed to cross over the expressway Roman Baldorioty de Castro, one of the main roads of the San Juan Metropolitan Area. This meant that, for the bridge demolition, the expressway had to be closed and traffic redirected. The Puerto Rico Department of Transportation granted permission for the demolition, and it was scheduled to start on March 10, 2019, at midnight, and to last 12 hours.

The demolition started as scheduled. For the demolition, the bridge was moored with two cranes. Figure 7.1 illustrates the rigging of the bridge. The bridge consisted of three spans from which the middle span was the first section to be moored. After the middle span was secured with the cranes, using a man lift that section was cut out with oxyacetylene. After the middle section was completely cut out (as shown in Figure 7.2), it was lowered to the street. Once it was laying in the street it was cut into smaller sections and loaded to a flatbed for final transportation to its disposal facility, as shown in Figure 7.3. This procedure was repeated with the other sections.

After the bridge had been removed and transported to a yard (see Figure 7.4), the company in charge of the demolition was contacted. A formal letter from the Polytechnic University of Puerto Rico (see Appendix C) was submitted to the demolition contractor requesting bridge fragments to be tested to determine mechanical properties of the bridge

elements. After several months of negotiations, on April 26, 2019, the contractor handed the fragments presented in Figure 7.5.



Figure 7.1: Bridge Rigging



Figure 7.2: Bridge Being Removed



Figure 7.3: Transportation of Demolished Bridge



Figure 7.4: Demolished Bridge at Junkyard (April 26, 2019)



Figure 7.5: Bridge Fragments Recovered

7.2 Applicable ASTM Standard

The standard applied to the testing of the recovered bridge fragments was ASTM E8 – *Standard Test Methods for Tension Testing of Metallic Materials* (2016). The purpose of following this ASTM is to perform tension tests to determine the yield strength of the fragments of the bridge that were recovered.

When conducting the tension test, it is important the specimen is held in a position where the load is transmitted axially through the whole cross-sectional area. For this, special care must be taken during the gripping of the specimen in the testing machine. Once the specimen is ready to start the load test, special attention also must be made for the speed of testing. The speed shall not be greater than that at which the lectures can be accurately recorded. When determining yield properties, the speed of testing shall be between 10,000 and 100,000 psi/min.

The specimens used in the tension tests must have the appropriate dimensions. For this project, the specimens were prepared following the ASTM requirements for a Rectangular Tension Test Specimen, as presented in Figure 7.6. The dimensions established from the figure are the following:

- Gauge Length (G): 2.000 ± 0.005 in
- Width (W): $1\frac{1}{2} + 1/8$ in
- Thickness (T): Thickness of material
- Radius of fillet, min (R): $\frac{1}{2}$ in
- Overall length, min (L): 8 in
- Reduced section (A); $2\frac{1}{4}$ in
- Length of Grip Section (B): 2 in
- Width of Grip Section (C): 2 in

Once the specimen has been cut, gauge marks have to be placed on it to determine the elongation. This marks may be painted, scribed, or drawn.

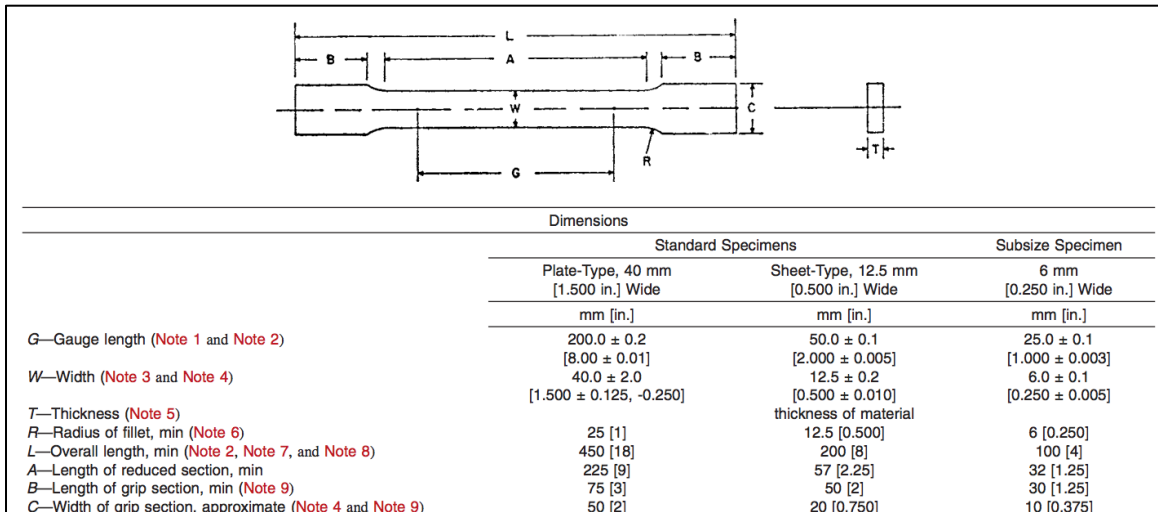


Figure 7.6: Dimensions of Rectangular Tension Test Specimens (ASTM E8, 2016)

7.3 Specimen Preparation and Test Procedure

The recovered fragments of the pedestrian bridge were taken to a metal workshop where a water-jet cutter (shown in Figure 7.7) was used to cut out the specimens to be tested. The specimens were specifically cut out from a C3x5 section used as a diagonal. The water-jet cutter was used so that the properties of the material were not altered by heat. Since deformations and heating by welding, or cutting may affect the properties of the material, special attention was taken to avoid areas of the channel that could have been affected during the demolition of the bridge.



Figure 7.7: Water-jet Cutter

Figure 7.8 illustrates four specimens that were ready to be tested. Prior to testing, the specimens were measured to assure that they had the dimensions required by the ASTM standard, as shown in Figure 7.9 and Figure 7.10.



Figure 7.8: Testing Specimens

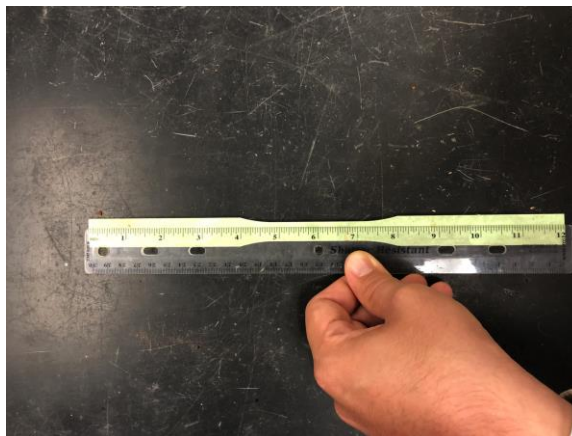


Figure 7.9: Specimen Cut Verification (1 of 2)



Figure 7.10: Specimen Cut Verification (2 of 2)

To determine the cross-sectional area in which the load for the tensile stress would be applied a Mitutoyo Digital Caliper was used. Using the caliper, three different measurements were taken for the thickness and the width of the specimen as illustrated in Figure 7.11 and Figure 7.12. Then the average of these measurements was calculated and used to compute the cross-sectional area of the specimen.



Figure 7.11: Measurement of Specimen Thickness



Figure 7.12: Measurement of Specimen Width

The tensile tests on the specimens were conducted at Construction Materials Laboratory of the Polytechnic University of Puerto Rico, San Juan Campus. The apparatus

that was used to conduct the tests was an Instron 300DX shown in Figure 7.13. A total of six specimens were tested and the results will be discussed in the following section.



Figure 7.13: Tensile Testing Apparatus

7.4 Results

Initially, only two specimens were tested for equipment verification, procedures verification, grip adjustment, and calibration purposes. After these two run tests, the other four specimens were tested, and the results were within expected ranges for the material strength. Table 7-1 illustrates the measurements that were obtained from those four specimens with respect to their width and thickness. With their width and thickness their corresponding area was determined, and this information was used with the tensile testing apparatus.

Table 7-1: Measurements of Specimens

Sample	Width	Average Width	Thickness	Average Thickness	Area
1	0.4690	0.4690	0.1740	0.1743	0.0818
	0.4690		0.1745		
	0.4690		0.1745		
2	0.5020	0.5033	0.1665	0.1670	0.0841
	0.5030		0.1670		
	0.5050		0.1675		
3	0.5055	0.5057	0.1695	0.1690	0.0855
	0.5045		0.1680		
	0.5070		0.1695		
4	0.5040	0.5030	0.1740	0.1728	0.0869
	0.5020		0.1750		
	0.503		0.1695		

Figure 7.14 presents the stress vs. strain diagrams that were obtained from the tension tests. Table 7-2 illustrates the results obtained from each curve. From these results, an average yield strength of 56.58 ksi, was obtained, as presented in Table 7-3. Given the results of the testing tests, it is suspected that the steel used for the construction of the bridge was A572. Still, the average yield stress of 56.6 ksi was used when modeling the bridge to evaluate the impact of wind loads.

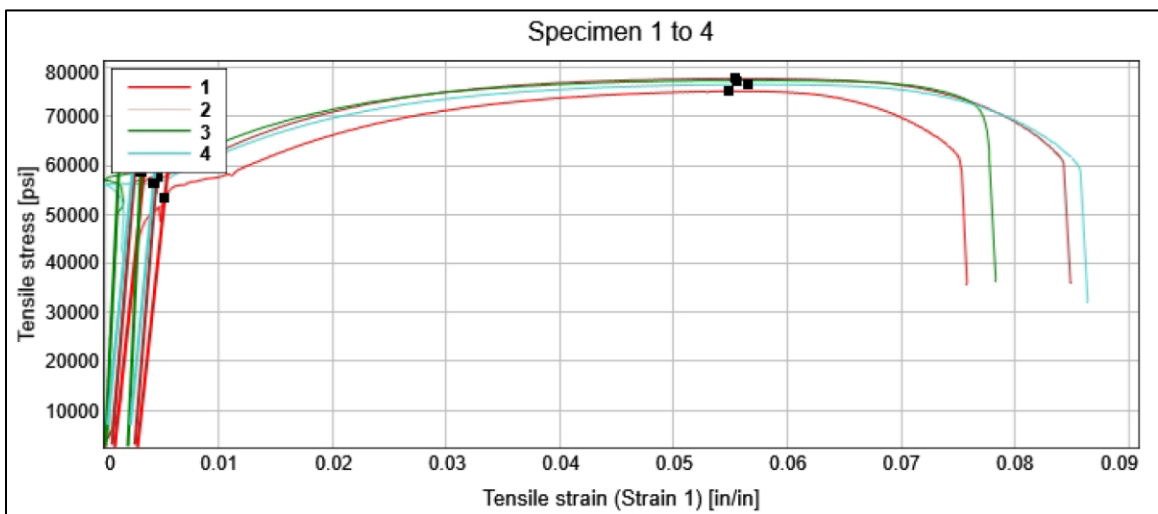


Figure 7.14: Stress vs. Strain Diagrams

Table 7-2: Tensile Tests Results

	UTS [ksi]	Peak Load [lbf]	YTS [ksi]	Elongation after fracture [%]
1	75.22	6149	53.46	26.25
2	77.83	6537	57.85	26.02
3	77.47	6621	58.70	20.42
4	76.62	6660	56.30	24.12

Table 7-3: Yield Strength Average

Curve	Yield Strength (ksi)	Average (ksi)
1	53.46	56.58
2	57.85	
3	58.70	
4	56.30	

8 Bridge Analytical Model and Results

A model was developed to analyze the expected performance of pedestrian bridge PB-1137 when subjected to wind loads similar to those of Hurricane Maria. This chapter presents a description of the analytical model generated and the steps followed to that end. Afterwards, it presents the results obtained when the model was analyzed considering two cases: the bridge with no signs attached to it and the bridge with the signs it had attached when it underwent Hurricane Maria.

8.1 Analytical Model

Using the Bailey Bridge Drawings and the field inspection results, an element list table was created in Excel. With the element list table and the drawings, the total quantities of elements in the bridge were determined. A partial view of the table is presented in Table 8-1.

Table 8-1: Elements List Table

Group	ID	Element	Qty	Sections	Total Qty	From Drawing	
						Nominal Size	Length [in]
Panel	LC1	Longitudinal Channel	1	21	21	4" x 2" x 1/4"	116
	LC2	Longitudinal Channel	1	21	21	4" x 2" x 1/4"	116
	LC3	Longitudinal Channel	1	21	21	4" x 2" x 1/4"	116
	LC4	Longitudinal Channel	1	21	21	4" x 2" x 1/4"	116
	VC1	Vertical Channel	3	21	63	3" x 1" x 1/8"	57
	DCP1	Diagonal Channel Parralel	2	21	42	3" x 1-1/2" x 1/8"	41
	DCP2	Diagonal Channel Parralel	2	21	42	3" x 1-1/2" x 1/8"	41
	DCP3	Diagonal Channel Parralel	2	21	42	3" x 1-1/2" x 1/8"	41
	DCP4	Diagonal Channel Parralel	2	21	42	3" x 1-1/2" x 1/8"	41
	RP1	Reinforcing Plate	1	21	21	9" x 9"	9
RP2	Reinforcing Plate	2	21	42	9" X 4-1/2"	4.5	
Floor	TB1	Transverse Beam	1	21	21	10" x 5" x 5/16"	108
	DGP5	Diagonal Channel Perpendicular	1	21	21	3" x 1-1/2" x 1/8"	51.5
	TBC	Transverse Bracing Channel	2	21	42	4" x 2" x 1/4"	75
	DBC1	Diagonal Bracing Channel	1	21	21	3" x 1-1/2" x 1/8"	89.2
	DBC2	Diagonal Bracing Channel	1	21	21	3" x 1-1/2" x 1/8"	89.2

Using the element list table created with the drawings, another excel table was developed, identifying the location of every member using XYZ coordinates. A partial

view of this table is presented in Table 8-2. The locations identified in the table were used to develop a grid mesh in SAP2000 v21, the program adopted for the analysis.

Table 8-2: Elements Location Table

X				Y				Z			
meters	inches	Description	ID	meters	inches	Description	ID	meters	inches	Description	ID
0	0	Truss Start	a	0	0	Column Start	1	0	0	Transverse Beam Start	k
0.05	2	1st Vertical Bar	b	0.71	28	1st Bracing	2	0.45	18	1st Column	l
0.177	7	Transverse Beam	c	2.41	95	2nd Bracing	3	0.55	22	1st Horizontal Bracing	m
0.7375	29	1st Bracing Center	d	4.11	162	3rd Bracing	4	1.4119	56	Half of Pedestrian Bridge	n
1.019	40	1st Horizontal Frame Connection	e	4.41	174	Column Finish	5	2.2238	88	2nd Horizontal Bracing	o
1.475	58	2nd Vertical Bar	f	4.82	190	Truss Start	6	2.3738	93	2nd Column	p
2.2125	87	2nd Bracing Center	g	4.87	192	1st Channel	7	2.8238	111	Transverse Beam Finish	q
2.543	100	2nd Horizontal Frame Connection	h	4.97	196	2nd Channel	8				
2.9	114	3rd Vertical Bar	i	5.147	203	Transverse Beam	9				
2.95	116	Truss Finish	j	5.6925	224	Center of Truss	10				
				6.365	251	Transverse Beam Bracing	11				
				6.415	253	3rd Channel	12				
				6.465	255	4th Channel	13				

To build the model, a grid mesh system was created, as presented in Figure 8.1. For this, the dimensions known of the bridge either by site inspection or obtained from drawings or from the War Department Technical Manual 5-277, were taken into consideration. The supports of the bridge extreme towers were modeled as pin supports (as per the detail shown in Figure 8.2 and Figure 8.3).

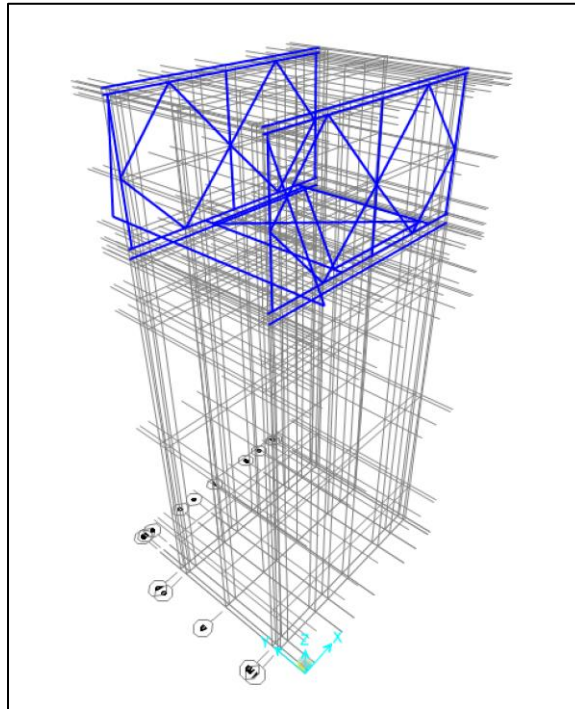


Figure 8.1: Grid Mesh Used to Develop the Analytical Model

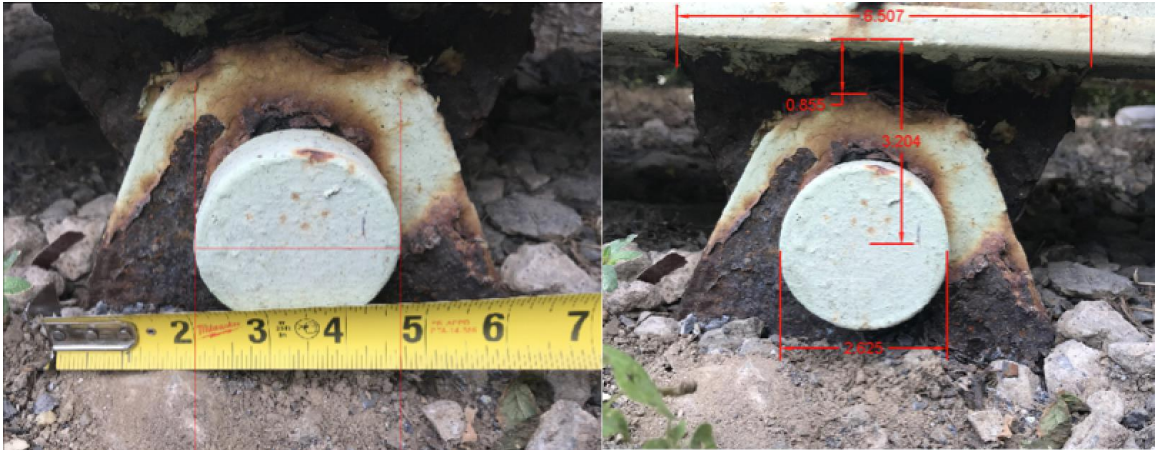


Figure 8.2: Frame Connection of Pedestrian Bridge with Column (1 of 2)

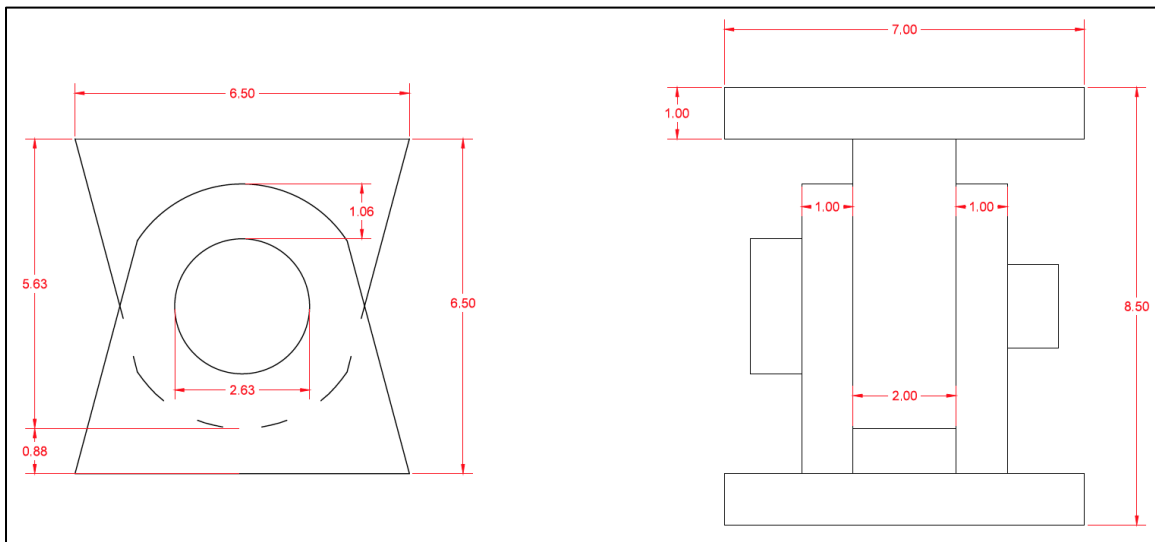


Figure 8.3: Frame Connection of Pedestrian Bridge with Column (2 of 2)

Figure 8.4 presents the simplified finite element model developed. The structural model adopted was a simplified bar element assembly, allowing 3D behavior. The model did not consider eccentricities, the effect of the bars used to support the security netting on top of the bridge, or the decking as a rigid diaphragm. All the bar elements have the shape obtained from the inspection and construction drawings. The main idea of the analysis was to propose a simple model and apply wind loads according to AASHTO specifications considering that the signs are not present, and then apply wind loads considering the presence of the signs. By comparing analysis results, it could be determined if the presence

of the signs produced a significant increase on members strength demand on the elements located in the area that experienced plastic lateral deflections, by comparing interaction equation results in both cases (without and with signs).

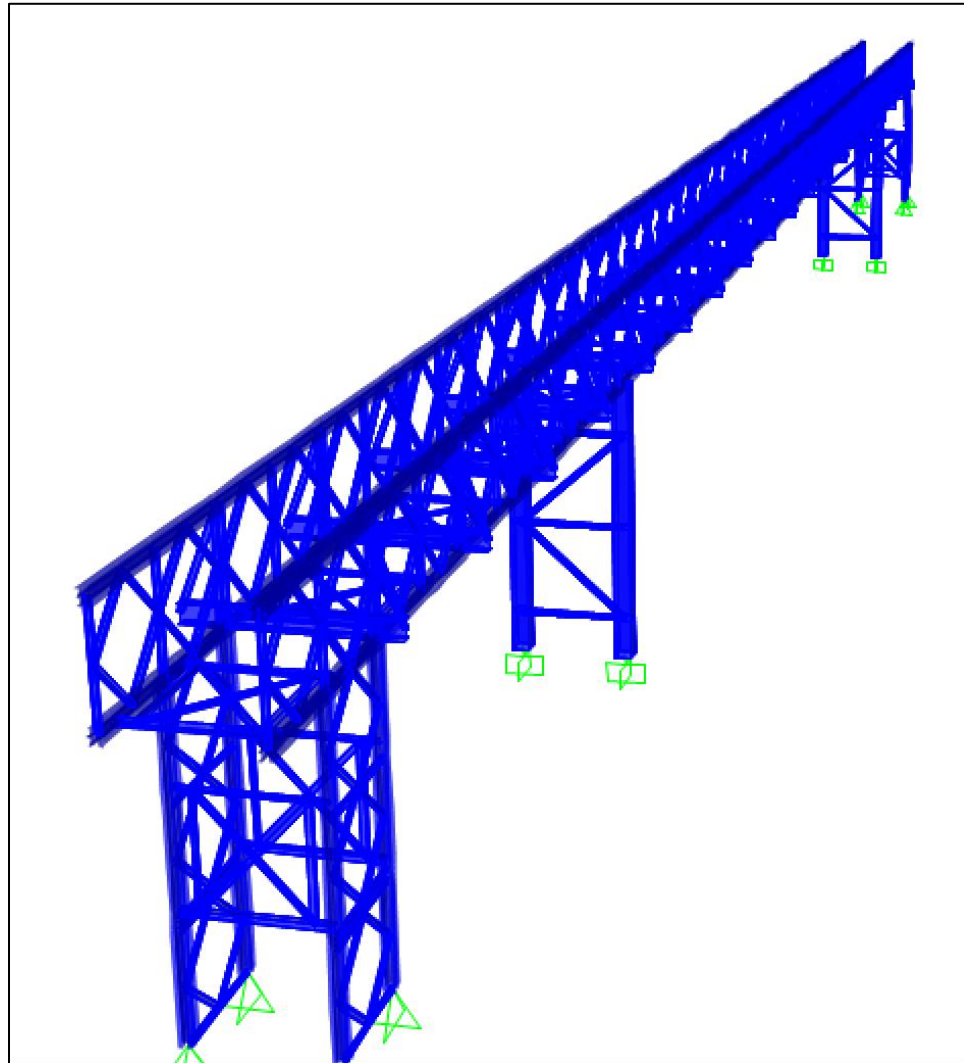


Figure 8.4: Simplified Finite Element Model

8.2 Analysis Without Signage

The first model analyzed was assuming no signage was installed in the pedestrian bridge, to determine the behavior of the bridge in its neutral state (previous to the installation of traffic signs).

Regarding wind loads, the *LRFD Guide Specification for the Design of Pedestrian Bridges* (AASHTO, 2009) refers to the *AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaries, and Traffic Signals* for the calculation of wind loads. Therefore, wind loads were calculated according to the *LRFD Specifications for Structural Supports for Highway Signs, Luminaries, and Traffic Signals*, 1st edition (2015), with interim revisions of 2017 to 2020.

Considering a wind gust speed of 135 mph, the computations presented in Figure 8.5 summarize the computations to obtain the pressure over truss elements, which resulted in 106.4 psf.

Loads on truss elements		
Basic wind speed (mph):	$V := 135$	This value was arbitrarily selected.
Height and exposure factor:	$Kz := 1.0$	AASHTO Signs Specifications suggests establishes that a $Kz = 1.0$ may conservatively be assumed for structures with a height of less than 33 ft.
Directionality factor:	$Kd := 1.0$	This factor accounts for "the reduced probability that the design event wind direction aligns with the most aerodynamically vulnerable direction of the structure...the owner may elect to use $Kd = 1.0$..."
Gust effect factor:	$G := 1.14$	This is the minimum value established by the AASHTO Signs Specifications. Is also the value used in the example presented in the AASHTO Pedestrian Bridge Specifications. For a more exact value, the equations of ASCE/SEI 7 have to be used.
Drag coefficient	$Cd := 2.0$	Value taken from AASHTO Sign Specifications, Table 3.8.7-1. Value is also used in the example presented in the AASHTO Pedestrian Bridge Specifications.
$Pz := 0.00256 \cdot Kz \cdot Kd \cdot V^2 \cdot G \cdot Cd$		
$Pz = 106.3757$	Load in pounds per square feet (psf) that has to be applied to the members of the truss laterally. For each member, this load shall be multiplied by the projected width to determine the corresponding load in pounds per foot (lb/ft). This load should be applied to both trusses (windward and leeward), according to the example presented in the AASHTO Pedestrian Bridge Specifications.	

Figure 8.5: Summary of Computations for Wind Pressure over Truss Elements

Figure 8.6 presents an image of the resulting lateral deflections due to wind action, while Figure 8.7 presents the results of the design check, showing that the interaction equation results in most of the elements in the plastic deflected area working at low stress demand.

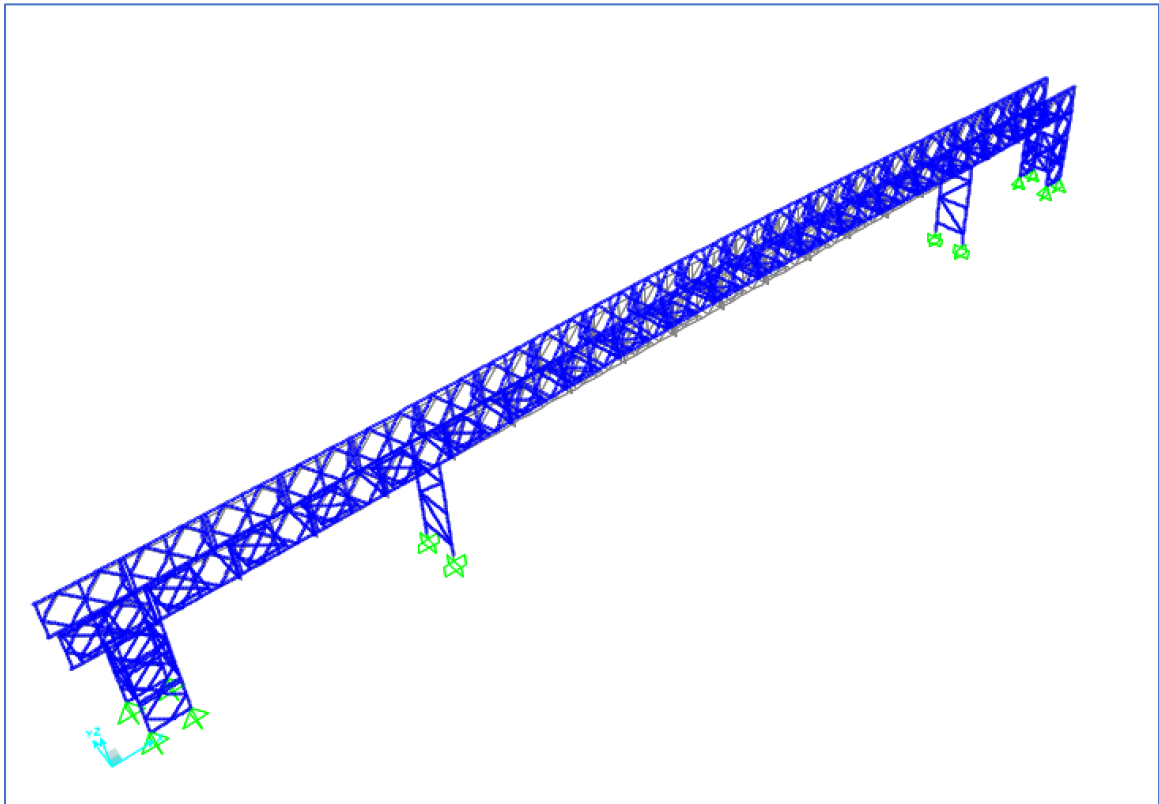


Figure 8.6: Lateral Deflections due to Wind - Without Signs

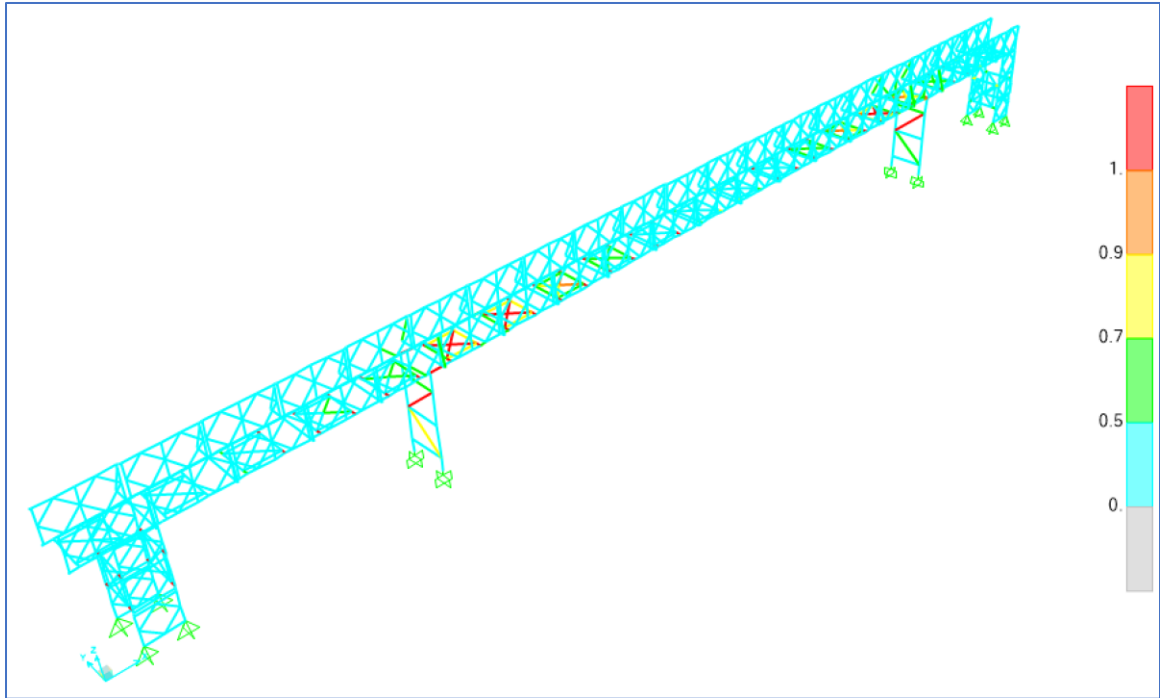


Figure 8.7: Interaction Equation (Strength Demand) Results - Without Signs

8.3 Analysis with Signage

The second model that was created was of the pedestrian bridge with the signage installed. With this model, the bridge can be analyzed to understand the implications that the signage brought to the bridge. Figure 8.8 illustrates the signage that was installed on the bridge at the time Hurricane Maria made landfall in Puerto Rico. Figure 8.9 through Figure 8.11 illustrate the structure that held the signage in place. From Figure 8.11 it was determined that the structure that held the signage in place had 5 vertical beams, and from Figure 8.10 it is evident that the beams were attached to the truss vertical channels with U-bolts in two points.



Figure 8.8: Signage Installed at the Pedestrian Bridge



Figure 8.9: Signage Support Structure (1 of 3)



Figure 8.10: Signage Support Structure (2 of 3)



Figure 8.11: Signage Support Structure (3 of 3)

Since the transportation signs had been removed previous to the moment of inspection, the dimensions of the signs were estimated from previous photos as 35-ft wide and 10-ft in height. It was assumed that the distribution of loads to the supporting beams was by tributary areas, and that each beam, in turn, distributed half the load to each support. To evaluate an overall response, instead of a local response, these support loads were applied directly to the top and bottom chord in the connection of the vertical elements to the chord.

Figure 8.12 presents an image of the resulting lateral deflections due to wind action. It can be appreciated that the general behavior observed on the bridge, with the plastic deformation, is captured.

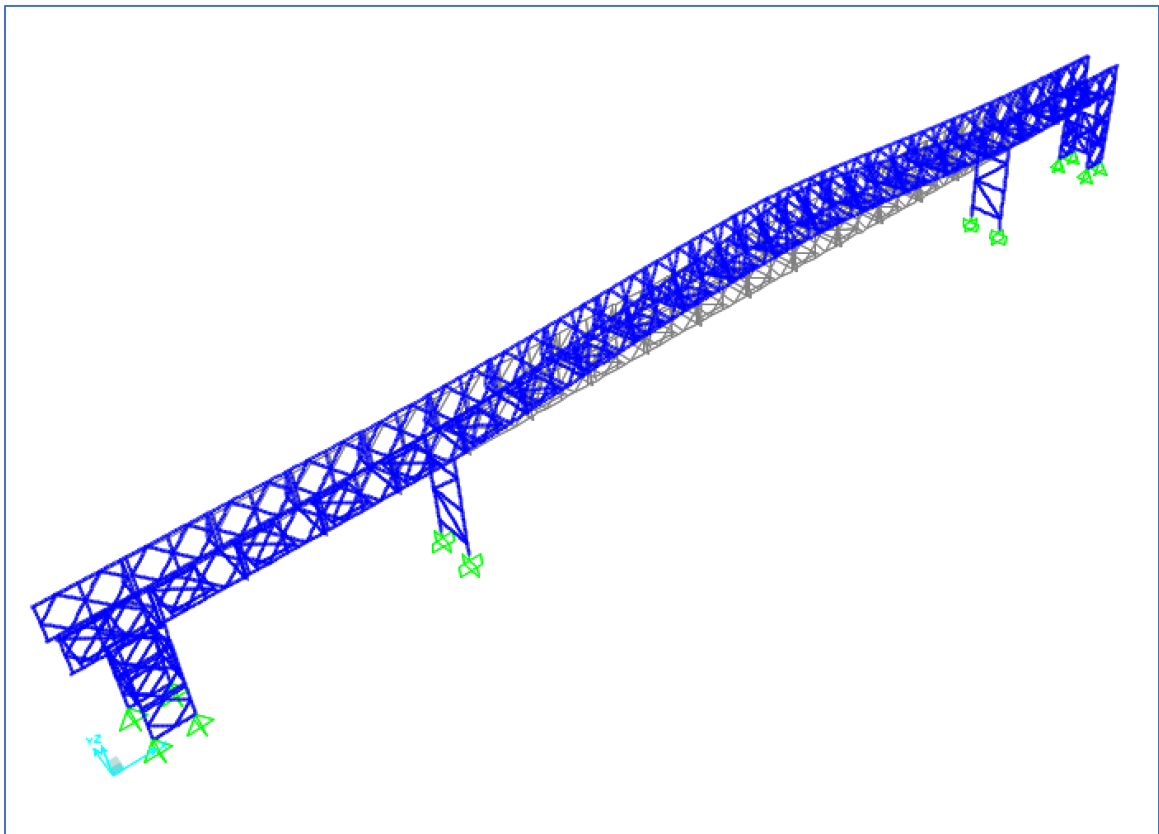


Figure 8.12: Lateral Deflections due to Wind - With Signs Included

Figure 8.13 displays the results of the design check, showing that the interaction equation results in most of the elements in the plastic deflected area working with a much higher stress demand that the previous analysis. This leads to the conclusion that the installation of the signs had an important role in the lateral inelastic deformations.

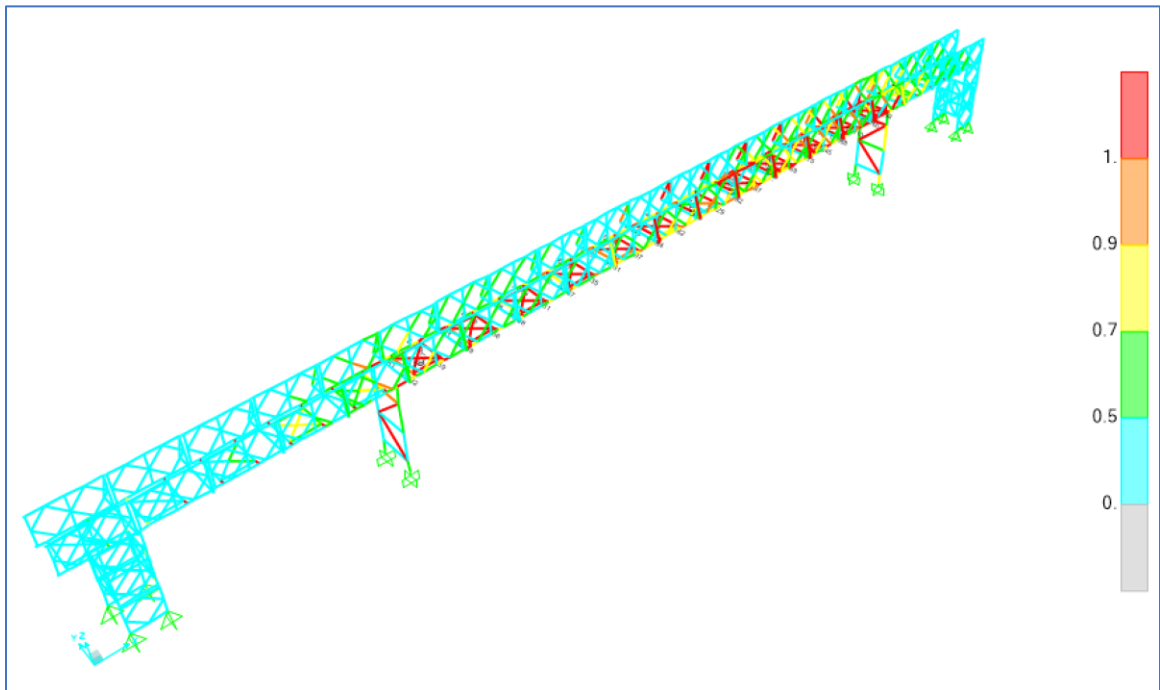


Figure 8.13: Interaction Equation (Strength Demand) Results - With Signs Included

8.4 Results analysis

The simplified FE analysis proposed demonstrated that large signs may impose significant overstress in bridge elements, and that the signs should have played a major role in the inelastic lateral deflections that the bridge experienced due to Hurricane Maria.

In addition to the presence of the signs, the resulting inelastic lateral deflections that actually occurred in the bridge may have been aggravated by: (a) the presence of many corroded elements and connections, (b) the detail of hinge support (explained below), and (c) the detail of sign connection to the truss (also explained below).

Figure 8.14 displays that the hinge support plate appeared to have experienced lateral deflection, resulting in twisting of the upper beam. This situation may be probably due to eccentricity between the lateral load applied by chords on top of the beam and the hinge location. Figure 8.15 presents other view of this situation. This condition requires further study to assess a proper design and may be an extension of current project.

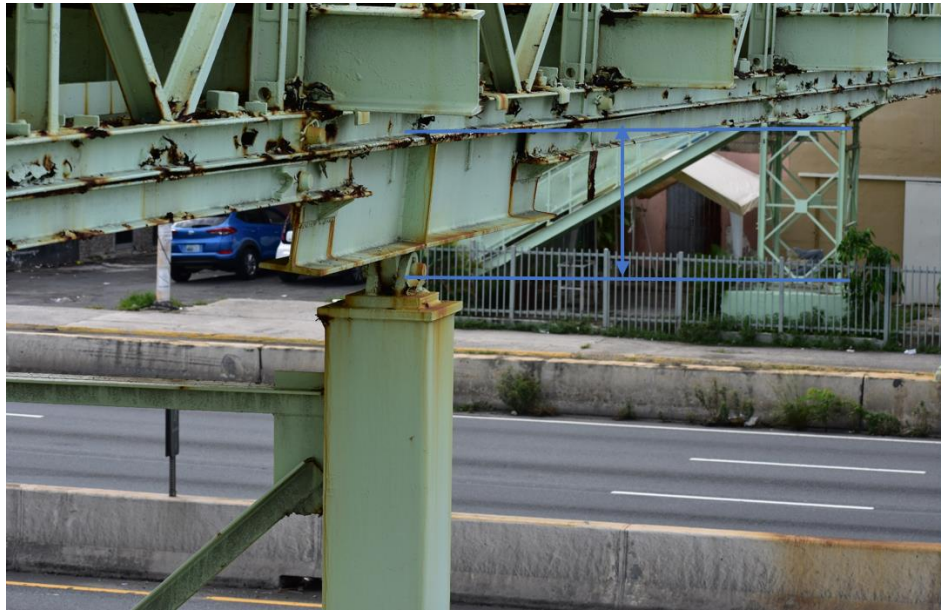


Figure 8.14: Eccentricity of Pin-Plate and Lateral Load



Figure 8.15: Twisted Pin-Plate and Supporting Beam

Due to their layout (see Figure 8.16), sign supports applied significant load to the top chord (with lateral bracing trough transverse beams and diagonals) that may be responsible of the resulting twisting of the truss panel displayed in Figure 8.17.



Figure 8.16: Location of the Signs Support



Figure 8.17: Twist of the Truss Panel

9 Conclusions and Recommendation

The main conclusions and recommendations of the project are the following:

- The visual inspection revealed advanced corrosion in almost all components of the bridge, that presented loss of section, loose connections, and some missing elements.
- The condition assessment based on the visual inspection resulted in a severe condition for all the components evaluated.
 - The bridge would have been probably dismantled despite the inelastic lateral deflections.
 - It was also a significantly old bridge (1968), 49 years old at the time of Hurricane Maria, probably near its expected service live.
- The simplified FE analysis performed demonstrated that large signs may impose significant overstress in bridge elements. This leads to the conclusion that the installation of the signs had an important role in the lateral inelastic deformations experienced by the bridge during Hurricane Maria.
- In addition to the presence of the transportation signs, the resulting inelastic lateral deflections that actually occurred in the bridge may have been aggravated by:
 - The presence of corroded elements and connections.
 - The details of truss hinge support.
 - The details of sign connection to the bridge elements.
- Placing signs on existing bridges should be avoided without the proper:
 - Analysis to determine if reinforcement of the structure is required.

- Design (and maintenance) of the sign support.

The analysis and design should be performed according to current codes (treated as a retrofit), to assure resilience under current design loads and state of the practice.

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Appendix A – Bridge Drawings 2001

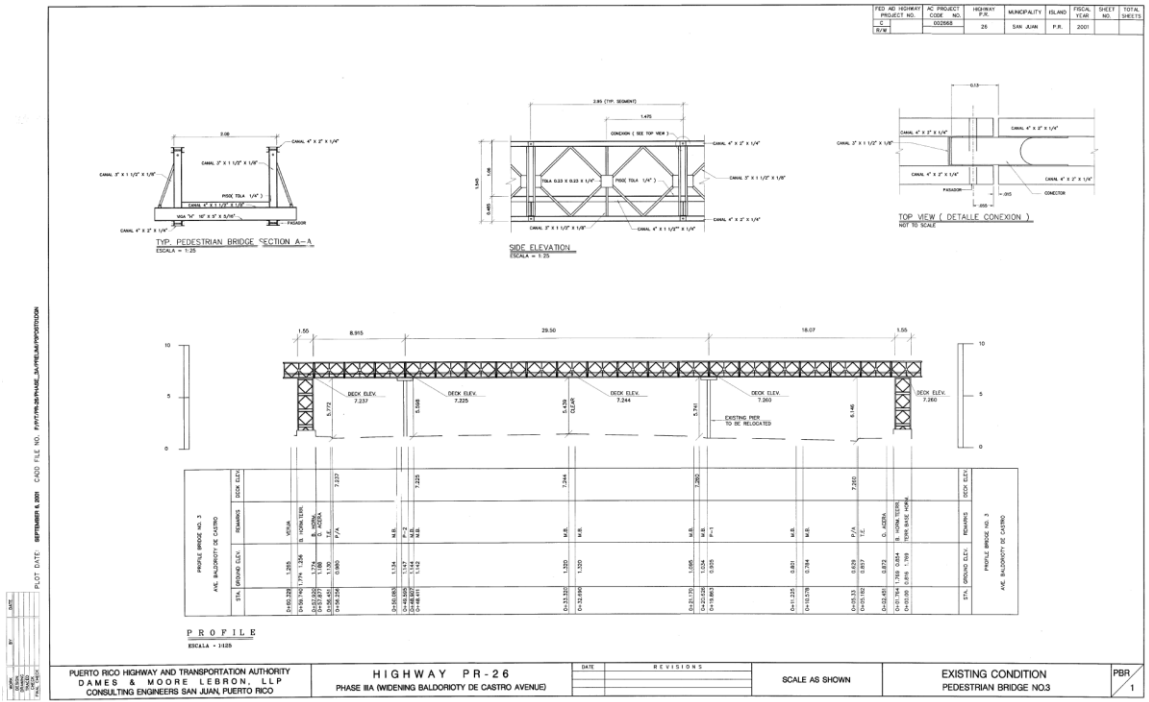


Figure A - 1: Bridge Drawings 2001 – Sheet 1

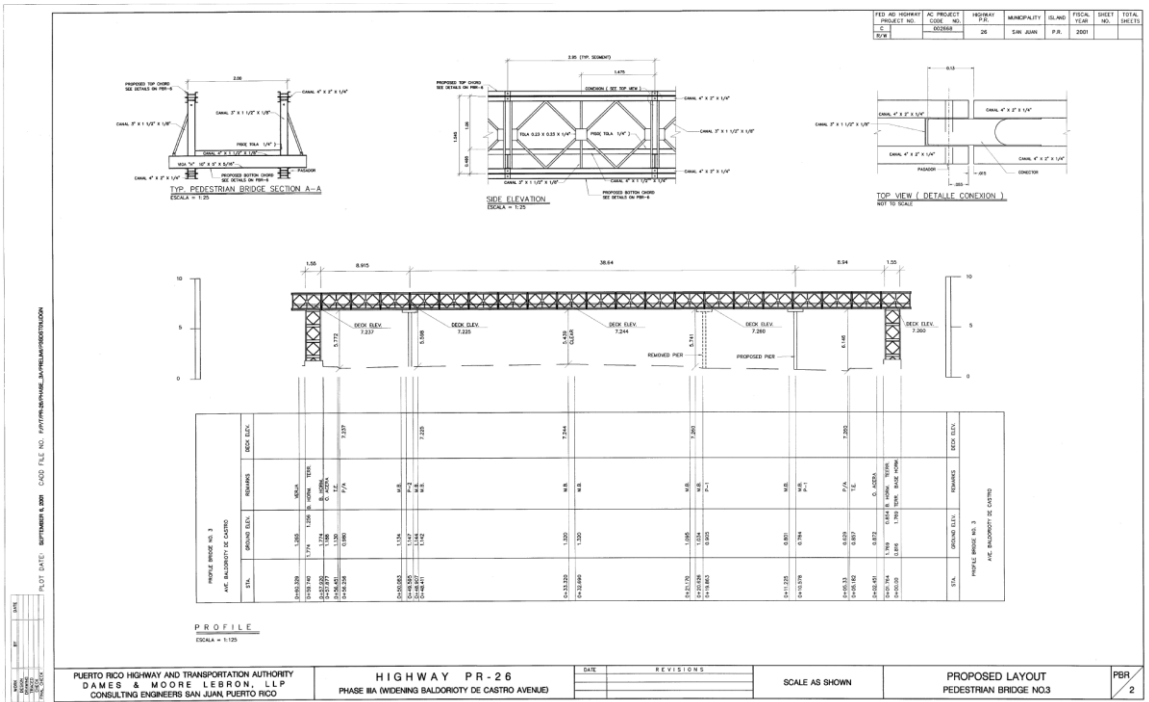


Figure A - 2: Bridge Drawings 2001 – Sheet 2

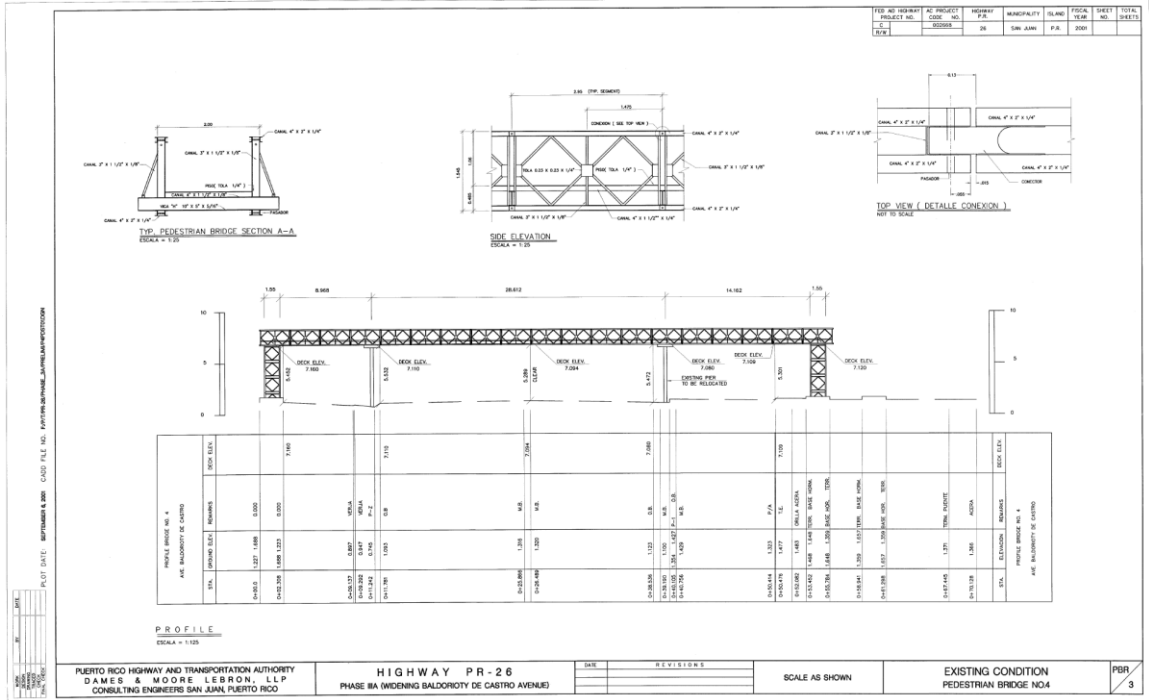


Figure A - 3: Bridge Drawings 2001 – Sheet 3

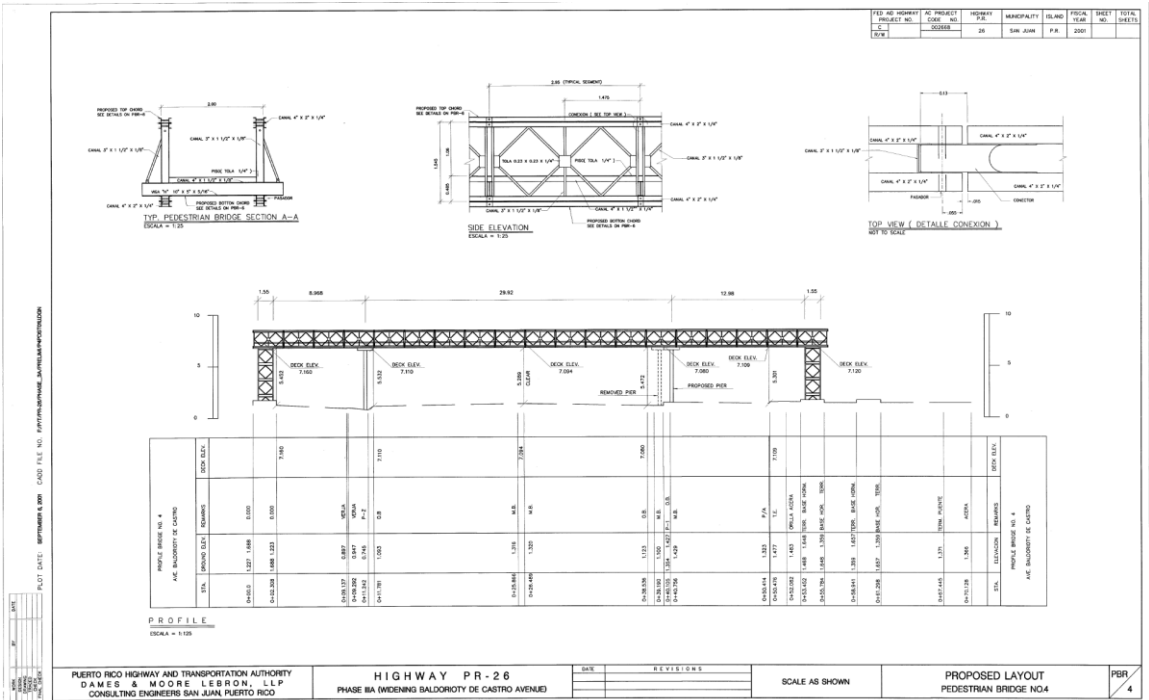


Figure A - 4: Bridge Drawings 2001 – Sheet 4

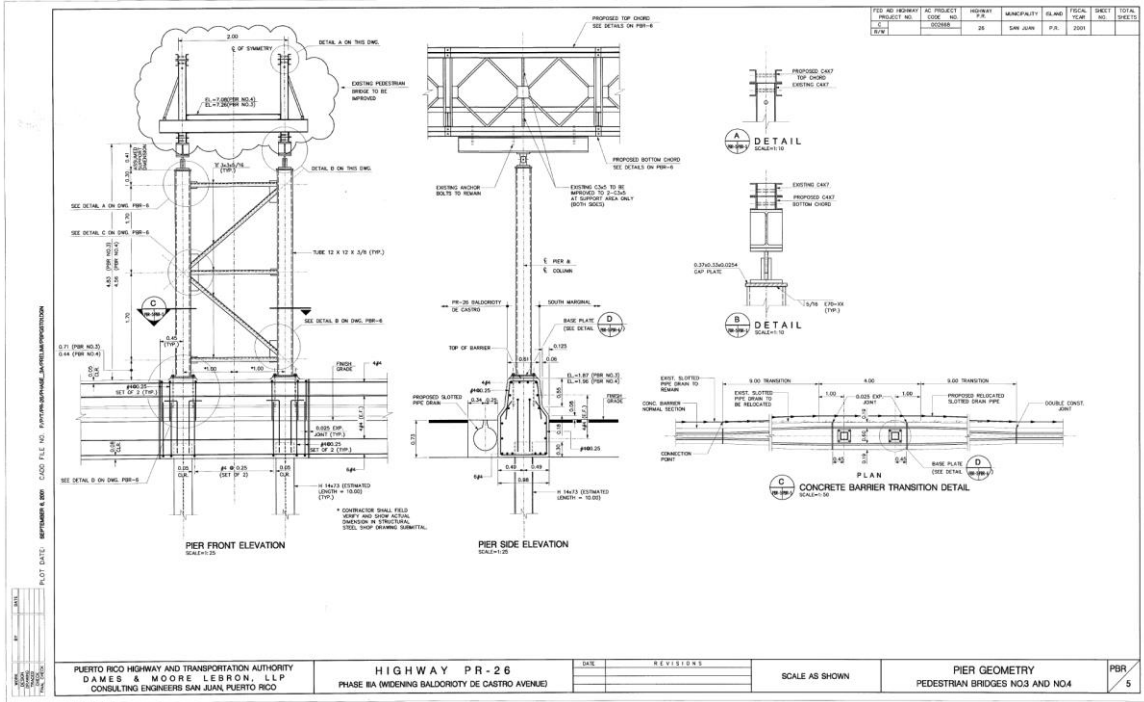


Figure A - 5: Bridge Drawings 2001 – Sheet 5

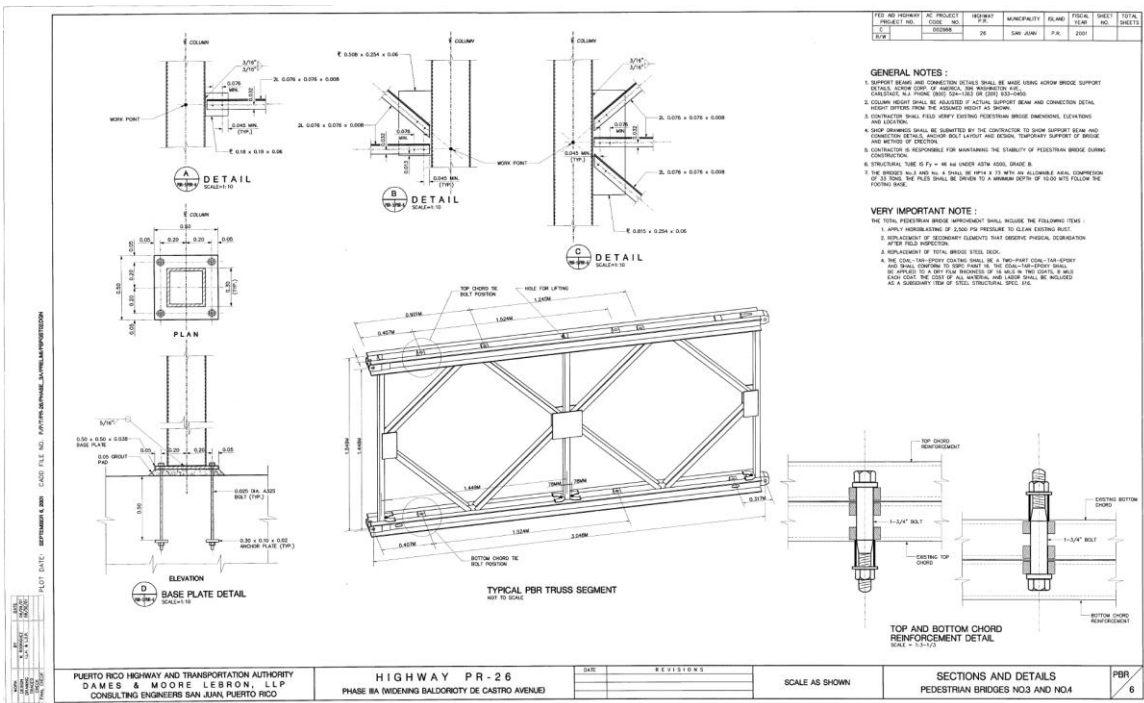


Figure A - 6: Bridge Drawings 2001 – Sheet 6

Appendix B - Collection of images of the bridge



Figure B - 1: East side panels from right to left 1, 2 & 3



Figure B - 2: East side panels from right to left 2, 3 & 4



Figure B - 3: East side panels from right to left 3, 4 & 5

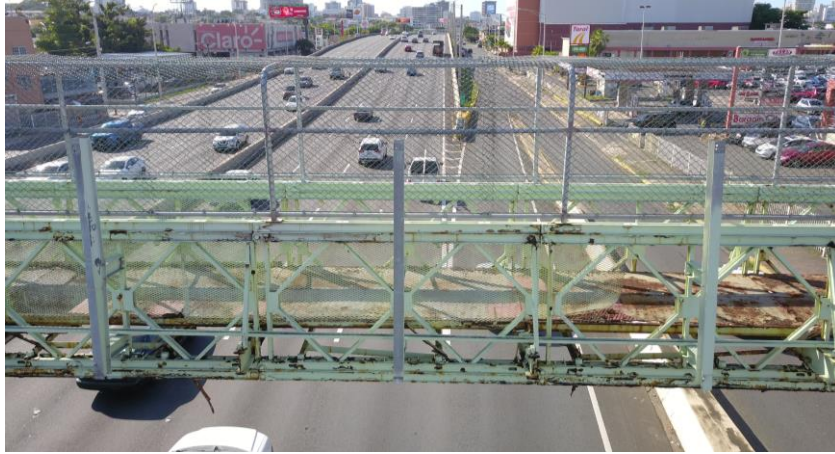


Figure B - 4: East side panels from right to left 3, 4 & 5

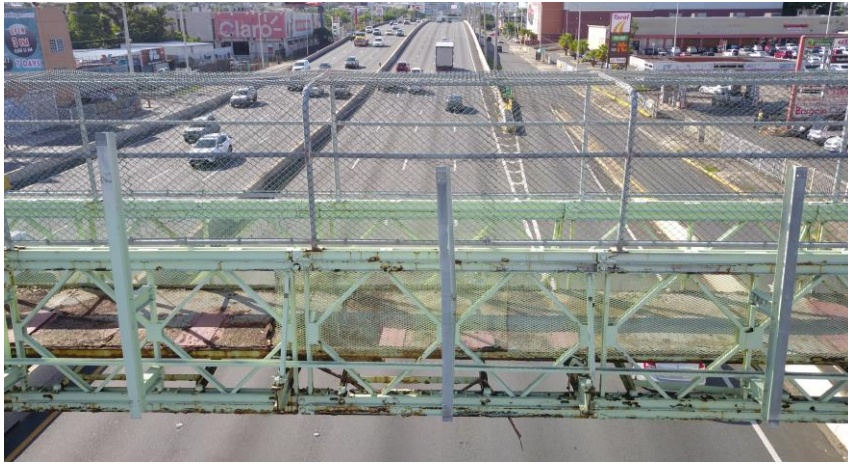


Figure B - 5: East side panels from right to left 6, 7 & 8



Figure B - 6: East side panels from right to left 8, 9 & 10



Figure B - 7: East side panels from right to left 9, 10 & 11



Figure B - 8: East side panels from right to left 10, 11 & 12



Figure B - 9: East side panels from right to left 10, 11, 12 & 13



Figure B - 10: East side panels from right to left 11, 12, 13 & 14



Figure B - 11: East side panels from right to left 13, 14 & 15



Figure B - 12: East side panels from right to left 16, 17, 18 & 19



Figure B - 13: East side panels from right to left 17, 18, 19 & 20

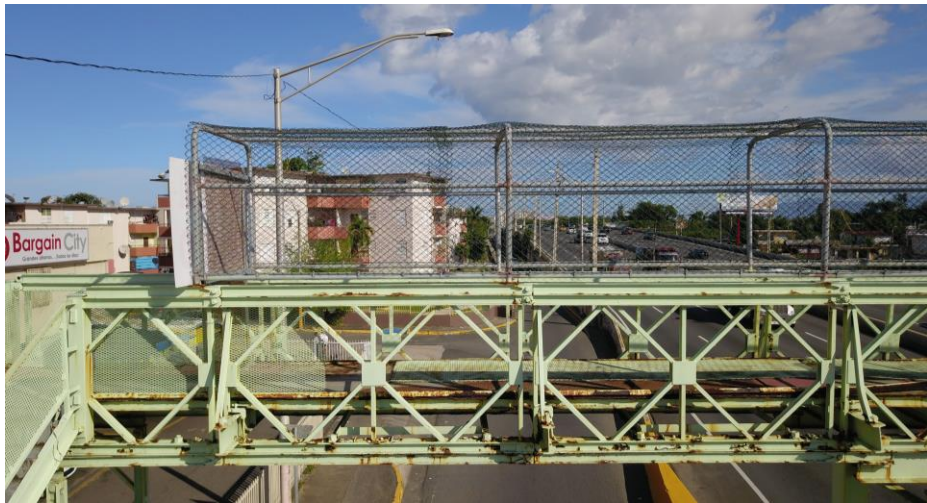


Figure B - 14: West side panels from left to right 1, 2, 3 & 4



Figure B - 15: West side panels from left to right 2, 3, 4 & 5



Figure B - 16: West side panels from left to right 4, 5, 6 & 7



Figure B - 17: West side panels from left to right 6, 7, 8 & 9



Figure B - 18: West side panels from left to right 8, 9, 10 & 11



Figure B - 19: West side panels from left to right 10, 11, 12 & 13



Figure B - 20: West side panels from left to right 11, 12, 13 & 14



Figure B - 21: West side panels from left to right 13, 14, 15 & 16



Figure B - 22: West side panels from left to right 14, 15, 16, 17 & 18



Figure B - 23: West side panels from left to right 17, 18, 19 & 20



Figure B - 24: West side panels from left to right 17, 18, 19 & 20

Appendix C – Letter Requesting Bridge Samples



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22 de abril de 2019

A quien pueda interesar:

La Universidad Politécnica de Puerto Rico recinto de San Juan se encuentra realizando un proyecto de investigación sobre el efecto del huracán María en la infraestructura de traspotación. Se tomó como caso de estudio con el puente peatonal que se encontraba en el Km 4.1 del Expreso Román Baldorioty de Castro, San Juan, Puerto Rico. El mismo fue removido recientemente ya que representaba un peligro para los peatones y el expreso Román Baldorioty de Castro.

El caso de estudio surge luego del huracán María ya que se observó que el puente había sufrido deflexiones laterales permanentes. Se entiende que la deflexión es resultado de los fuertes vientos producidos por el huracán sobre un rotulo que se encuentra en el puente. El propósito general del caso de estudio es obtener los datos sobre la geometría y materiales del puente, para luego desarrollar modelos avanzados del mismo y poder obtener las velocidades del viento del Huracán María que provocaron las deflexiones, y corroborar si la presencia de los letreros fue determinante en la respuesta o no.

Como parte del caso del estudio nos interesa obtener piezas del puente para poder determinar, mediante pruebas de laboratorio, las propiedades mecánicas del material que se utilizó para construir el puente en la década del 60. Por este medio le solicitamos si es posible pasar por las facilidades de donde se encuentra el puente para remover algunas piezas las cuales nos ayudarán con el caso de estudio. Muchas gracias de ante mano por su colaboración; la misma será muy valiosa para PR y el entendimiento de los efectos del huracán María en la isla.

Cordialmente,

Gustavo E. Pacheco-Crosetti, PhD, PE
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Full Professor, Department of Civil Engineering, Environmental Engineering and Land Surveying
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